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three phases: phase 1 involved the model verification tests, phase 2 involved the Mayport Naval Basin study, and phase 3 involved the Mill Cove study. Phase 2 is reported herein; phases 1 and 3 are reported in Reports 1 and 3 of this series

The model verification tests described in Report 1 indicated that the model hydraulic and salinity regimes were in satisfactory agreement with those of the prototype for comparable conditions. Model verification also included a comprehensive shoaling verification of shoaling rates and patterns in the navigation channel and Mayport Naval Basin. During the shoaling verification, model operation procedures were developed by trial and error to achieve satisfactory reproduction of observed prototype shoaling distribution patterns within the various reaches of the navigation channel and in Mayport Basin. This report contains the results of tests conducted for phase 2 of the study.

Based on model shoaling test results conducted for 11 proposed plans, four plans were selected on their merit to reduce shoaling within the basin for further, more extensive testing. Plans 1, 3, 4B, and 5B were tested extensively. Model test results showed that none of the four plans would cause any major change in and along the navigation channel with respect to existing current velocities, salinities, channel shoaling, or dye flushing; however, each plan did result in considerable changes within the basin.

Plans 1 and 3 resulted in better navigation conditions in the basin in the vicinity of the carrier berths as maximum current velocities in this area were generally less than those of base conditions. Plans 4B and 5B resulted in increased maximum flood current velocities in the carrier berthing area that could create berthing problems for the carriers and other vessels.

Flow predominance calculations showed that plan 1 resulted in increasing the net flow in the ebb direction throughout the basin by about 2.7 percent, while plan 3 resulted in decreasing the net flow in the ebb direction by about 2.7 percent. Plans 4B and 5B changed the base condition net ebb flow into a significantly strong net flood. Maximum effects to base condition flow predominance for each plan were similar in that greatest effects were observed at middepth and bottom elevations.

Average salinity concentrations in navigation channel, exit channel, and basin were generally slightly lower than the base for each of the four plans investigated. There was no indication that one plan resulted in more or less change than another, as a very large percentage of the data was within limits of accuracy of repeating identical model tests, particularly within the basin.

Results of dye tests do not indicate any significant improvement of flushing for plans 1 and 3. Results for plans 4B and 5B indicate a significant improvement in overall flushing within the basin near the bottom. Lesser reductions were observed at shallower depths with essentially no change near surface.

Plans involving no secondary openings, gated or otherwise, were more effective in reducing shoaling in the Mayport Naval Basin. Plan 1 was the most effective as model tests showed a reduction in the present rate of approximately 47 percent. Plan 2 showed a reduction in shoaling rates of about 46.6 percent, and plan 3 resulted in reducing the shoaling rate by about 39.5 percent. Plans involving ungated openings (plans 4, 4A, 4G, 5, 5A, and 5C) each resulted in an increase of shoaling in the basin. Plans involving gated openings (4B and 5B) effected a reduced shoaling rate in the basin but were not as efficient as plans which did not include an opening.

Surface current pattern photographs showed no adverse effects resulting from any of the four plans investigated extensively.

Unclassified

PREFACE

The model study reported herein was requested by the U. S. Army Engineer Division, South Atlantic, in a letter to the Office, Chief of Engineers, U. S. Army, dated 14 April 1973, and was subsequently approved in a letter to the South Atlantic Division, dated 5 November 1973. Authority to initiate the investigation was granted by the U. S. Army Engineer District, Jacksonville, in a letter to the Director, U. S. Army Engineer Waterways Experiment Station (WES), dated February 1974.

Design and construction of the model were accomplished during the period of February 1974 to November 1975; hydraulic and salinity verification were carried out during the period December 1975-June 1976. Collection of base test data and navigation channel and Mayport Basin shoaling verification were accomplished during the period July 1976-February 1977. After completion of all phases of model verification and base tests, the Mayport Basin study phase of the investigation was initiated. All programmed plan testings for the Mayport study phase were completed in September 1977. This report describes the problems that necessitated this phase of the model investigation and the studies that were conducted in the model.

The study was conducted in the Hydraulics Laboratory of WES under the general supervision of Messrs. H. B. Simmons, Chief of the Hydraulics Laboratory; F. A. Herrmann, Jr., Assistant Chief of the Hydraulics Laboratory; R. A. Sager, Chief of the Estuaries Division; G. M. Fisackerly, Chief of the Harbor Entrance Branch; and N. J. Brogdon, Jr., Project Engineer. Technicians of the Estuaries Division who assisted throughout the investigation included Messrs. J. W. Parman, D. M. White, and D. M. Stewart. This report was prepared by Mr. Brogdon.

Directors of WES during the course of this investigation and the preparation and publication of this report were COL G. H. Hilt, CE, COL John L. Cannon, CE, and COL Nelson P. Conover, CE. Technical Director was Mr. F. R. Brown.

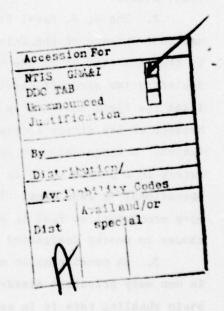
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CONVERSION FACTORS, U. S. CUSTOMARY TO METRIC (SI) UNITS OF MEASUREMENT

U. S. customary units of measurement used in this report can be converted to metric (SI) units as follows:

Multiply	Ву	To Obtain
cubic yards	0.7645549	cubic metres
cubic feet per second	0.02831685	cubic metres per second
feet	0.3048	metres
feet per second	0.3048	metres per second
miles (U. S. statute)	1.609344	kilometres
square feet	0.09290304	square metres
square miles (U. S. statute)	2.589988	square kilometres



MAYPORT-MILL COVE MODEL STUDY MAYPORT NAVAL BASIN STUDY Hydraulic Model Investigation

PART I: INTRODUCTION

Background

- 1. This model study was a joint effort by the Department of the Navy and the U. S. Army Engineer District, Jacksonville, to study two separate problem areas on the St. Johns River. This report (Report 2) presents the results of the Mayport Naval Basin study (Navy study); Report 1 presents the hydraulic, salinity, and shoaling verification phase; and Report 3 presents the results of the Mill Cove study (District study).
- 2. The U. S. Naval Station Mayport is vitally important to the national defense of the United States of America and is best exemplified by the fact that it is the home port of 27 ships of the Atlantic Fleet, including two aircraft carriers. The compact layout of support facilities for fleet units makes Mayport an economically efficient base and because of its direct access to open sea, it is also a strategic installation. A large ship can get under way and be operating in unrestricted waters in a period of 15 to 30 min; smaller vessels, of course, can be operational in less time. This advantage also makes routine training more economical as fuel is saved due to relatively short transit distances to nearby designated fleet operating areas.
- 3. In consideration of above important advantages, however, there is one very prevalent disadvantage: Mayport shoaling history. The basin shoaling rate is in excess of 600,000 cu yd* annually. Maintenance of the basin design depth of 42 ft mean sea level (msl) necessitates

^{*} A table of factors for converting U. S. customary units of measurement to metric (SI) units is presented on page 3.

expensive periodic dredging operations and hampers the operational advantages discussed in the above paragraphs. Not only is present dredging cost excessive but nearby disposal sites are fast becoming extinct, which means even greater future cost as the dredged material has to be carried to remote dumping areas.

Purpose

4. The purpose of the model study as stated above was undertaken to investigate two problem areas located in the St. Johns River system. The first study, reported herein, was conducted for the Department of the Navy in an effort to help develop and investigate plans that would reduce shoaling in the Mayport Basin. The second study was conducted for the Jacksonville District to develop and test plans that would improve flushing in Mill Cove and reduce the silting rate. Details of the Mill Cove investigation are contained in Report 3 of the series.

Scope

5. This report describes the results of the Mayport Basin study. Eleven proposed improvement plans were investigated during the course of this phase of the model study. Seven of the plans were subjected to brief testing, primarily visual observations and shoaling tests.

The Model

6. The Mayport-Mill Cove model reproduces approximately 287 square miles of the prototype area including a portion of the St. Johns River upstream to Hibernia Point (4 miles upstream from Doctors Lake); about 93 square miles of the Atlantic Ocean from about 5 miles south and north of the respective jetties and offshore areas well beyond the -60 ft contour; and the system of sloughs, creeks, and rivers that affect tidal action throughout the model area. The Atlantic Intracoastal Waterway from the point of intersection with the St. Johns River navigation

channel was reproduced about 5 miles in the north and south directions. The model upstream from South Jacksonville was bent slightly (11 degrees) to the east in order to fit it within the shelter. The Doctors Lake area was also bent 11 degrees to fit this area in the shelter. The limits of the area reproduced are shown in Figure 1 and Plate 1.

- 7. The model was constructed to linear scale ratios, model to prototype, of 1:500 horizontally and 1:50 vertically. From these basic ratios the following scale relations were computed by the Froudian relations: slope 10:1, velocity 1:7.07, time 1:70.7107, discharge 1:176,777, volume 1:12,500,000, area (cross section) 1:25,000, and area (horizontal) 1:250,000. The salinity and dye concentrations ratios for the study were 1:1. One prototype cycle (semidiurnal) of 12 hr 25 min was reproduced in the model in 10 min 32.34 sec. Horizontal grid coordinates are based on the Florida coordinate system (East Zone), and vertical control was based on USC&GS msl data. The model was approximately 500 ft long and 100 ft wide at its widest point and covered an area of about 32,000 sq ft. It was completely enclosed to protect it and its appurtenances from the weather, and to permit uninterrupted operation.
- 8. For the tests reported herein, the model was a fixed-bed type, molded to the most recent prototype hydrographic surveys. A description of the model and appurtenances, details of the model adjustment, and model verification are presented in Report 1 of this series.

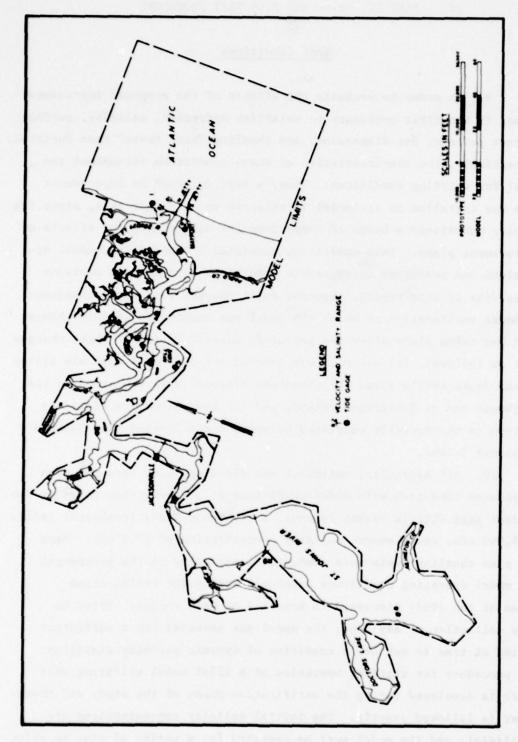


Figure 1. Model limits

PART II: BASE AND PLAN TEST PROCEDURE

Test Conditions

- 9. In order to evaluate the effects of the proposed improvement plans, it was first necessary to establish hydraulic, salinity, surface current pattern, dye dispersion, and shoaling "base tests" that depicted, respectively, the characteristics of these conditions throughout the model for existing conditions. Thus, a test in which no improvement plan was installed in the model is referred to as a base test, since its results constitute a basis of comparison for determining the effects of improvement plans. Base conditions consisted of the 38-ft channel dimensions and prototype hydrographic conditions discussed in previous paragraphs of this report. Several small changes were made subsequent to model verification in which the model was updated to include changes that had taken place after the prototype surveys in 1974. These changes were as follows: (a) one pier was removed and one small slip was filled in the Jacksonville area. (b) the diked disposal area was added on the northwest end of Quarantine Island, and (c) Back River was filled to conform to the recently completed Offshore Power Systems slip located on Blount Island.
- 10. All hydraulic, salinity, and dye dispersion base and plan tests were conducted with model conditions of a 5.4-ft tide range at the control gage (Little Talbot Island), a St. Johns River freshwater inflow of 8,940 cfs, and a source salinity concentration of 33.0 ppt. Base and plan shoaling tests were conducted identically to the procedures and model operating conditions developed during the verification phase of the study discussed in Report 1 of this series. Prior to data collection of any type, the model was operated for a sufficient period of time to achieve a condition of dynamic salinity stability. The procedure for starting operation of a tidal model utilizing salt water is developed during the verification phase of the study and thereafter is followed exactly. The initial salinity concentrations are artificial; and the model must be operated for a period of time to allow

salinities to reach stable values with respect to time, depth, and location. It was found in the Mayport-Mill Cove model that the best procedure was to flood the area upstream from South Jacksonville with fresh water and the ocean and downstream area with ocean water (salt water) to high-water elevation. A barrier installed across the model at South Jacksonville to separate the fresh and salt water during the flooding stage was removed and the tide generator and freshwater inflow were initiated to begin the test. The model was operated until salinity stability was achieved prior to initiation of data collection. For the 8,940-cfs freshwater inflow conditions it was necessary to operate the model for about 12 tidal cycles (about 2 hr) before relatively stable conditions existed, after which data collection could be initiated. To ensure a higher degree of salinity stability when obtaining salinity or dye measurements, salinity and dye samples were not taken until the model had operated for at least 20 tidal cycles (about 3-1/2 hr).

- 11. Locations of stations monitored during base and plan tests are shown in Plate 1. Tide gage locations are identical with those in the verification except that three gages were added (1A, 9, and 10) to more accurately define tidal changes resulting from any plan investigated. Gage 1A reflects tidal conditions in the ocean, while gages 9 and 10 are located at Dame Point and in the Mill Cove weir, respectively. Gage 1 was the control gage throughout all base and plan tests.
- 12. The verification of current velocities and salinities was based on the results of data obtained at 37 locations throughout the estuary (see Report 1). This coverage was not sufficient in the Mayport Basin area to give a complete composite picture of the effects of any possible future improvement plan to be tested; therefore the station location and numbering scheme used during the verification phase of the model study was abandoned during the testing phase for a sampling scheme that would better define the effects of proposed plans.
- 13. Mile 0 of the authorized navigation channel was approximately 5,000 ft west of the outer end of the jetties and was the point of origin for the new numbering scheme for stations in the St. Johns River. Thus, mile 0 was designated range 0. Ranges located along the St. Johns

River generally were designated by a number representing their locations in river miles. Ranges located in between river miles were given composite designations, e.g. ranges OA and OB were located at miles 0.4 and 0.7. Ranges located seaward of mile 0 and in the Blount Island Channel likewise were assigned special designations. These special designations were required because of the way data processing computer codes were encoded. Individual stations located on each range are designated with a letter, beginning with "A" at the southernmost station. Stations located in Mayport Basin were designated by letters prefixed by MB (e.g. MBA, MBB, etc.).

14. Prior to conducting hydraulic, salinity, or dye dispersion tests in the Mayport Basin study, shoaling tests were conducted of 11 proposed plans to identify those most likely to be effective in reducing maintenance dredging costs. The best plans then were subjected to more detailed testing.

Hydraulic Tests

Tides

15. Tidal height data were obtained at 11 locations throughout the model (Plate 1). The data, collected at half-hour intervals, were plotted and smooth curves were drawn through the points.

Current velocities

locations throughout the model for the Mayport Basin study, except that one additional station was included in tests of two plans which incorporated a new channel from the basin to the river. Data were obtained at the surface, middepth, and bottom at half-hour intervals at all locations where bottom elevations exceeded -6 ft msl. Only surface and bottom data were obtained at locations where bottom depths were between -2 and -6 ft msl, while only one sample (surface) was obtained at locations where the bottom elevation was -2 ft msl or less. The half-hour measurements were plotted and smooth curves were drawn through the points. Locations of current velocity stations are shown in Plate 1 and Figure 2.

17. Current velocity data for base and all plan tests were analyzed to determine flow predominance. This method of presenting current velocity data reduces magnitude, direction, and duration of the currents to a single expression that defines the predominant direction and percentage of total flow at any given point. This expression was derived from a conventional plot of velocity versus time at any given point. The area subtended by both ebb and flood portions of the curve was measured and summarized. The area subtended by the flood portion of the curve was then divided by the total area and multiplied by 100 to determine what percentage of the total flow was in the flood direction. A negative (-) sign and a positive (+) sign were designated to indicate ebb direction and flood direction, respectively. For simplification, the percentage of flow in the flood direction was calculated, then a value of 50 percent was subtracted from the calculation to determine predominant direction and magnitude. Using this method of analysis, a value of 0 percent indicates that flows in both the ebb and flood directions are equally balanced, i.e., the areas under the ebb and flood curves are equal. A value of +50 percent indicates that flow at that point is in the flood direction at all times during a tidal cycle, while a -50 percent value indicates flow in the ebb direction throughout a tidal cycle.

Salinity Tests

18. Base test salinity data were obtained at the same 33 locations throughout the model as for velocities. The procedure for determining the number of depths sampled per location was identical with that used for current velocity measurements, except in the basin where five depths were monitored. Salinity concentrations were determined with a salinity meter and were later plotted and smooth curves drawn through the points. Locations of salinity stations are shown in Plate 1 and Figure 2.

Dye Dispersion Tests

19. Model tests were made to determine the flushing rate of the

basin using the following test procedure; the basin was blocked off at low water and a given amount of dye was thoroughly mixed with the water. The initial concentration of the dye mixture in the basin for base and each plan test was 8,700 ppb. The uniform salinity of the water in the basin after mixing was a combination of the average salinity existing in the basin at the time the block was installed plus 1 litre of 33 ppt salt water in which the dye was mixed prior to introduction into the basin. Following the mixing step, the block or blocks (secondary opening when appropriate) were removed at the next low water and sampling was initiated and continued for 16 tidal cycles. The first basin water samples were obtained at hour 4.75, or 3 hr after the block had been removed. Prior to initiating the dye test, the model was operated for 20 tidal cycles to establish salinity stability throughout the model. The exact procedure was followed for base and all plan tests to ensure that the initial concentration was the same for all tests.

20. Locations of the stations sampled are shown in Plate 1 and Figure 2 and were the same as the stations used for velocity and salinity measurements. Within the basin proper, samples were obtained at five depths (surface, quarter, middepth, three-quarter, and bottom), while only two depths were monitored outside the basin (surface and bottom). Outside the basin, samples were obtained as near as possible to time of local occurrence of high-water slack (hws) and low-water slack (lws) at each individual location over a period of 16 tidal cycles. Currents in the basin are very complex and erratic. Slack times in the basin varied over such a wide range that it was decided to sample at times that corresponded to middepth slack periods occurring at stations located in the navigation channel adjacent to the basin. The sampling procedure was identical for all tests and is described as follows. Hws samples at stations located at the sump and in the ocean were obtained at hours 7.0 and 7.75, respectively; stations located in the Atlantic Intracoastal Waterway (AIWW), north and south of the navigation channel, were sampled at hour 9.25; stations located in the entrance, including the basin, starting at range Y up to and including range 5 were sampled at hour 10.25; stations located on ranges 7, 9, and

10 were sampled at hours 11.0, 11.25, and 11.5, respectively. Lws samples in the sump and ocean were obtained at hours 1.0 and 1.75, respectively; the AIWW stations were sampled at hour 3.0; stations just upstream of the basin (ranges 1A-3) were sampled at hour 4.5; Mayport Basin and range 5 samples were obtained at hour 4.75; samples on ranges Y-0B and 9 were collected at hour 5.0; and stations on ranges 7-10 were collected at hour 5.25. Concentrations of the samples were measured by means of a fluorometer. The fluorescent dye used in the model was conservative, that is, it exhibited very minimal decay with time. Model dispersion and flushing rates thus cannot be directly related to the prototype without application of the appropriate decay rate. All measurements are plotted on semilogarithmic graph paper to make a detailed analysis possible at each sampling point.

Surface Current Pattern Mosaics

- 21. Surface current pattern mosaics made for base test and Mayport Basin plan conditions include a portion of the Atlantic Ocean, the entrance area, and portions of the estuary on either side of the navigation channel up to about range 5. These mosaics were used in evaluating the proposed plans effects on current patterns and navigation. The mosaics also provide a means for current velocity measurements in areas too shallow for measurements with the velocity meter. Surface current pattern photographs were made with the model reproducing a 5.4-ft tide and a freshwater inflow of 8,940 cfs.
- 22. The mosaics were prepared from time-exposure photographs of confetti floating on the water surface. A bright light was flashed immediately before the camera lenses were closed, resulting in a bright spot at approximately the end of each confetti streak which indicates the direction of flow. Current velocities can be determined from the photographs by measuring the lengths of the confetti streaks and comparing the lengths with the velocity scale presented in each mosaic. Photographs were taken at hourly (prototype) intervals throughout a complete tidal cycle.

Channel and Basin Shoaling Tests

- 23. As discussed in Report 1, the channel shoaling verification was conducted with the 34-ft channel installed. Base and all plan tests were conducted with the authorized 38-ft channel and model updates described in paragraph 9. For base and plan tests the channel was divided into nine reaches. The new reach (AA) was located in the navigation channel seaward of mile 0.0. The individual sections within each reach were changed from the variable lengths used in the model verification to a uniform length of 1,000 ft. Locations of test reaches and individual test sections are shown in Plate 2. The procedure followed during base and plan was identical with that described in Report 1, the only difference being the plan which was being investigated at that time. The model shoaling material was introduced into the model in exactly the same locations and manner as described in Report 1, even though the channel sections had been modified.
- 24. Two separate base condition tests using gilsonite were conducted for the basin. One base test (base test 1) was conducted with the 38-ft channel and above model revisions. Shoaling tests of plans 1, 2, and 3 (plans without a secondary opening to the basin) were conducted with the identical procedure. Following visual observations of the plans by the District and Department of the Navy personnel, it was decided that basin shoaling tests with improvement plans involving the secondary opening should be tested with the fleet (27 ships) reproduced to scale and anchored in the model, as shown in Plate 3. Each individual ship in the fleet was constructed from wood to the distorted model scale and placed in the basin in accordance with the anchorage scheme furnished by the Department of the Navy. The model ships were weighted to obtain proper draft. A second base test was then conducted with the addition of the ships as the only change. All basin shoaling tests with plans involving the secondary opening (plans 4, 4A, 4B, 4C, 5, 5A, 5B, and 5C) gated and ungated were conducted with the fleet anchored in the basin and results compared with results of base test 2. A comparison of the two base tests showed that the presence of the fleet resulted in a reduction in

the shoaling rate of about 8.2 percent as shown in the following tabulation:

Section	Base 1	Base 2	Base 1 % of Total	Base 2 % of Total
TB1	0	0	0.0	0.0
TB2	10	0	0.4	0.0
TB3	20	0	0.8	0.0
TB4	167	180	6.3	7.5
TB5	415	525	15.8	21.7
NE	730	755	27.7	31.3
SE	528	450	20.1	18.6
SW	357	240	13.6	9.9
NW	275	180	10.4	7.5
Slip	130	85	4.9	3.5
Total	2,632	2,415	100.0	100.0
2 Chan	ge	-8.2		

Since the base tests were sufficiently similar, plan tests 1, 2, and 3 were not retested with the ships in the model.

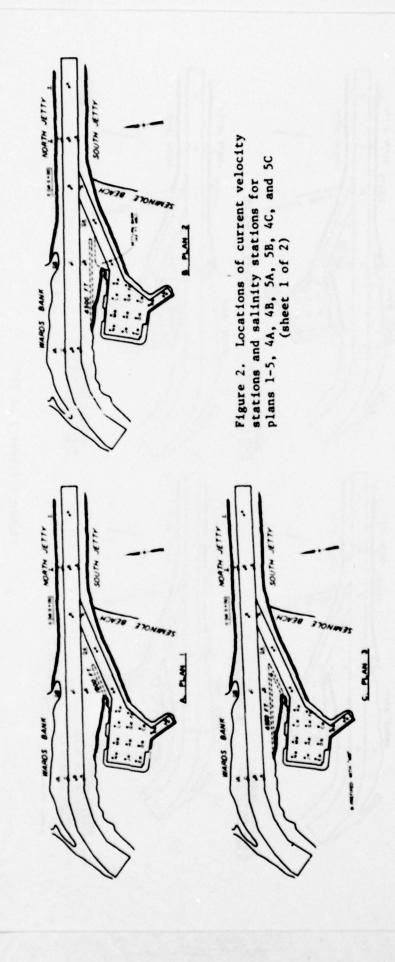
- 25. In addition to the two shoaling base tests conducted inside the basin, a base test 3 using granulated plastic was conducted for the navigation channel.
- 26. A minimum of two identical runs was made with each plan installed in the model. Following the tests, the results were averaged and compared with the base test results to determine effects resulting from the construction of the plans.

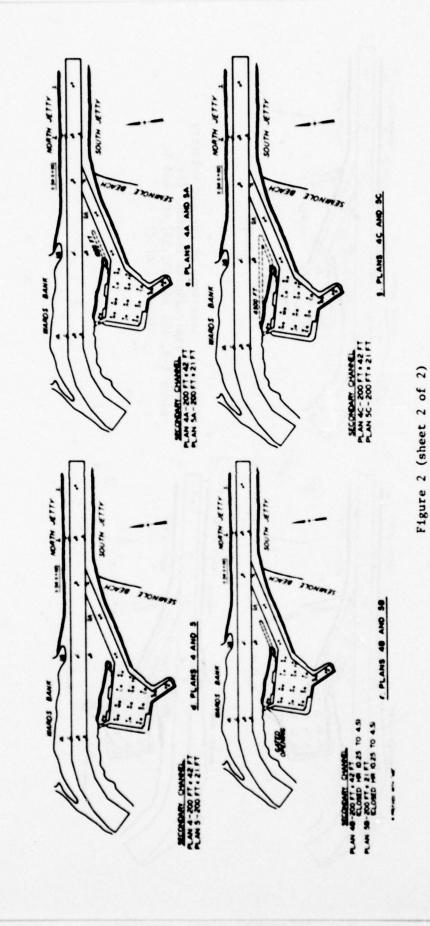
Elements of Plans

27. Prior to selection of plans for model testing and establishment of a testing program, a conference was held at WES between Navy, Jacksonville District, and WES personnel for the purpose of discussing possible solutions to the shoaling problem in the Mayport Naval Basin. Plans included in these discussions were a sea-level canal connecting the basin (slip area) to the ocean and a canal connecting the basin (west side) to Chicopit Bay located west of the basin. These two plans were acknowledged as being potentially promising. However, Navy personnel eliminated the plans on grounds of being economically infeasible

since several rather large and expensive Naval facilities would have to be relocated in order to construct either canal.

28. Five basic plans were investigated during the course of the study; however, basic plans together with various combinations of these plans resulted in a total of 11 overall plans. The elements of the plans are described as follows. Plan 1 (Figure 2a) consisted of a dike about 1,900 ft in length which paralleled the basin exit channel. This dike and all dikes and/or groins in other plans were constructed to be impervious in the model and had top elevations above high water. Plan 1 was referred to as an extension to Wards Bank training wall. Plan 2 (Figure 2b) consisted of a 4,500-ft dike in the river constructed parallel to the navigation channel, with its beginning adjacent to the northwest end of the basin (the upstream end of the existing Wards Bank training wall). A small settling/turning basin was dredged immediately inside the dike to elevation -42 ft msl. Plan 3 (Figure 2c) consisted of the plan 2 dike with a connection back to the seaward end at Wards Bank training wall. This was essentially a combination of plans 1 and 2. Plans 4 and 5 (Figure 2d) involved only the secondary opening in the northwest corner of the basin. The opening width for plans 4 and 5 and all other plans involving the opening was held constant at 200 ft. Plan 4 was molded into the model at a bottom depth of -42 ft msl, while plan 5 had a bottom depth of -21 ft msl. Plans 4A and 5A (Figure 2e) were combinations of plan 1 (1,900-ft dike parallel to exit channel) and the secondary opening located at the northwest corner of the basin. Plan 4A had an opening depth of -42 ft mal, while 5A had an opening depth of -21 ft msl. Elements of plans 4B and 5B (Figure 2f) were identical with plans 4A and 5A, respectively, except that the openings were gated (closed) during the period between hour 10.25 of one tidal cycle and hour 4.5 of the next tidal cycle (ebb current period). The gate was opened during the period between hours 4.5 and 10.25 of each tidal cycle (flood current period). Plans 4C and 5C (Figure 2g) were a combination of the plan 3 dikes and the secondary ungated openings. Plan 4C had an opening depth of -42 ft mel, while plan 5C had an opening depth of -21 ft msl. The secondary opening on all plans had vertical walls.





PART III: RESULTS AND DISCUSSION OF RESULTS

Shoaling Tests

- 29. Shoaling tests were conducted for the base and 11 plan conditions. Results of these tests are shown in Table 1 and in Figure 3. Locations of shoaling sections are shown in Plate 2. The shoaling section identified in Figure 3 as "EXIT CH" is actually the sum of sections TB-1 through TB-5. Base conditions, as previously discussed, varied for the Mayport Basin shoaling study. Base test 1 (no ships) was used as the base to determine effects in the basin of plans that did not involve a secondary opening (Plans 1, 2, and 3). Base test 2 (with model ships) was used as the base for determining effects in the basin of plans involving a secondary opening (Plans 4, 4A, 4B, 4C, 5, 5A, 5B, and 5C). Base test 3, utilized to determine effects of the various plans on shoaling in the navigation channel, did not include the model ships.
- 30. Tests to determine effects of the 11 basic plans on channel shoaling were limited to the downstream three reaches, AA, A, and B; results are shown in Table 1. Model tests showed that plans 4, 4A, and 4B resulted in a reduced shoaling rate in reach AA, while the remaining eight plans resulted in increased shoaling in this reach. The respective shoaling indexes in reach AA for plans 4, 4A, and 4B were 83.3, 66.7, and 33.3 percent, respectively. Shoaling indexes for plans 1, 2, 3, 4C, 5, 5A, 5B, and 5C were 216.7, 150.0, 133.3, 116.6, 116.6, 133.4, 133.3, and 133.3 percent, respectively. Although the shoaling index values for this reach appear to be drastic, it is pointed out that reach AA was not included in the channel verification as no prototype shoaling records were available. During base tests, material that deposited within the limits of this reach was recovered and measured; but the results represent an unknown deposition rate and pattern. It is emphasized that the volumes (30 cc for the entire reach in the base test) recovered for base and plan conditions were extremely small. For example, plan 1 showed an index value of 216.7 percent but had a total

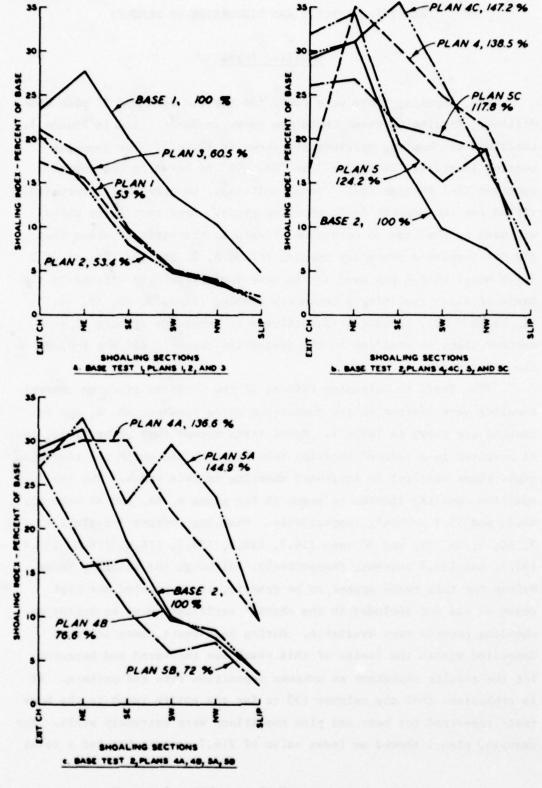


Figure 3. Effects of plans on basin shoaling

volume of only 65 cc of material deposited within the limits of the reach. On the other hand, no shoaling material was introduced into the model in or adjacent to this reach as was done for all other reaches; thus, the only source of material to reach AA was that material moved downstream through reach A by the currents. No reliable conclusions concerning the shoaling volume in reach AA can be reached as a result of channel shoaling tests for this area; however, it can be assumed that the volume would be relatively small.

- 31. Changes in shoaling index values in reach A resulting from plan tests were generally within the limits of accuracy of repeating identical model tests of this type. No plan resulted in reducing the shoaling index for this reach below 90.9 percent (plan 1), and only plan 4 (120.2 percent) increased the shoaling index above 107.2 percent. Very little effects were noted in shoaling patterns.
- 32. Shoaling in reach B was likewise not affected by any of the 11 plans investigated as all test results were well within the limits of accuracy of repeating tests of this type. Maximum shoaling index was 106.0 percent with plan 2 and minimum shoaling index was 91.9 percent with plan 5 installed. Again, as in reach A, no significant changes were noted in shoaling patterns.
- 33. Mayport Basin and exit channel shoaling test results for the 11 proposed improvement plans installed in the model are shown in Table 1 and in Figure 3. The results of plans 1-3 are shown in Figure 3a. These three plans did not involve a secondary opening. Plans 4, 4C, 5, and 5C results are shown in Figure 3b. These four plans involved secondary openings in combinations with the dike of plan 3. Figure 3c shows the effects of plans 4A, 4B, 5A, and 5B. These four plans involved the secondary opening (gated and ungated) in combination with the dike of plan 1.
- 34. Figure 3a shows that plans 1-3 reduced shoaling rates in the exit channel and basin, but altered overall shoaling patterns very little. Plans 1-3 shoaling index values were determined by comparing results with base test 1 (no ship). The shoaling index for plans 1, 2, and 3 are 53.0, 53.4, and 60.5 percent, respectively. Plans 1 and 2

reduced basin shoaling rates a greater amount than any of the other plans investigated.

- 35. Results with plans 4 and 5, shown in Figure 3b, compared with base test 2 data show that each would result in increased overall shoaling rates in the basin by 38.5 and 24.2 percent, respectively.

 Plan 4 resulted in a very substantial reduction in the shoaling rates in the exit channel, but the improvement in this area was offset by the large increases in other areas of the basin, especially in the southeastern, southwestern, and northwestern sections. Plan 4C resulted in a shoaling index of 147.2 percent, a substantial increase in shoaling. Shoaling patterns with plan 4C were similar to plan 4; however, exit channel shoaling was very near the base condition and the peak shoal was located in the southeast section instead of the northeast section with plan 4.
- 36. Shoaling indexes for plans 5 and 5C were 124.2 and 117.8 percent, respectively (Figure 3b). Plan 5 shoaling patterns were very similar to base conditions with small increases in all areas with the exception of the northwest section. Increased shoaling in this one area alone accounted for about 50 percent of the overall increase in shoaling in the entire basin. Plan 5C caused a reduction in shoaling in the exit channel and in the northeast section of the basin; however, the reductions in these areas were heavily outweighed by the large increases in the southwest and northwest sections of the basin. Each plan involving an ungated secondary opening and the plan 3 dike (plans 4, 4C, 5, and 5C) caused increased shoaling in the slip.
- 37. Flans 4A and 5A (Figure 3c), which consisted of secondary openings in combination with the dike of plan 1, resulted in rather significant increases in shoaling in all areas of the basin except the exit channel where small decreases were observed. Shoaling index values for plans 4A and 5A were 136.6 and 144.9 percent, respectively. Plan 4A showed significant increases in all sections of the basin except in the northeast and the exit channel. Plan 5A shoaling rates and patterns were very similar to base conditions in the exit channel and in the northeast and southeast sections of the model; however, very heavy shoals

developed in the southwest and northwest sections of the basin. Shoaling rates in the slip were increased significantly with plans 4A and 5A.

- 38. Results of tests conducted with plans 4B and 5B (gated openings in combination with plan 1) are shown in Figure 3c. Plans 4B and 5B resulted in reducing the shoaling index to 76.6 and 72.0 percent, respectively. With plan 4B installed in the model, major reductions in basin shoaling were realized in the northeast section and to a lesser extent in the exit channel. Shoaling rates or patterns in the other areas were not significantly affected by this plan. With plan 5B installed in the model, major reductions were obtained in the northeast, southeast, and southwest sections of the basin. Maximum effect was in the northeast section. The exit channel, northwest section, and slip were not significantly changed from base conditions.
- 39. Plans 1, 2, 3, 4B, and 5B resulted in reduction of the shoaling rates in the basin. Shoaling was increased with plans 4A, 4C, 5, 5A, and 5C. Plans involving only a secondary channel (gated or ungated) resulted in increased basin shoaling. Secondary channel plans combined with the best dike plan resulted in higher basin shoaling rates than the dike above. On the basis of these shoaling test results, plans 1, 3. 4B, and 5B were selected for more detailed testing. Plan 2 was not selected for additional testing because the model results indicated that the additional cost of constructing the settling basin (as compared with the cost of the 1,900-ft-long dike included in both plans 1 and 2) did not result in any less shoaling in the basin than that for plan 1. Thus, the additional construction cost would not yield any reduction in maintenance costs. Although plans 3, 48, and 58 would have higher construction costs than plan I and also exhibited higher shoaling rates, they were included in the detailed test phase to ensure the selection of a satisfactory plan from the standpoint of basin flushing.

Tidal Observations

40. None of the four plans (1, 3, 4B, and 5B) investigated extensively had any effects on base condition tidal heights or phasing, as shown in Plates 4-9. Data presented in these plates show that all plan test results were well within the limits of accuracy of repeating identical model tests.

Current Observations

41. Hourly current velocity measurements resulting from tests conducted with plans 1, 3, 4B, and 5B installed in the model are shown in Plates 10-76. Results of flow predominance calculations are shown in Table 2 and in Figures 4-9. Surface current patterns for base and plan conditions are shown in Photos 1-25.

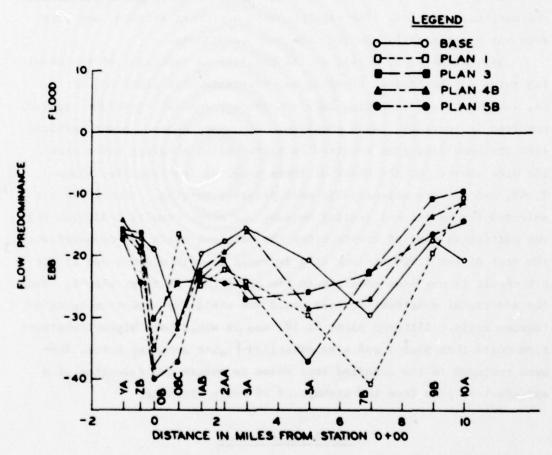


Figure 4. Surface flow predominance, channel center-line stations

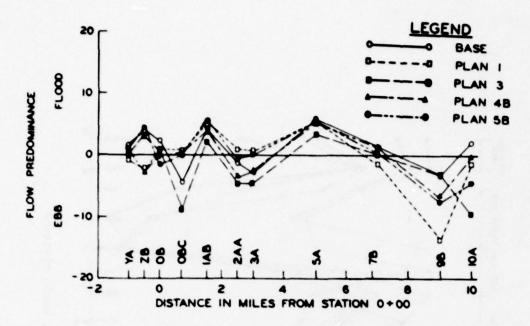


Figure 5. Middepth flow predominance, channel center-line stations

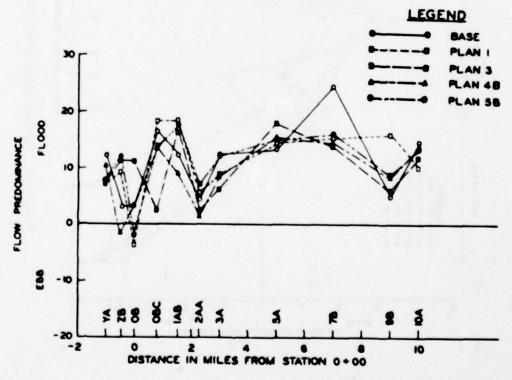


Figure 6. Bottom flow predominance, channel center-line stations

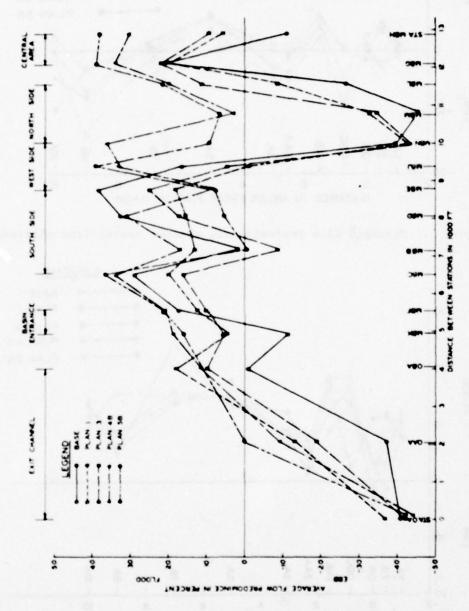


Figure 7. Surface flow predominance, basin stations

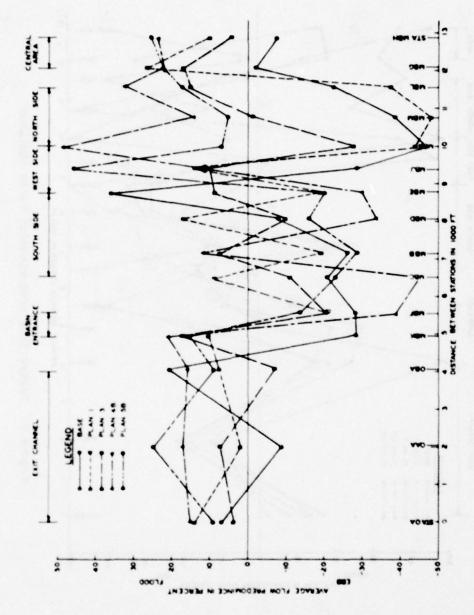


Figure 8. Middepth flow predominance, basin stations

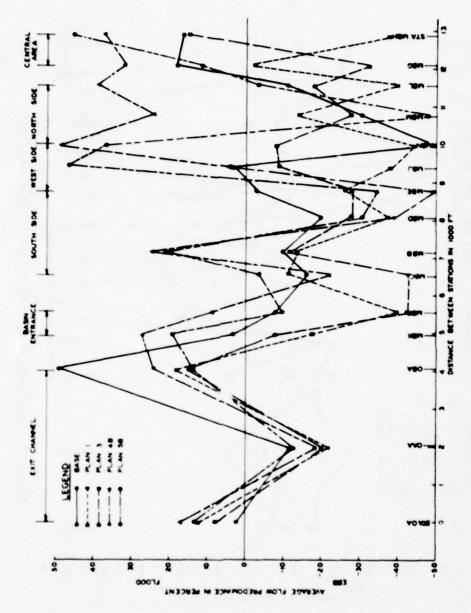


Figure 9. Bottom flow predominance, basin stations

- 42. None of the four plans resulted in any significant changes in maximum currents measured along the navigation channel except in the immediate vicinity of the plans (sta OB, OBB, OBC, 1AA, 1AB, and 1AC). Likewise, no significant changes were observed in time of slack waters or flow predominance in the channel other than in the immediate vicinity of the plans. The greatest effects on maximum currents in the navigation channel occurred during ebb flows and were observed at sta OB (Plates 15 and 48) and OBC (Plates 19 and 52), located on the center line of the navigation channel opposite the plans. Effects were the greatest at sta OB. Plan 1 resulted in the greatest change at sta OB, with increases in maximum ebb currents of 1.3, 1.6, and 2.0 fps, surface, middepth, and bottom, respectively. Plan 1 caused maximum surface currents at sta OB to increase from 2.8 (base) to 4.1 fps, while maximum middepth currents increased from a base condition value of 2.3 to 3.9 fps. Bottom depth maximum currents increased from 1.7 to 3.7 fps. Plan 1 caused a slight increase in flood currents at this location, but not to the degree observed during ebb flows. The greatest effect with plan 5B was likewise observed at sta OB, surface depth, at which point the maximum ebb current was increased from 2.8 to 4.6 fps. Range 1A was adjacent to the river end of the secondary opening of plans 4B and 5B, and surface flood velocities were increased at the closest station (sta 1AA, Plate 53) by about 1.0 fps but decreased in the center of the channel (sta 1AB, Plate 54) and on the opposite side of the river (sta IAC, Plate 55) by about 1.0 fps. Plans 3, 4B, and 5B resulted in increased maximum ebb currents in the navigation channel adjacent to the basin on the order of 0.5 to 1.5 fps. Small changes were observed at other channel stations but were generally within the limits of accuracy of repeating identical tests.
- 43. Flow predominance values shown in profile form for stations located along the navigation channel center line are shown in Figures 4-6. There were no reversals of predominant flow direction except when base and plan values were within the range of +5 percent. Since this range is essentially balanced flow, the changes in direction are insignificant. A relatively large increase in surface ebb predominance

(10 to 20 percent) was noted at each of the channel stations located near the mouth of the basin exit channel and proposed dikes (sta 2B, OB, and OBC). Several rather large increases (10 to 15 percent) in surface ebb predominance can be seen at sta 3A, 5A, and 7A (Figure 4). In general, the plans have minimal influence on flow predominance in the navigation channel.

- 44. Hourly current velocity measurements collected within the basin (sta MBA-MBN) with plans 1 and 3 (see Figures 2a and 2c for locations) installed are shown in Plates 30-42, while plans 4B and 5B (see Figure 2f for locations) hourly velocity data for the same stations are shown in Plates 63-76. Current velocities at locations within the basin were too low to measure using the current meter, therefore velocity magnitude had to be determined with floats and dye, as described in Report 1. Velocity measurements in the basin exit channel (sta OA, OAA, and OBA) are presented in Plates 14, 16, and 17 for plans 1 and 3 and in Plates 47, 49, and 50 for plans 4B and 5B.
- 45. Except at the surface of sta OAA and flood velocities at sta OBA for plans 4B and 5B, velocity changes in the exit channel were rather small for all four plans. Maximum ebb and flood velocities generally were changed on the order of +0.5 fps or less. At sta OAA (Plates 16 and 49), however, the surface current phasing was completely reversed and the maximum surface ebb velocity was reduced by about 1 to 2 fps. This station was located immediately adjacent to the downstream end of the proposed dike. During the base test, surface flow at sta OAA was dominated by main channel flow; however, in the plan tests, the station was protected from main channel flow by the dike and was dominated by flow to and from the basin. Examination of velocity measurements at sta OBA (Plates 17 and 50) indicates that the surface and bottom waters for both base and plan are out of phase. During the early phase of the rising tide (hours 2 to 6), the basin is being filled from the surface of the exit channel; whereas during the final phase of the rising tide (hours 6 to 8), the basin fills from the bottom. During the falling tide, the basin empties from the surface but continues to be fed water from the bottom of the exit channel; thus, suspended sediments can be

transported into the basin throughout the entire tidal cycle. Maximum flood velocities at sta OBA were increased by 0.5 to 1.0 fps for plans 4B and 5B.

46. Neither plan 1 nor 3 resulted in any significant increases in maximum current velocities at stations located within the basin, as in each case, base and plans, no velocities were measured in excess of 1.5 fps. The maximum velocity of 1.5 fps was observed at sta MBF (Plate 35) at the surface depth with base conditions during a flooding tide. Plans 1 and 3 maximum velocities at this location and time were 1.4 and 1.2 fps, respectively. Maximum ebb currents in the basin were generally much less than maximum flood currents. In general, maximum velocities were about 0.5 to 0.8 fps for plans 1 and 3 and the base test.

47. Plans 4B and 5B (plans with a gate in the secondary channel open only during flood tide, hours 4.5 to 10.25) had a greater effect than either plans 1 or 3 on maximum current velocities inside the basin. Maximum effects on maximum currents were observed during flooding conditions at four stations: MBF, MBK, and MBL (Plates 68, 72, and 73), each located in or near the basin entrance, and at sta MBM (Plate 74), located alongside the carrier docking area. Maximum ebb currents at the above stations were in almost all cases slightly lower than those observed during base conditions. Maximum flood currents observed at the four stations discussed above with plans 4B and 5B installed are shown in the following tabulation.

				Velo	city, f	ps .			
	Base			P	Plan 4B		Plan 5B		
Station	Sur- Mid- Bot- face depth tom	Sur- face	Mid- depth	Bot- tom	Sur- face	Mid- depth	Bot-		
MBF	1.5	0.5	0.5	1.8	0.4	1.2	1.3	0.7	1.3
MBK	0.8	0.3	1.2	1.8	1.3	1.6	1.2	1.8	1.2
MBL	0.7	0.3	0.5	2.0	1.4	1.3	1.3	0.7	0.9
MBM	0.2	0.2	0.6	1.2	1.2	0.9	0.9	0.6	0.0

The greatest current velocity (2.0 fps) anywhere in the basin was observed at sta MBL, surface depth, with plan 4B installed; however,

maximum surface flood currents at the other three locations shown above with plan 4B installed were very close to the maximum velocity observed at sta MBL. Essentially, all middepth and, to a lesser extent, bottom maximum currents at these four stations were increased as a result of both plans 4B and 5B. These increases, generally less than those for surface currents, probably are significant enough to adversely influence berthing requirements in the area.

- 48. Changes in maximum currents at other locations were generally very small. The only other station that was affected to any significant degree was sta MBN (Plate 75), located near the secondary opening. Maximum flood currents at this location were increased about 0.5 to 0.8 fps, whereas ebb currents were essentially unchanged.
- 49. Maximum currents measured at sta MBP (located in secondary opening, see Figure 2f for location and Plate 76 for data) were generally about 2.4 to 2.7 fps at each depth for plans 4B and 5B. These were flood currents only, as the openings were closed during ebb flow. These velocities are sufficient to cause some scouring; therefore the bed of the secondary channel might have to be protected by some means. Effects of the opening on navigation conditions in the channel with either plan would be insignificant as indicated by the surface current patterns shown in Photos 16-25.
- 50. Flow predominance calculations for stations located in the basin and exit channel are shown in Table 2 and in Figures 7-9. It should be emphasized at this point that due to varying flow direction in the basin in combination with the very low velocities and method of data collection (floats and dye), the degree of accuracy for current velocity measurements in the basin is not as good as in the main channel. In order to show flow predominance in profile form for the exit channel and basin stations, a pattern or route was established. The starting point was at sta OA, located in the exit channel, and continued into the basin in a clockwise direction. Neither sta MBA (in the slip) nor sta MBP (in the secondary opening of plans 4B and 5B) is included in these figures. Interpolation between points shown in Figures 7-9 only should be done considering the location of each station.

- 51. Table 2 and Figures 7, 8, and 9 present flow predominance for the surface, middepth, and bottom, respectively, and show that each of the four plans resulted in changes to existing flow predominance in the basin itself and in the exit channel. Analysis of these data shows considerable differences in effect of the plans from surface to bottom. Trends can be observed if the data are separated or grouped to show effects in individual areas of the basin.
- 52. In general, the two plans consisting only of training dikes (plans 1 and 3) exhibited similar changes to surface, middepth, and bottom flow predominances. In the exit channel to the basin, minimal changes resulted at the outer end (sta OA). A significant reduction in ebb predominance at the surface, a change from slight ebb to slight flood predominance at middepth, and a slight increase in ebb predominance were measured in the midportion (sta OAA). A change from balanced flow to flood predominance at the surface, reduction of flood to slight flood for plan 1 and slight ebb predominance for plan 3 at middepth, and marked reduction from essentially total flood to moderate flood predominance at the bottom for the interior portion (sta OBA) occurred. These changes at the mouth of the basin (sta OBA), combined with the minimal changes in maximum velocities (Plate 17) that were observed, would have the effect of a moderate increase in inward transport at the surface, a reduced moderate inward transport for plan 1 and change to slight outward transport for plan 3 in the middepth, and a marked decrease in inward transport on the bottom. Although consideration must be given to the phase difference in velocity at the various depths and the settling velocity of the material, the general trend should result in an overall reduction in material brought to the entrance to the basin for plans 1 and 3.
- 53. Immediately inside the basin, a change from moderate ebb to slight flood predominance on the north side (sta MBK) and a minimal reduction of the moderate flood predominance on the south side (sta MBF) at the surface were observed. Change from strong ebb to moderate flood predominance on the north side and a slight decrease in strong ebb predominance on the south side were measured at middepth. A marked change

from slight flood on the north side and slight ebb on the south side to strong ebb predominance occurred at the bottom. These changes combined with the observed changes in maximum velocities should result in minimal changes to net flood transport at the surface, significant reduction of ebb transport at middepth, and marked increase to strong ebb transport at the bottom immediately inside the entrance to the basin. The overall change should be a reduction of material brought into the basin with a considerable improvement in flushing near the bottom for plans 1 and 3.

- 54. Inside the basin in the southern portion (sta MBC, MBB, MBD, and MBE), essentially all locations showed a decrease in the flood predominance presently existing at the surface. At middepth, a reduction in ebb predominance or change to flood predominance occurred from the generally ebb predominance presently existing. The one exception is in the southwest corner (sta MBE) where an existing flood predominance is changed to slight flood and moderate ebb predominance for plans 3 and 1, respectively. Near the bottom, both plans result in an increase in the ebb predominance presently existing. Conclusions for the southern portion are difficult to reach.
- 55. In the northern portion (sta MBN, MBM, and MBL), existing strong ebb predominance was generally reduced minimally by plans 1 and 3.
- 56. In the central portion (sta MBG, MBH, and MBJ), relatively minimum changes to flood predominance at the surface, changes from slight or strong ebb predominance to moderate flood predominance at middepth, and generally changes from moderate or slight flood to moderate or strong ebb predominance at the bottom occurred.
- 57. As shown for plans 1 and 3, plans 48 and 58 (with a secondary opening into the basin) exhibited similar results. In general, the results are most similar for all four plans tested in the exit channel and become progressively dissimilar nearer the location of the secondary opening.
- 58. In the exit channel, surface and bottom flow predominances were similar for all plans. At middepth, plans 4B and 5B tended to develop a stronger flood predominance than plans 1 and 3. In general,

conclusions applicable to plans 1 and 3 are applicable to plans 4B and 5B; however, because of the higher flood velocities associated with plans 4B and 5B as compared with plans 1 and 3 particularly at the basin end of the channel (Plates 17 and 50), plans 4B and 5B should cause a higher transport of material to the basin.

- 59. Immediately inside the basin, the surface predominance changes from slight ebb to moderate flood on the north side (sta MBK) and changes minimally from the moderate flood on the south side (sta MBF). At middepth, the north side changes from strong ebb to moderate flood predominance and the predominance increases and decreases from the strong ebb on the south side for plans 4B and 5B, respectively. The predominance changes for the bottom are an increase from slight flood to strong flood on the north side, and on the south side no change from the slight ebb for plan 5B and change to moderate flood for plan 4B. In general, the flow predominance results would indicate an increase in the material transported to the basin when compared with existing conditions.
- 60. The flow predominance for the southern portion of the basin (sta MBC, MBB, MBD, and MBE) is relatively complex; however, the general trend is for plans 4B and 5B to develop more of a surface flood predominance trend than plans 1 and 3. At middepth, plans 4B and 5B caused stronger ebb predominance than plans 1 and 3 while on the bottom no general trend is evident. The comparison with existing conditions is not nearly as clear.
- 61. At most locations in the central and northern portion of the basin (sta MBN, MBM, MBL, MBG, MBH, and MBJ), surface, middepth, and bottom clearly were changed to flood-predominant when compared with plans 1 and 3 and particularly existing conditions. The only exceptions were on the bottom for plan 4B where sta MBM was changed to essentially all ebb flow and sta MBL was changed to essentially balanced flow. Plans 4B and 5B resulted in marked increases in flood velocities through the area and out through the secondary opening. Conclusions on material transport through the area are dependent strongly on the amount of material in suspension during the flood phase; however, the results of the

flow predominance computations indicate a significant transport of material out of the basin in this area.

62. The following tabulation summarizes the effects of the plans on average flow predominance in the basin. The values shown were obtained by averaging results at 12 locations within the basin. Sta MBA, located in the extreme end of the slip, was excluded.

Average Flow Predominance Depth Base Plan 3 Plan 4B Plan 5B Plan 1 +2.9 +2.0 +2.3 +20.3 Surface +24.2 Middepth -18.1-8.0 +1.1 +3.5 +3.7 -8.6 -27.0 -19.8+9.8 Bottom +4.5

Average -8.2 -10.9 -5.3 +12.5 +9.5

NOTE: Minus sign (-) denotes ebb predominance; plus sign (+) denotes flood predominance.

63. Based on the results of velocity measurements and flow predominance calculation, plans 1 and 3 appear to reduce the potential for shoaling in the basin due to an apparent reduction in potential for introduction of material to the basin at the entrance. The results of plans 4B and 5B are not nearly as conclusive. The potential to increase materials introduced to the basin exists; however, the transport of material out of the basin via the secondary opening could more than offset this increase. The velocity data, flow predominance data, and dye results combined with additional information on the characteristics of the material not presently available could provide a basis for a much more rigorous analysis of the shoaling of the basin. Such an analysis is beyond the scope of this investigation.

Salinities

64. The effects of plans 1 and 3, 4B and 5B on hourly salinity concentrations at locations throughout the model over a complete tidal cycle are shown in Plates 77-122, and 123-169, respectively. Plan

effects on salinity values averaged over a tidal cycle are shown in Table 3 and in Figures 10 and 11.

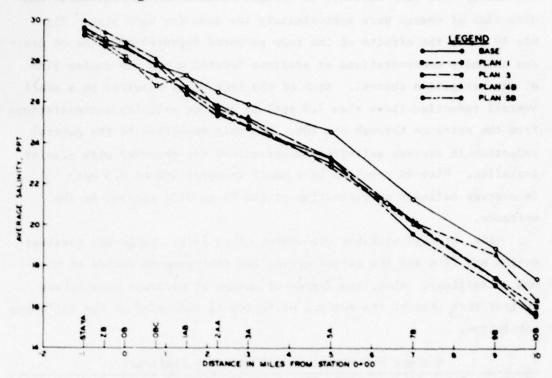


Figure 10. Average salinity profile, navigation channel center-line stations

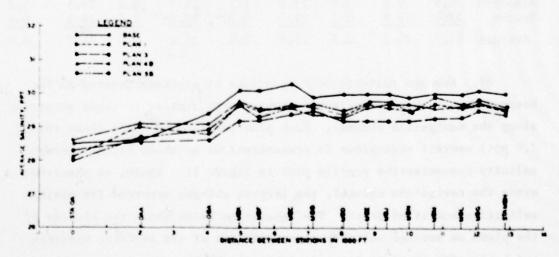


Figure 11. Average salinity profile, Mayport Basin

- 65. Effects of all proposed improvement plans on base condition salinities were very similar, as changes in salinity concentration and direction of change were approximately the same for each plan. Figure 10 shows the effects of the four proposed improvement plans on average salinity concentrations at stations located along the center line of the navigation channel. Each of the four plans resulted in a small overall reduction (less than 2.0 ppt) to average salinity concentrations from the entrance through sta 10A. The only exception to the general reduction in average salinity concentrations was observed with plan 4B installed. Plan 4B resulted in a small increase (about 0.3 ppt) in average salinity concentration at sta YA to OBC, located in the entrance.
- 66. At most stations the degree of salinity change was greatest during ebb flow and the period around lws (the general period of minimum salinities). Also, the degree of change at middepth usually was greater than that at the surface or bottom as indicated in the following tabulation.

Average Salinities, ppt (Channel C Stations)

Depth	Base	Plan 1	Diff	Plan 3	Diff	Plan 4B	Diff	Plan 5B	Diff
Surface	21.1	20.6	0.5	21.0	0.1	21.0	0.1	19.9	1.2
Middepth	26.8	25.9	0.9	25.7	1.1	26.3	0.5	25.5	1.3
Bottom	29.0	28.8	0.2	28.2	0.8	28.8	0.2	28.6	0.4
Average	25.6	25.1	0.5	25.0	0.6	25.4	0.2	24.7	0.9

67. Average salinity concentrations at stations located in the basin and exit channel reflected changes very similar to those occurring along the navigation channel. Each plan resulted in small (less than 2.0 ppt) overall reductions in concentrations as shown by the average salinity concentration profile plot in Figure 11. Again, as observed along the navigation channel, the largest changes occurred for minimum salinities and at middepth. The tabulation below shows the effects of the plans on average salinity concentrations at the surface, middepth, and bottom for stations located within the basin.

	Average	Salinity,	ppt (Basin	Stations)	Link this The
Depth	Base	Plan 1	Plan 3	Plan 4B	Plan 5B
Surface	24.5	24.3	23.7	24.0	24.7
Middepth	31.0	30.5	30.5	30.6	30.2
Bottom	32.4	32.2	31.9	32.3	32.1
Average	29.3	29.0	28.7	29.0	29.0

68. The effects of the plans on average salinities are generally less than 1.0 ppt, and many results are within or reasonably close to the limits of accuracy in repeating identical tests of this type. The influences of the plans on salinities are small, but the model observations consistently show that all of the plans would reduce average salinity by 0.5 to 2.0 ppt throughout at least the lower 10 miles of the estuary, including the Mayport Basin.

Dye

- 69. The effects of plans 1 and 3 on hws and lws dye samples at locations throughout the basin and estuary over the entire 16-cycle test period are shown in Plates 170-201 and 202-233, respectively. Similar data collected with improvement plans 4B and 5B installed in the model are shown in Plates 234-266 and 267-299 for hws and lws periods, respectively. Dye data used to prepare the above plates are also shown in tabular form in Tables 4 and 5, hws and lws samples, respectively.
- 70. Although all data obtained during the dye tests are presented in the figures and tables, data obtained at tidal cycle 6 and later are not considered valid. In the course of conducting the test, dye is introduced into the ocean during ebb flow. Portions of this dye reach the sump by the second tidal cycle through the mixing weirs skimming water from the surface of the ocean. Inspection of the results and observations of the model indicate that this dye was not reintroduced into the study area until approximately the sixth tidal cycle.
- 71. In addition, the plume of dyed water that was introduced into the ocean during the initial ebb flow circulates in the ocean in a counterclockwise pattern and is reintroduced into the entrance of the

river during tidal cycle 6. Both of the occurrences result in conditions that are not considered to be representative of what would occur in the prototype.

72. Results of the dye test during the first five tidal cycles were evaluated in several ways. Comparisons were made of the base and plan results in the main navigation channel as well as within the basin. None of these comparisons resulted in any definite trends with one exception. The data within the basin were averaged for all stations at each tidal cycle; results are presented in Table 6 and Figures 12-15. In comparing the data prism with the sixth tidal cycle, none of the data for plans 1 and 3 show any improvement in flushing; the data for plans 4B and 5B do show an improvement in flushing. The greatest improvement in flushing occurs for data taken near the bottom. Dye concentrations for the fifth tidal cycle at hws are reduced from 62 ppb to 12 ppb for plan 4B and to 34 ppb for plan 5B. At lws reductions are from 76 ppb to 13 ppb for plan 4B and to 45 ppb for plan 5B. Lesser reduction occurred at shallower depths with no reduction occurring near the surface for hws data and small reductions for the lws quarterdepth and middepth.

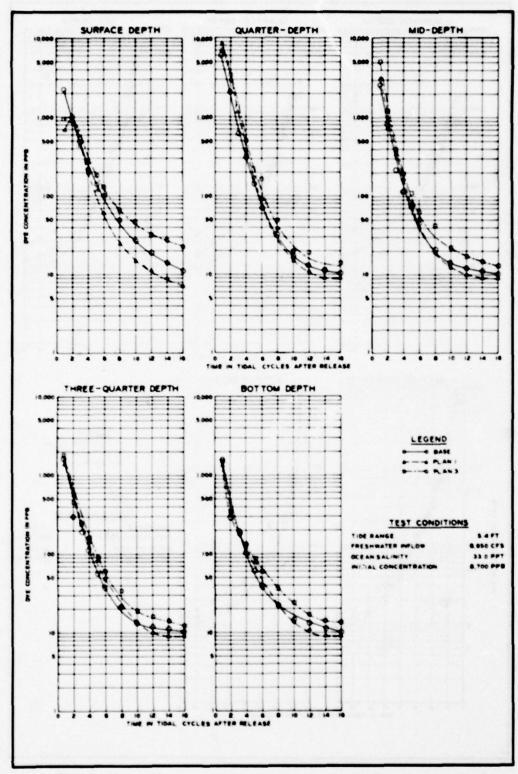


Figure 12. Effects of plans 1 and 3 on average high-water slack dye concentrations, basin stations

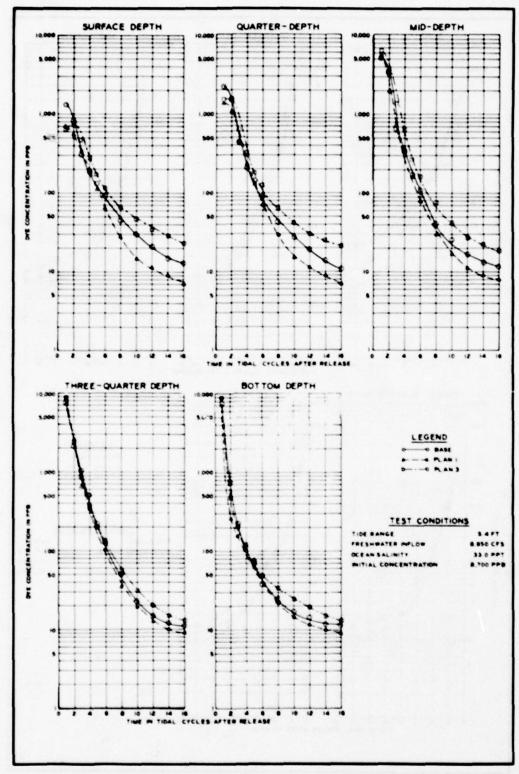


Figure 13. Effects of plans 1 and 3 on average low-water slack dye concentrations, basin stations

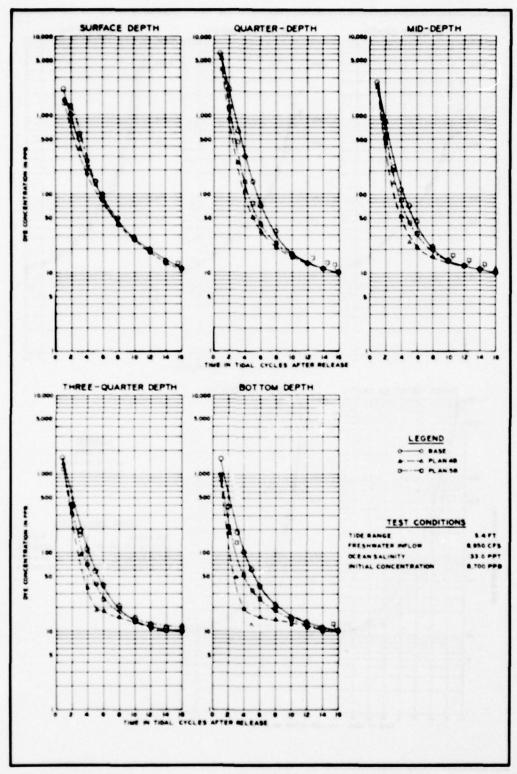


Figure 14. Effects of plans 4B and 5B on average high-water slack dye concentrations, basin stations

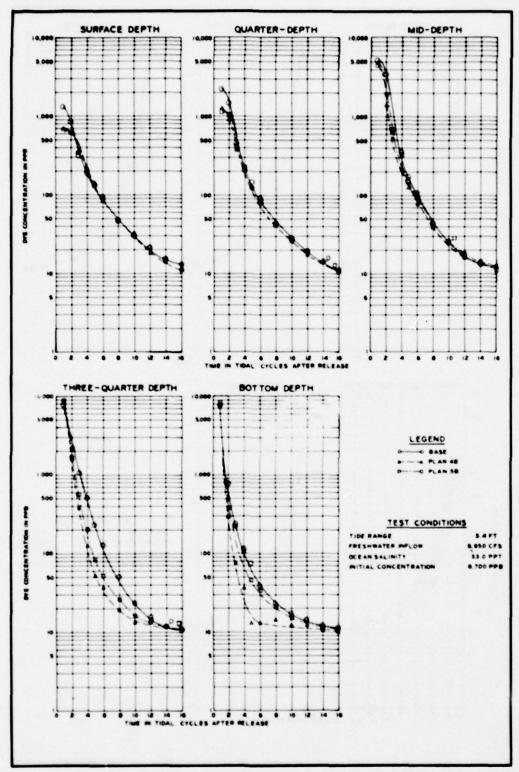


Figure 15. Effects of plans 4B and 5B on average low-water slack dye concentrations, basin stations

PART IV: CONCLUSIONS AND RECOMMENDATIONS

Conclusions

- 73. On the basis of model shoaling test results, 4 of the 11 plans tested to determine effects on basin shooling were selected for detailed testing to determine their effect on hydraulics, salinity, and dye flushing in the basin area. Those plans selected were: plan 1, approximately 1900-ft extension of Wards Bank training wall; plan 3, enclosed area with dikes parallel to exit channel and navigation channel; plan 4B, gated opening 200 ft by 42 ft in combination with plan 1; and plan 5B, gated opening 200 ft by 21 ft in combination with plan 1.
- 74. On the basis of model test results discussed in previous paragraphs in this report, it is concluded that none of the four plans investigated extensively would have any effect on existing tidal heights, tide range, or phase through the estuary. Effects on current velocities or patterns and flow predominance throughout the estuary other than in the basin and immediate vicinity of the basin were generally very small. Each of the four plans resulted in a decrease in average salinity of 0.5 to 2.0 ppt along the navigation channel. Shoaling in the navigation channel was not affected by any of the plans.
- 75. Although effects in the main estuary and navigation channel were generally small and insignificant, there were considerable effects in the basin and immediate surrounding area. In respect to maximum current velocities within the basin, plans 1 and 3 had the least effect and would not result in causing berthing problems for the carriers. Plan 4B had the greatest effect on maximum current velocities, resulting in flood current in the area of the carrier berth of about 2.0 fps, which could cause berthing problems should a carrier approach this area during this phase of the tide. Maximum current velocities in the navigation channel were increased with each of the four plans but should present no problem to navigation.
- 76. Results of the flow predominance indicate that plans 1 and 3 should result in an overall reduction of material transported to

the entrance of the basin. Plans 4B and 5B should also result in an overall reduction of material transported to the entrance of the basin; however, because of higher flood velocities, the reduction should be less than that for plans 1 and 3.

- 77. Immediately inside the basin, flow predominance changes for plans 1 and 3 indicate a reduction of material transported into the basin. Results for plans 4B and 5B indicate an increase in material transport into the basin.
- 78. Within the basin, flow predominance changes occurred. The most significant change occurred for plans 4B and 5B near the secondary opening. Results indicate that material in suspension in the area will be transported out of the basin.
- 79. Based on the results of velocity measurements and flow predominance calculation, plans 1 and 3 appear to reduce the potential for shoaling in the basin due to an apparent reduction in potential for introduction of material to the basin at the entrance. Results of plans 48 and 58 are not nearly as conclusive. The potential to increase materials introduced to the basin exists; however, the transport of material out of the basin via the secondary opening could more than offset this increase.
- 80. Average salinity concentrations in the navigation channel, exit channel, and basin were generally slightly lower than the base for each of the four plans investigated. There was no indication that one plan resulted in more or less change than another, as a very large percentage of the data was within the limits of accuracy of repeating identical model tests, particularly within the basin.
- 81. Results of the dye tests do not indicate any significant improvement of flushing for plans 1 and 3. Results for plans 4B and 5B indicate a significant improvement in overall flushing within the basin near the bottom. Lesser reductions were observed at shallower depths with essentially no change near the surface.
- 82. Plans 1, 2, and 3 each proved to be very effective in reducing overall shoaling rates in the exit channel and basin. All plans involving the ungated secondary opening (plans 4, 4A, 4C, 5, 5A, and 5C)

resulted in increased shoaling in the basin. Plans involving gated secondary openings (4B and 5B) reduced the overall shoaling rate but not to the degree of plans 1, 2, and 3.

Recommendations

83. On the basis of model tests described herein, it is recommended that plan 1 be installed in the prototype. Plan 1 was the best plan with respect to shoaling reduction, which is the primary objective of the study.

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	1.0	2,632	-	1,195	23.0	1.405	13.4	1.3%	60.3	2,313	100.0	1,745	1.84.3	3360	136.6	1850	14.6	3,393	141.2	3,000	128.2	3,521	6.41	1,740	72.0	2,845	
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		127			67.73		****	****	****		*11.3	(**)	3.3	1	47.0	100		0.0		1,570	25.5	1,180	16.9	96.	13.6		

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Notice to become recovered.

Southed to the total amount of material recovered for a plan test divided by the total amount of material recovered for the base test.

Southed total amount of calculate percent change for plans 1, 2, and 3, and 2, 3, 34, 39, and 30 (plans involving second opening).

Base test 2 (with size installed) was used to calculate percent change for plans 1, 3, 40, 5, 34, 39, and 30 (plans involving second opening).

Shouther material used for all basis tests was disconts with a specific gravity of 1,05.

Tall channel shouther were conducted without hips installed in the basis. Shouther material used was plantic with a specific gravity of 1,05.

	100	Book -	100			Boal	100	- Breeze	98	-	2		2		125	١.	25		75		2		21	-100
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	10	9.1	13		0	0.0		0.0	0	0.0												0.0	0	0.0
***		4 4				0.0	0	0.0	0	0.0												0.0	0	0.0
500		0.1	122		0	0.0	120	0	110	1.6												9.4	0	0.0
*	***	* *			***	* *	04.6		318														270	8.0
**	70	***	2.		200	***	200		24.7	***													8	
20	0 1	0.0	0 1		200			2.4	2 1															
13	8	2.0	607		2342	3.9		2.3	203	2.3														
**	0	0.0	70		0	0.0	0	9.0	0	5.0													2	
2	70	2.7	0		65	0.0	a	0.0	e	0.0												2,0	>	0.0
316	135	0.2	0		0	0.0	0	0.0	0	0.0												0.0	0	0.0
Total	6.98	100.0	6.347	8.0	1,463	101.2	1,390	103.1	6.393	120.2	1,073	102.3	6,405	1.1	7,470	1 6.901	1,230 1	103.5 6	6,315	13.3	6,600	97.3	6,630	
												1	81											
	1	* *	63			0 0		0.0																0.0
24	630		::		101	0.1	10	0.10																1.1
32	100		820		1.136	10.6	843	8.0																5.3
NO	5.	0.2	1 10		2500	2.1	5775	5.5																2.2
11	13	0.1	300		130	1.1	173	3.9															_	0.3
2	2,613	24.4	2,170		3.750	28.2	3,000	19.0																13.6
11	1,215	11.6	1,160		1,000	9.7	2,400	13.3															-	9.77
*	18	1.3	180		569	6.6	24	0.4									-							19.6
35	630	6.0	555		1.240	11.8	1,510	14.3																10.2
*	1.405	13.4	2,010		1.430	13.6	1.760	1.6.7																2.0
11	2,043	19.4	1.700		1,630	13.3	1,963	27.72									-						_	20.0
R	653	6.2	730	1.1	850	9.1	089	6.5	1,50	4.3	\$1.5	4.9	360	5.3	180	1.1	510	6.9	740	1.0	120	6.8	1,110	10.5
Three	10,530	100.0	10,185		11,160	106.9	11,100	103.4						-								-		8.2

Table 2
Effects of Plans on Flow Predominance

				Plan		
Station	Depth	Base	1	_3_	14B	_ 5B
		Channel	Stations			
YA	Surface	-16.5	-16.7	-15.7	-14.2	-17.
	Middepth	1.8	-0.8	-0.4	0.4	0.
	Bottom	12.2	7.3	7.8	10.5	7.
ZA	Surface	-26.4	-17.4	-21.7	-18.7	-27.
	Middepth	-0.5	3.3	-4.1	4.5	2.
	Bottom	10.3	4.7	6.6	6.3	8.
ZB	Surface	-17.1	-19.3	-16.1	-18.6	-24
	Middepth	4.1	-2.2	3.3	-2.7	14
	Bottom	3.1	9.3	11.4	-1.7	12
ZC	Surface	-29.8	-26.9	-24.7	-22.7	-35
	Middepth	-13.4	-9.7	-9.8	-12.3	-14
	Bottom	-1.0	1.9	7.5	1.9	3
OB	Surface	-18.9	-35.2	-30.2	-38.2	-40
	Middepth	2.3	0.8	0.1	0.9	-1
	Bottom	3.1	-3.5	11.1	3.5	-2
OBB	Surface	-24.2	-44.3	-25.7	-38.6	-46
	Middepth	-10.9	-21.5	-9.2	-12.2	-20
	Bottom	0.4	-4.8	-8.8	0.3	2
OBC	Surface	-31.0	-16.6	-24.5	-31.9	-37
	Middepth	-4.4	0.6	-8.8	0.1	0
	Bottom	16.3	18.3	2.5	13.8	13
1AA	Surface	-25.2	-26.0	-24.5	-17.1	-18
	Middepth	-2.2	0.9	-1.9	-2.0	-2
	Bottom	9.3	10.9	11.2	2.1	15
1AB	Surface	-19.7	-24.8	-23.9	-22.1	-23
	Middepth	4.3	5.4	2.1	4.0	5
	Bottom	12.3	18.4	16.3	9.0	17
1AC	Surface	-19.6	-31.0	-24.5	-27.2	-27
	Middepth	-4.4	2.4	-2.7	0.9	0
	Bottom	7.6	7.7	6.9	7.7	14
2AA	Surface	-17.7	-22.2	-20.3	-19.3	-24
	Middepth	-1.2	-1.0	-4.7	-3.6	-0
	Bottom	4.5	5.6	2.4	1.7	7
3A	Surface	-15.3	-24.2	-27.0	-15.7	-25
	Middepth	-2.9	0.7	-4.5	-2.9	0
	Bottom	12.4	9.0	6.2	8.7	12
		(Cont	inued)			

A negative sign (-) denotes flow predominance in the ebb direction; values without a sign are positive values and denote flow predominance in the flood direction.

(Sheet 1 of 3)

Table 2 (Continued)

				Plan		
Station	Depth	Base	_1	3	4B	5B
	Cha	nnel Statio	ons (Conti	nued)		
5A	Surface	-20.7	-29.6	-25.9	-28.5	-37.6
	Middepth	5.9	5.5	3.6	5.4	5.2
	Bottom	13.3	14.5	17.9	15.6	-15.
7B	Surface	-29.8	-30.8	-22.9	-27.0	-22.
	Middepth	1.2	-1.6	0.3	1.2	0.
	Bottom	24.4	15.3	14.0	14.3	15.
9B	Surface	-17.3	-8.2	-10.6	-13.6	-16.
	Middepth	-3.0	-13.4	-3.1	-6.4	-7.
	Bottom	4.9	15.9	6.0	8.4	8.
9AB	Surface	-21.1	-19.6	-21.0	-17.7	-20.
	Middepth	-6.2	-3.0	-4.1	-6.1	-2.
	Bottom	33.5	25.5	28.0	23.4	26.
10A	Surface	-20.7	-10.3	-9.6	-11.1	-14.
	Middepth	2.1	-1.5	2.8	-0.2	-4.
	Bottom	14.4	10.0	11.8	14.0	13.
		Basin	Stations			
OA	Surface	-40.7	-36.5	-41.9	-46.0	-44.
	Middepth	7.7	14.6	4.3	15.4	9.
	Bottom	2.7	12.9	8.0	17.3	13.
OAA	Surface	-37.0	-12.4	-18.3	-8.8	0.
	Middepth	-8.8	2.4	7.8	17.6	25.
	Bottom	-11.4	-21.6	-20.4	-18.7	-12.
OBA	Surface	-0.2	18.7	10.5	12.0	11.
	Middepth	21.3	8.1	-6.3	16.5	9.
	Bottom	49.6	19.2	15.3	24.8	14.
MBA	Surface	Slack	-15.4	-3.8	-18.1	-24.
	Middepth	Slack	-28.4	6.5	-16.1	16.
	Bottom	Slack	-22.3	-36.9	-28.0	13.
MBB	Surface	1.3	13.4	-8.9	17.1	-0.
	Middepth	-25.8	-19.0	8.0	12.4	-28.
	Bottom	-9.3	-13.1	-11.0	25.5	21.
MBC	Surface	29.2	20.5	16.3	34.7	37.
	Middepth	-20.3	9.2	-10.6	-45.1	-22.
	Bottom	-15.8	-10.5	-43.6	-22.6	-3.
		(Con	tinued)			

(Sheet 2 of 3)

Table 2 (Concluded)

				Plan		
Station	Depth	Base	1	_3_	4B	5B
	Ве	asin Station	s (Continu	ied)		
MBD	Surface	32.8	15.8	17.6	31.6	5.5
	Middepth Bottom	-7.6 -19.8	17.6 -37.6	-9.9 -27.7	-33.6 -38.6	-16.0 -30.5
MBE	Surface	38.9	18.5	25.2	8.8	8.7
	Middepth Bottom	36.5 -2.5	-17.7 -26.0	9.1	-29.5 -49.2	-20.0 -34.0
MBF	Surface	17.5	12.8	10.4	21.2	21.6
	Middepth Bottom	-27.9 -7.9	-20.6 -39.8	-20.5 -43.6	-38.4 11.6	-13.2 -9.1
MBG	Surface	21.0	20.1	21.6	34.3	. 39.1
	Middepth Bottom	-1.8 18.5	27.0	17.4	22.7	27.8
МВН	Surface	-10.6	6.3	9.4	30.5	38.2
	Middepth Bottom	-7.3 16.8	10.6	15.2	24.1 37.6	25.3 45.6
MBJ	Surface	16.9	-0.9	4.2	33.5	39.3
	Middepth Bottom	-28.1 3.4	14.7 -38.0	10.3	10.7	46.4
МВК	Surface	-11.1	5.0	5.3	19.3	16.6
	Middepth Bottom	-28.3 3.3	11.0 -21.1	17.4 -7.3	21.6	15.7
MBL	Surface	-26.7	-8.4	11.3	20.1	21.7
	Middepth Bottom	-22.1 -10.7	-37.3 -40.0	15.8	32.9 39.4	17.6 -3.0
МВМ	Surface	-45.3	-32.8	-34.7	3.0	6.8
	Middepth Bottom	-38.8 -29.2	-48.3 -13.5	-1.0 -27.8	14.6 24.3	5.5 -48.2
MBN	Surface	-39.6	-42.1	-42.4	36.4	9.5
	Middepth Bottom	-16.6 -50.0	-43.5 -44.2	-27.9 -6.8	49.3	7.1 36.9
МВР	Surface				49.8	49.7
	Middepth Bottom	=	=	=	44.5	49.7

Table 3

Effect of Plans on Average Salinities, ppt

				Plan		
Station	Depth	Base	1	_ 3_	4B	_5B
		Channel	Stations			
YA	Surface	26.0	25.9	25.8	26.6	25.
	Middepth	30.8	30.7	30.7	31.0	30.
	Bottom	32.0	32.2	31.8	32.1	32.
	Average	29.6	29.6	29.4	29.9	29.
ZA	Surface	25.4	25.6	25.8	26.1	25.
	Middepth	30.7	30.0	30.1	30.4	29.
	Bottom	31.8	31.7	31.6	31.7	31.
	Average	29.3	29.1	29.2	29.4	28.
ZB	Surface	24.9	24.6	25.1	25.7	24.
	Middepth	30.5	30.2	30.0	30.5	30.
	Bottom	32.0	31.8	31.6	31.9	31.
	Average	29.1	28.9	28.9	29.4	28.
ZC	Surface	25.0	25.0	25.5	26.2	24.
	Middepth	30.3	29.8	29.8	30.5	29.
	Bottom	31.8	31.5	31.4	31.8	31
	Average	29.0	28.8	28.9	29.5	28
ОВ	Surface	23.7	23.4	24.0	24.2	22.
	Middepth	30.4	29.5	30.2	30.3	29.
	Bottom	32.1	31.8	31.6	32.2	31.
	Average	28.7	28.2	28.6	28.9	28.
ОВВ	Surface	23.9	24.8	25.1	24.6	24.
	Middepth	28.4	29.7	27.7	28.4	27.
	Bottom	30.7	31.6	29.2	30.1	29.
	Average	27.7	28.7	27.3	27.7	27.
OBC	Surface	22.4	23.0	24.6	23.8	22.
	Middepth	29.5	26.7	28.5	29.0	28.
	Bottom	31.4	30.6	30.7	31.6	31.
	Average	27.8	26.8	27.9	28.1	27.

(Continued)

(Sheet 1 of 5)

Table 3 (Continued)

		100		Plan		
Station	Depth	Base	1	3	<u>4B</u>	5B
	Chan	nel Statio	ns (Contin	ued)		
1AA	Surface	21.8	21.8	22.8	21.8	20.5
	Middepth	29.0	28.3	27.5	28.6	27.8
	Bottom	30.1	30.4	29.5	29.9	30.2
	Average	27.0	26.8	26.6	26.8	26.2
1AB	Surface	22.2	21.7	21.9	20.9	20.7
	Middepth	29.4	29.4	27.6	28.5	28.5
	Bottom	30.3	30.8	29.6	30.6	30.5
	Average	27.3	27.3	26.4	26.7	26.6
lac	Surface	22.8	22.1	23.3	21.9	20.
	Middepth	28.0	27.7	27.5	27.8	27.
	Bottom	30.2	30.2	29.4	30.1	30.2
	Average	27.0	26.7	26.7	26.6	26.
ZAA	Surface	21.5	20.8	20.9	20.5	19.
	Middepth	27.8	26.5	26.2	27.2	26.
	Bottom	30.2	29.8	29.2	29.6	29.
	Average	26.5	25.7	25.4	25.8	25.
3A	Surface	20.4	19.8	19.8	19.7	19.
	Middepth	27.4	26.0	25.8	26.3	25.
	Bottom	29.8	29.4	28.7	29.4	29.
	Average	25.9	25.1	24.8	25.2	24.
5A	Surface	19.4	16.8	18.0	18.1	17.
	Middepth	26.1	24.5	24.0	24.7	23.
	Bottom	28.3	27.5	27.1	27.1	28.
	Average	24.6	22.9	23.0	23.3	23.
7B	Surface	16.8	16.4	16.0	16.9	14.
	Middepth	22.6	21.4	21.0	21.4	19.
	Bottom	24.6	24.5	24.0	24.4	23.
	Average	21.3	20.8	20.3	20.9	19.
9B	Surface	14.6	13.0	13.0	14.3	12.
	Middepth	19.0	17.5	17.4	18.4	16.
	Bottom	23.1	22.1	21.8	23.0	21.
	Average	18.9	17.5	17.4	18.6	17.
		(Cont	inued)			

(Sheet 2 of 5)

Table 3 (Continued)

				Plan		
tation	Depth	Base	_1_	3	4B	5B
	Chan	nel Statio	ns (Contin	ued)		
9AB	Surface	14.7	13.0	13.2	13.6	12.8
	Middepth	18.7	17.6	17.7	18.3	17.9
	Bottom	23.3	23.0	22.2	22.4	22.
	Average	18.9	17.9	17.7	18.1	17.
10A	Surface	13.5	12.6	11.8	12.4	11.
	Middepth	16.8	15.2	15.1	15.9	14.
	Bottom	21.8	20.6	20.2	21.0	20.
	Average	17.4	16.1	15.7	16.4	15.
		Basin S	tations			
OA	Surface	23.5	23.1	22.8	23.8	22.
	Middepth	29.8	29.4	30.2	30.1	29.
	Bottom	32.2	31.8	31.8	32.1	32.
	Average	28.5	28.1	28.3	28.6	28.
OAA	Surface	23.6	24.1	23.6	24.4	23.
	Middepth	30.2	30.7	30.4	30.7	30.
	Bottom	32.1	32.2	31.9	32.4	32.
	Average	28.6	29.0	28.6	29.1	28.
OBA	Surface	24.8	23.7	23.4	23.5	23.
	Middepth	31.2	30.6	30.7	31.4	30.
	Bottom	32.3	32.2	31.7	32.5	32.
	Average	29.4	28.8	28.6	29.1	28.
MBA	Surface	25.1	24.3	23.8	23.8	24.
	1/4	30.0	28.6	27.9	29.0	28.
	Middepth	31.1	30.4	30.1	30.5	30.
	3/4	32.2	31.6	31.5	31.8	31.
	Bottom	32.4	32.0	31.8	32.0	31.
	Average	30.2	29.4	29.0	29.4	29.
MBB	Surface	24.1	24.5	24.0	23.1	25.
	1/4	29.7	28.5	28.8	28.8	28.
	Middepth	31.1	30.6	30.9	30.8	30.
	3/4	32.2	32.0	31.9	31.9	31.
	Bottom	32.5	32.3	32.0	32.2	32.
	Average	29.9	29.6	29.5	29.4	29.
		(Cont	inued)			

(Sheet 3 of 5)

Table 3 (Continued)

				Plan		
tation	Depth	Base	_1_	3	<u>4B</u>	_5E
	Bas	in Station	s (Continu	ed)		
MBC	Surface	25.1	24.6	24.2	24.1	25.
	1/4	30.2	28.7	28.7	28.9	28.
	Middepth	31.3	30.7	30.5	30.7	30.
	3/4	32.4	32.0	31.7	32.1	31.
	Bottom	32.6	32.3	31.9	32.2	32.
				29.4	29.6	29.
	Average	30.3	29.7	29.4	29.6	29.
MBD	Surface	24.7	24.5	24.2	24.2	24
	1/4	29.2	28.0	28.1	28.2	28
	Middepth	31.1	30.4	30.3	30.2	30
	3/4	32.3	31.8	31.7	31.9	31
	Bottom	32.6	32.1	31.9	32.1	32
	Average	30.0	29.4	29.2	29.3	29
MBE	Surface	24.5	24.9	24.3	24.4	25
	1/4	29.9	28.7	28.3	28.8	28
	Middepth	30.9	30.4	30.0	30.7	30
	3/4	32.3	32.0	31.6	31.9	31
	Bottom	32.5	32.3	31.9	32.4	32
	Average	30.0	29.7	29.2	29.6	29
MBF	Surface	24.3	23.8	23.4	23.5	23
	1/4	29.7	28.5	28.6	29.0	28
	Middepth	31.6	30.8	30.7	30.8	30
	3/4	32.2	32.1	31.7	31.9	31
	Bottom	32.5	32.3	31.7	32.3	32
	Average	30.1	29.5	29.2	29.5	29
MBG	Surface	24.9	24.7	23.8	24.4	26
	1/4	29.3	28.4	28.6	28.6	28
	Middepth	31.5	30.9	31.0	30.9	30
	3/4	32.4	32.1	31.9	32.0	31
	Bottom	32.5	32.4	32.1	32.4	32
	Average	30.1	29.7	29.5	29.7	29
МВН	Surface	24.9	24.4	24.2	23.5	25
	1/4	29.3	28.4	28.3	28.6	28
	Middepth	31.1	30.4	30.6	30.8	30
	3/4	32.4	31.8	31.9	32.0	31
	Bottom	32.4	32.3	32.2	32.4	32
	Average	30.0	29.5	29.4	29.5	29
		10	inued			

Table 3 (Concluded)

				Plan		
Station	Depth	Base	1	3	4B	_5B
	Bas	n Station	s (Continu	ed)		
10 T	C	24.7	24.7	22.0	0). 5	25
MBJ	Surface			23.9	24.5	25.
	1/4	29.4	28.7	28.5	29.2	28.
	Middepth	31.1	30.6	30.2	30.7	30.
	3/4	32.2	31.9	31.7	32.0	31.
	Bottom	32.4	32.3	31.9	32.2	32.
	Average	30.0	29.6	29.2	29.7	29.
MBK	Surface	25.4	24.5	23.3	24.6	24.
	1/4	29.7	28.5	28.4	28.9	28.
	Middepth	31.0	30.9	30.7	30.9	31.
	3/4	32.1	32.1	31.7	32.0	31.
	Bottom	32.5	32.3	31.9	32.4	32.
	Average	30.1	29.7	29.2	29.7	29.
MBL	Surface	24.3	24.1	23.8	24.4	24.
MULL						
	1/4	29.5	28.4	28.2	28.7	28.
	Middepth	31.5	30.9	30.8	30.8	30
	3/4	32.4	32.2	31.8	32.0	31
	Bottom	32.5	32.5	32.1	32.3	32.
	Average	30.0	29.6	29.3	29.6	29
MBM	Surface	24.4	24.6	23.6	24.0	25
	1/4	29.6	28.8	28.6	29.1	29
	Middepth	31.2	30.7	30.5	30.9	30
	3/4	32.4	32.1	31.8	31.9	31
	Bottom	32.5	32.5	32.0	32.2	32
	Average	30.0	29.7	29.3	29.6	29
MBN	C	24.1	24.5	23.6	24.5	2).
PLDIN	Surface	29.7				24
			28.5	28.4	29.1	28
	Middepth	31.0	30.3	30.1	30.5	30
	3/4	32.2	32.0	31.8	32.0	31.
	Bottom	32.4	32.3	31.9	32.2	32
	Average	29.9	29.5	29.2	29.7	29
MBP	Surface		8		23.8	24
	Middepth				30.4	29
	Bottom				32.2	31.
	Average				28.8	28.

(Sheet 5 of 5)

National States Dre Concentrations, 335

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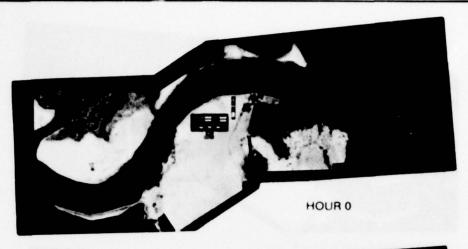
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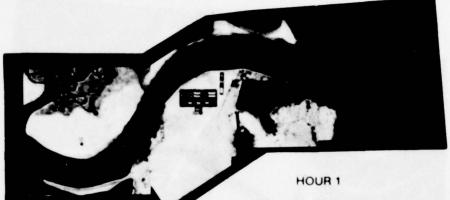
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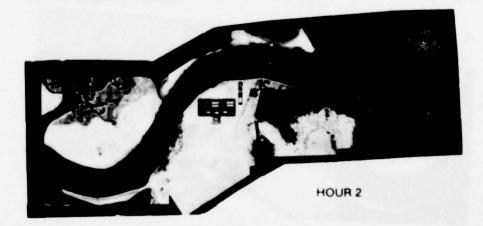
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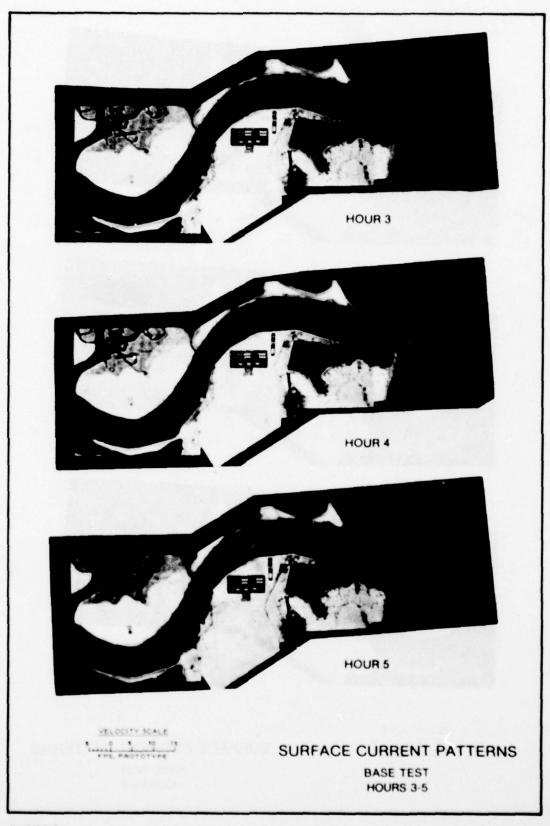


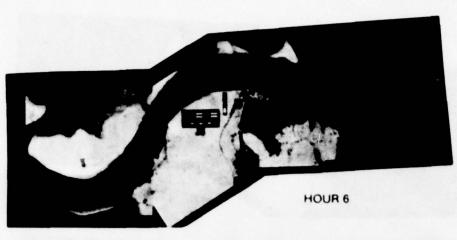


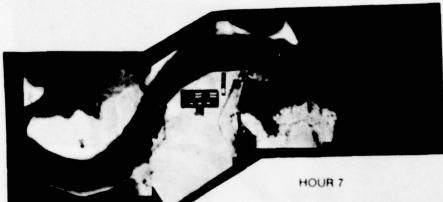


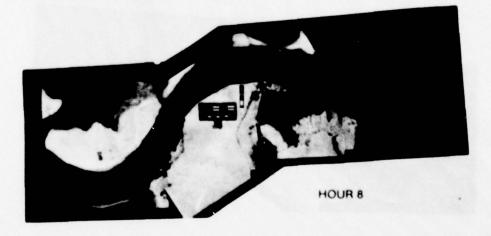
SURFACE CURRENT PATTERNS

BASE TEST HOURS 0-2









SURFACE CURRENT PATTERNS

BASE TEST HOURS 6-8

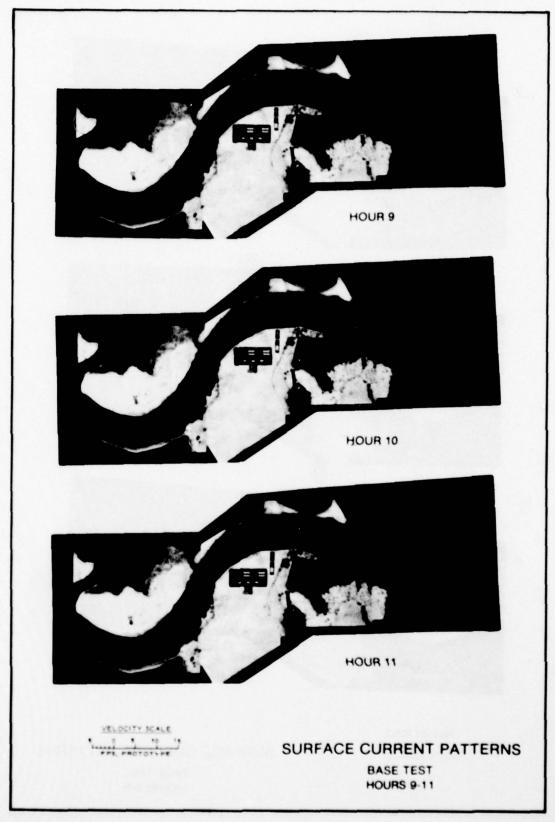
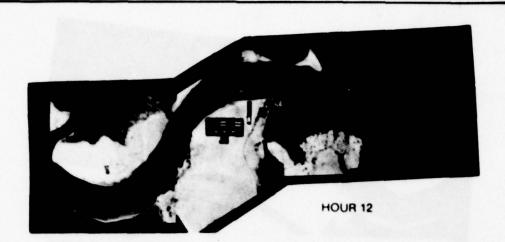
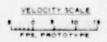


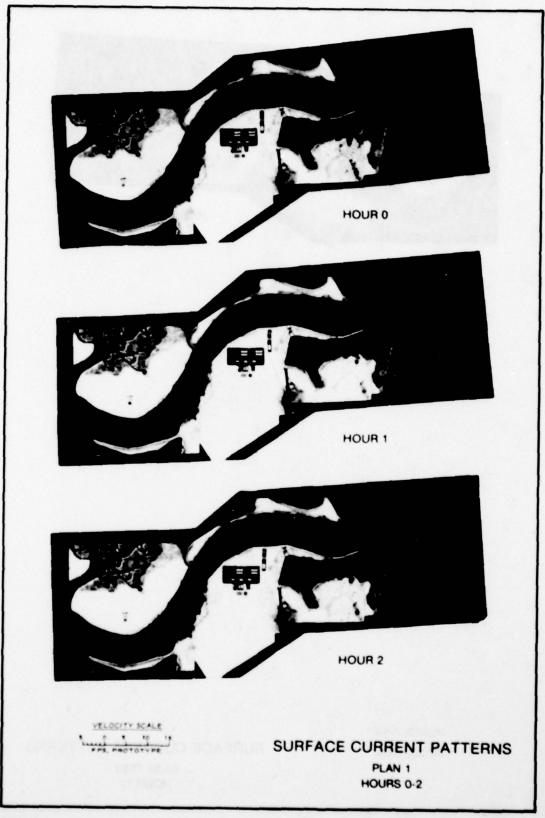
PHOTO 4



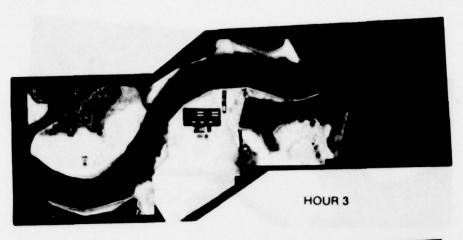


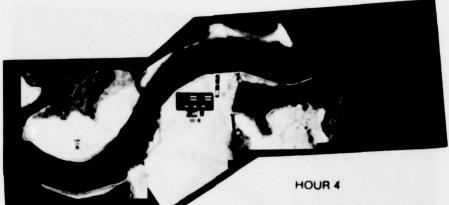
## SURFACE CURRENT PATTERNS

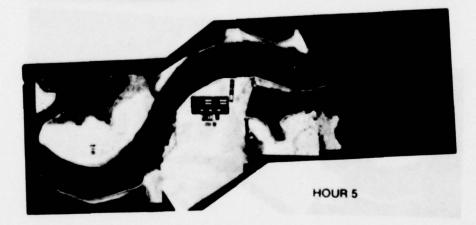
BASE TEST HOUR 12



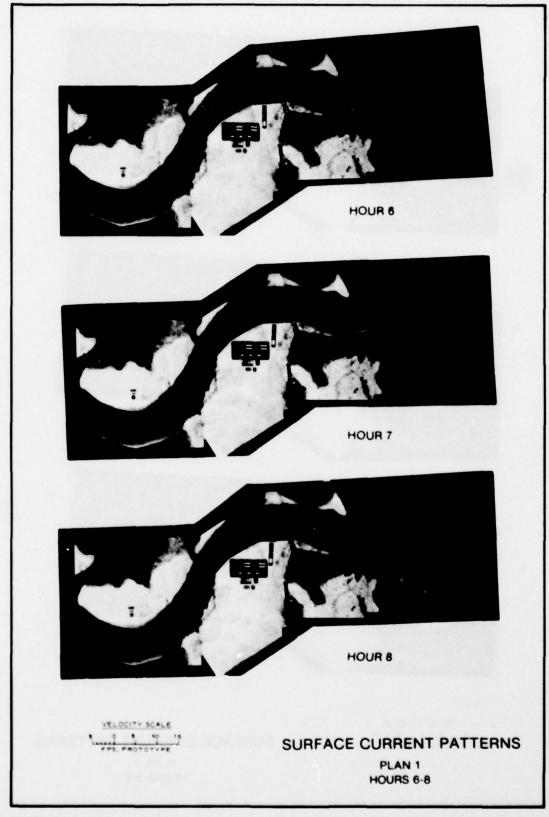
РНОТО 6



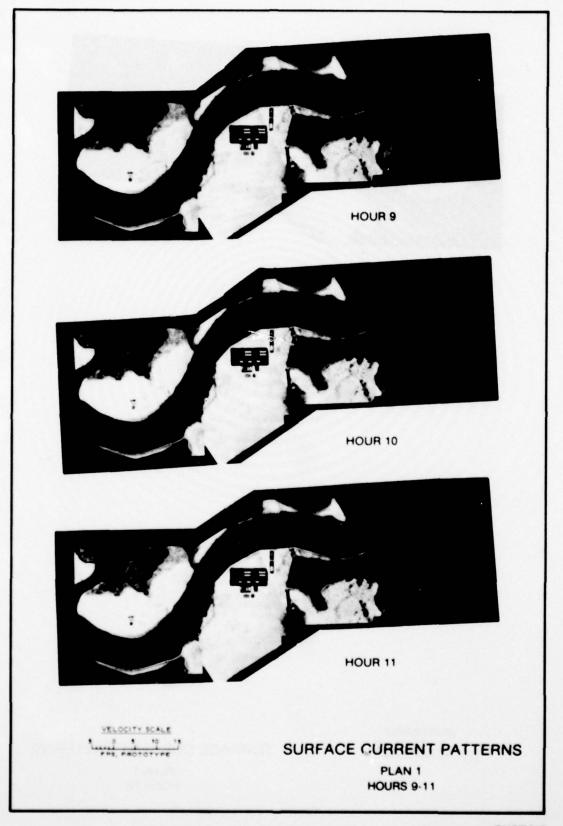


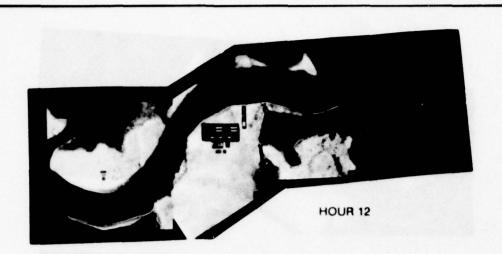


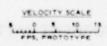
SURFACE CURRENT PATTERNS
PLAN 1
HOURS 3-5



РНОТО 8

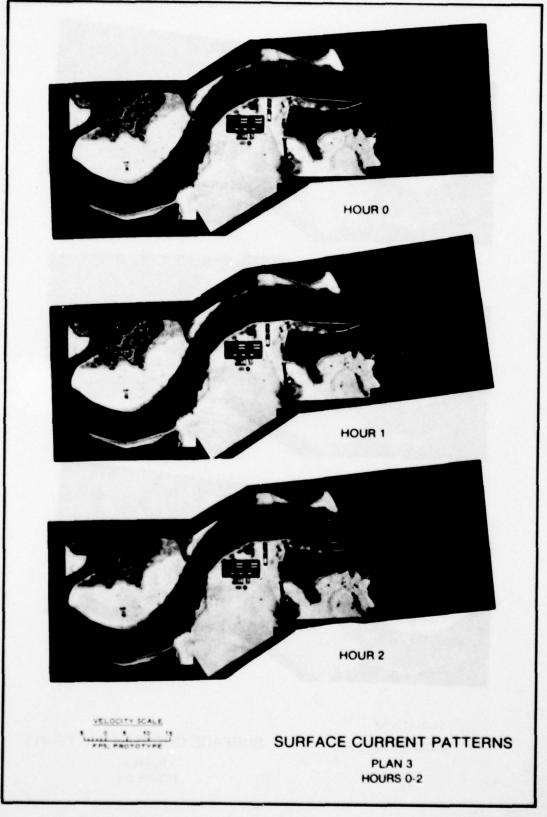




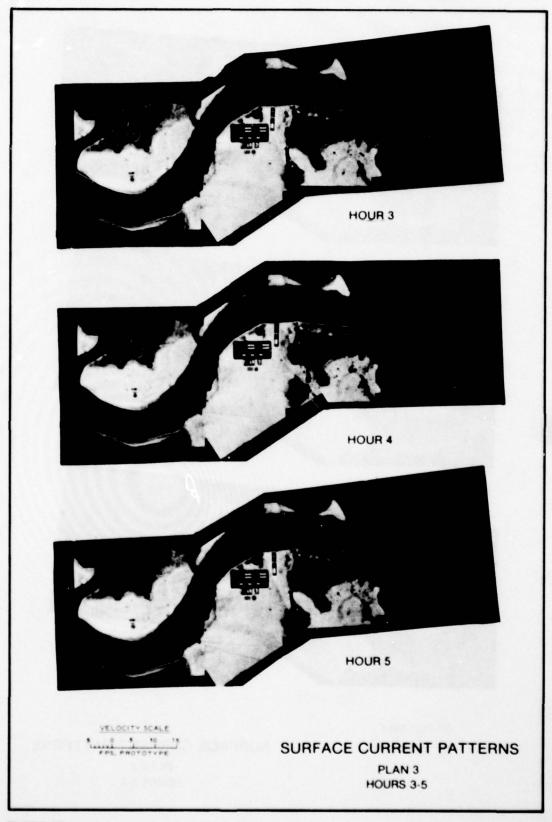


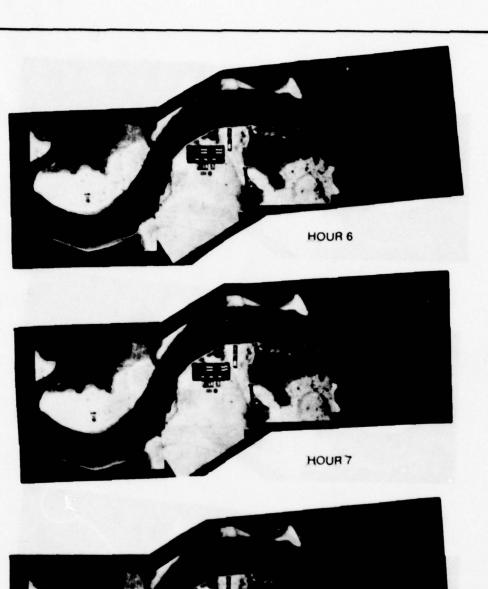
## SURFACE CURRENT PATTERNS

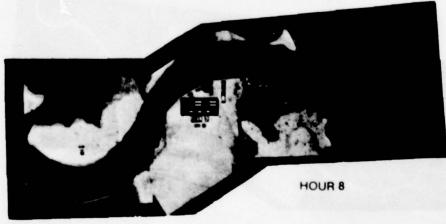
PLAN 1 HOUR 12



**PHOTO 11** 

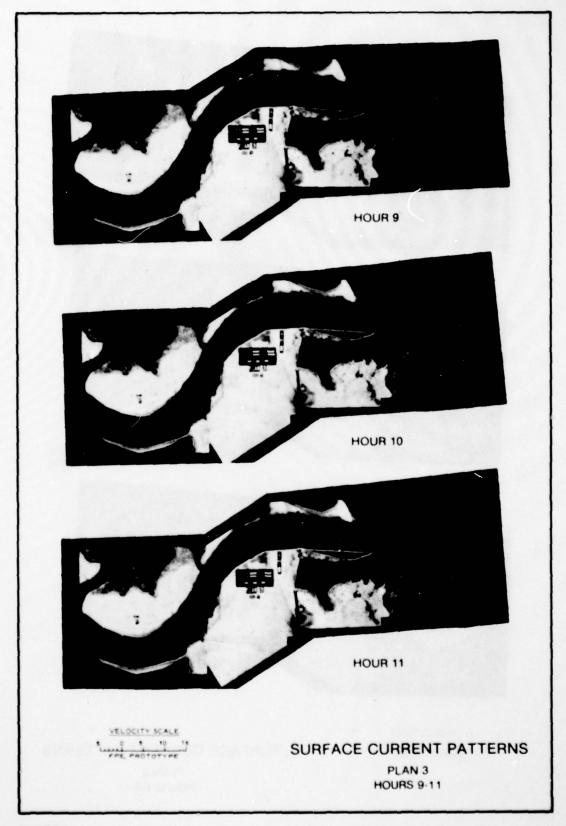






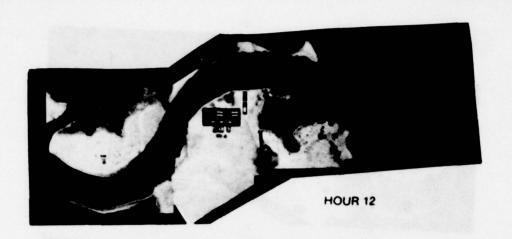
SURFACE CURRENT PATTERNS

PLAN 3 HOURS 6-8



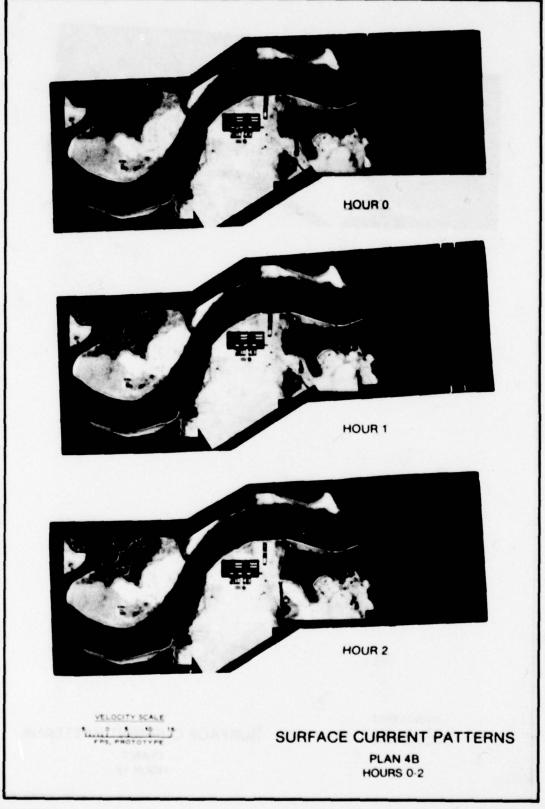
**PHOTO 14** 

ARMY ENGINEER WATERWAYS EXPERIMENT STATION VICKSBURG MS F/G 8/8
MAYPORT-MILL COVE MODEL STUDY. REPORT 2. MAYPORT NAVAL BASIN ST--ETC(U)
AUG 79 N J BROGDON
WES-TR-HL-79-12
NL AD-A077 046 UNCLASSIFIED 2 of 5 AD-A077046

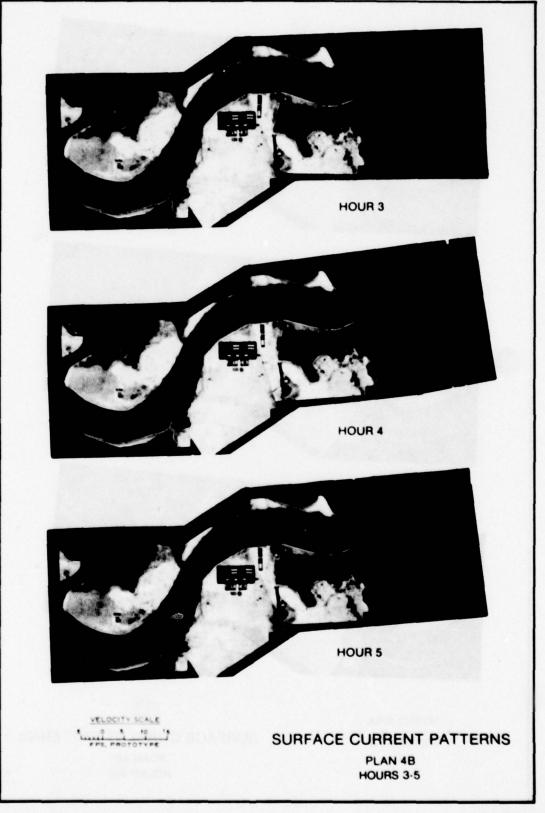


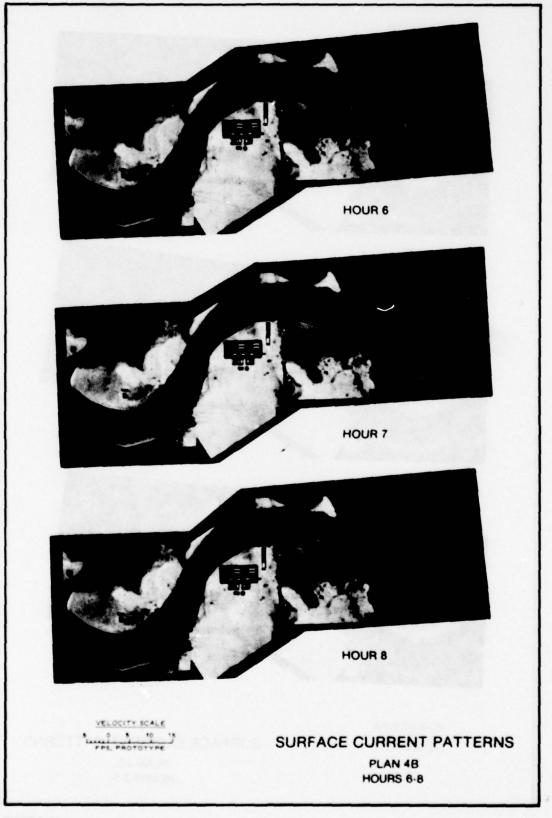
SURFACE CURRENT PATTERNS

PLAN 3 HOUR 12

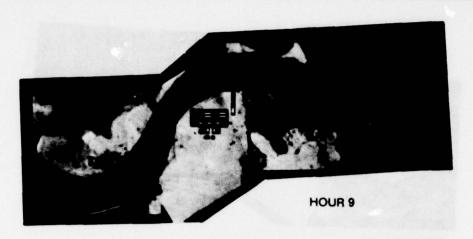


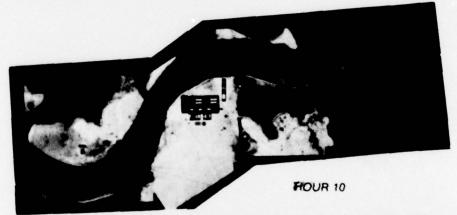
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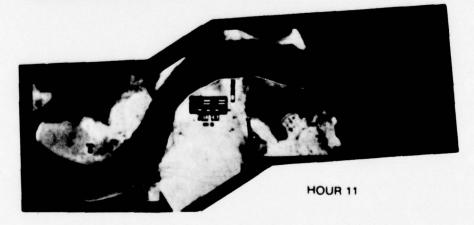


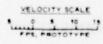


**PHOTO 18** 

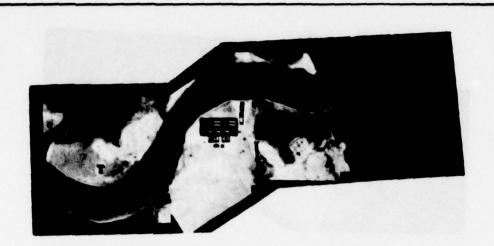






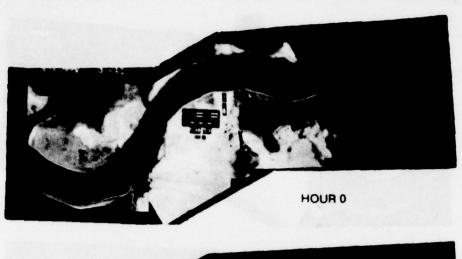


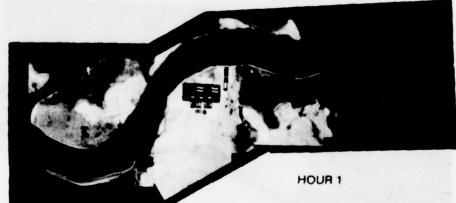
SURFACE CURRENT PATTERNS
PLAN 4B
HOURS 9-11

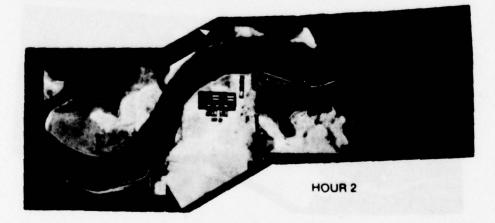


SURFACE CURRENT PATTERNS

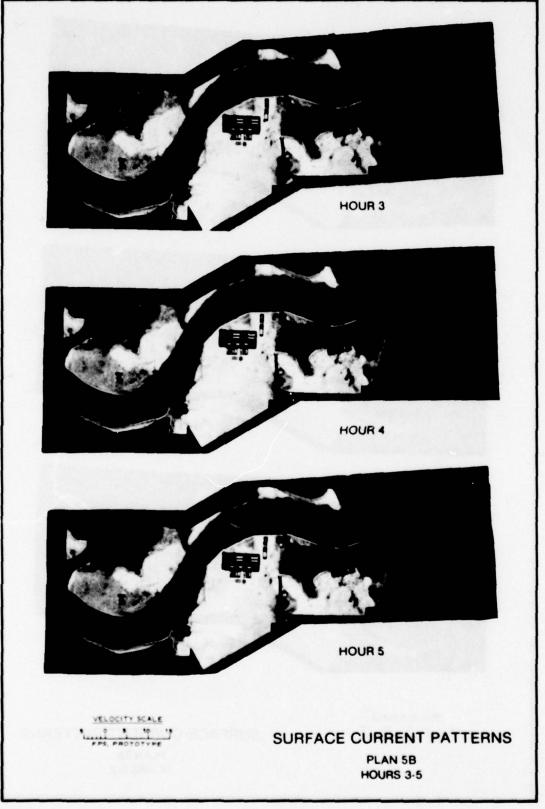
PLAN 4B HOUR 12



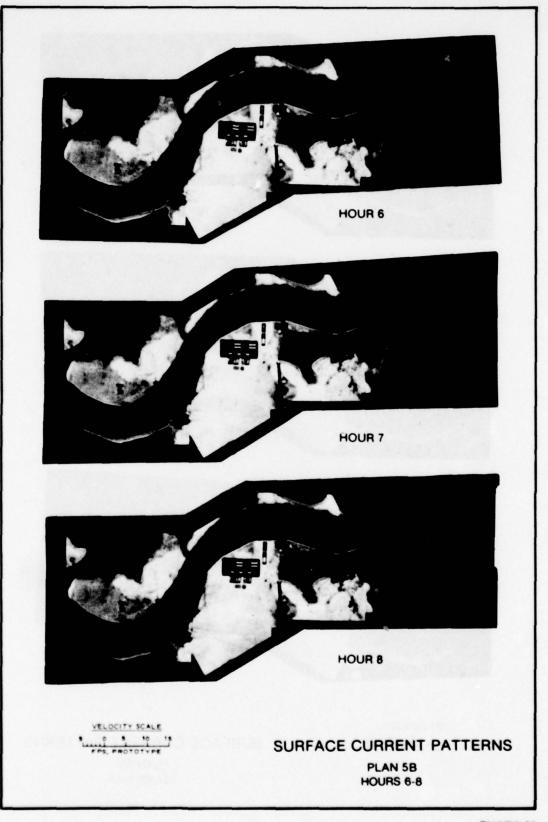


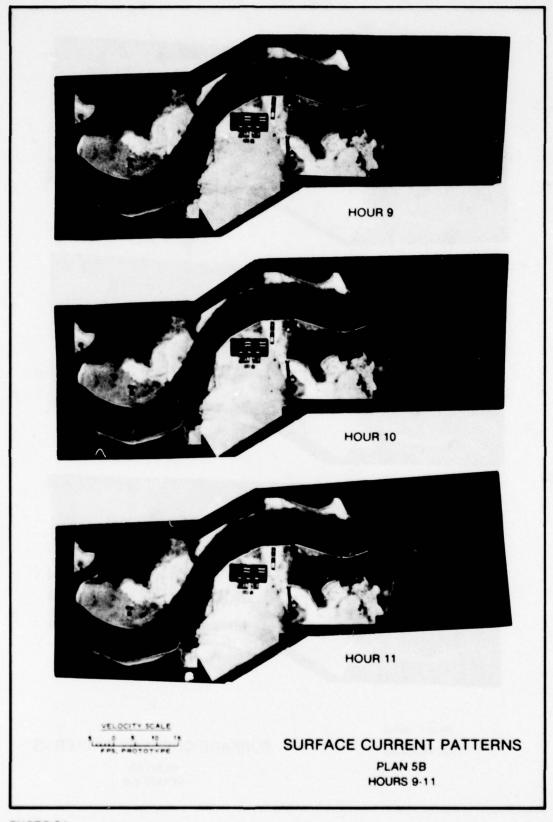


SURFACE CURRENT PATTERNS
PLAN 5B
HOURS 0-2

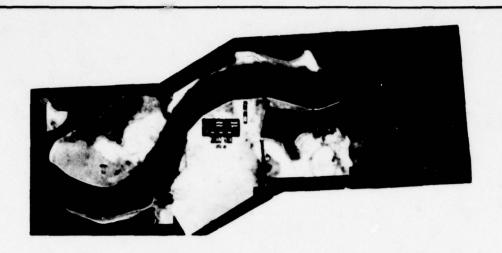


**PHOTO 22** 





**PHOTO 24** 



VELOCITY SCALE

## SURFACE CURRENT PATTERNS

PLAN 5B HOUR 12

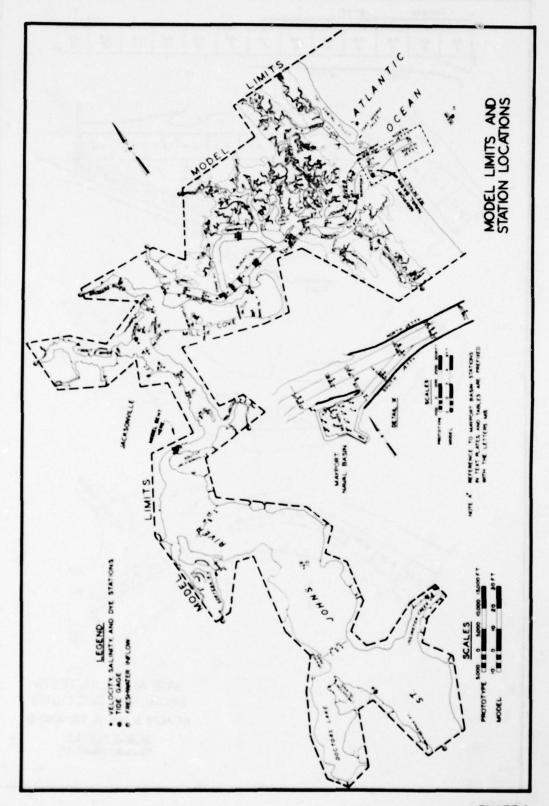


PLATE 1

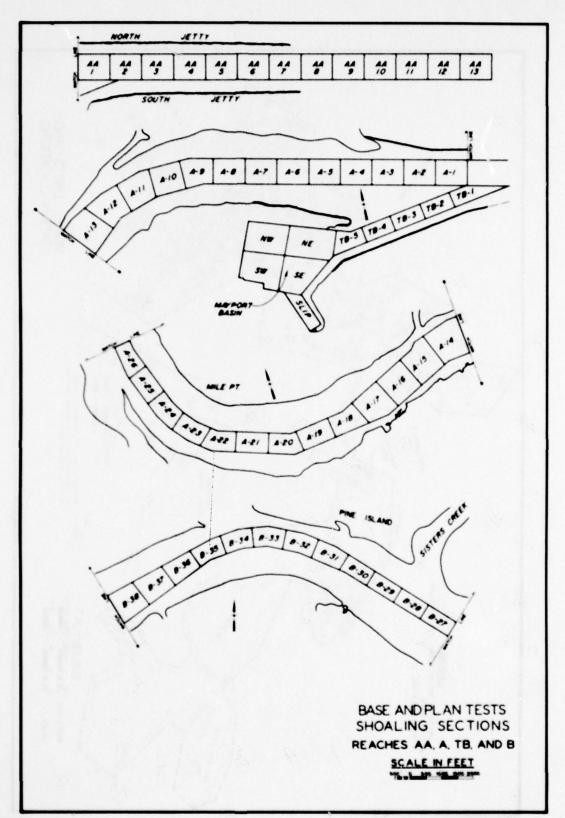


PLATE 2

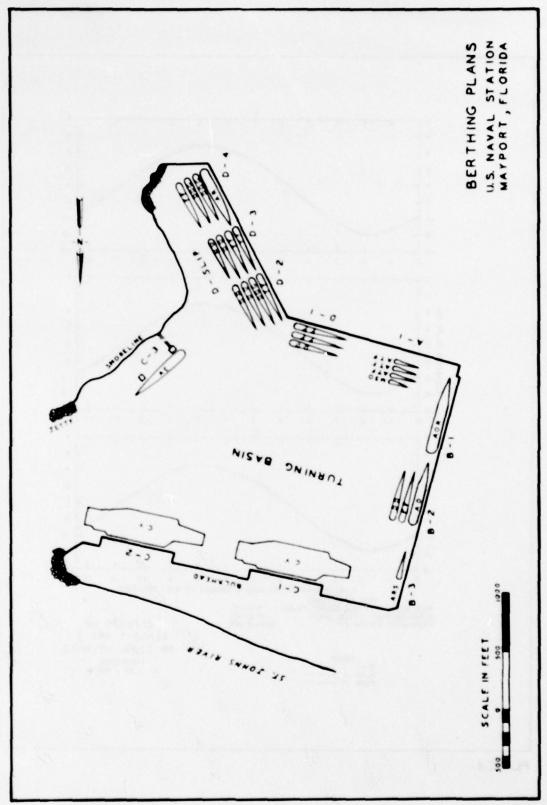


PLATE 3

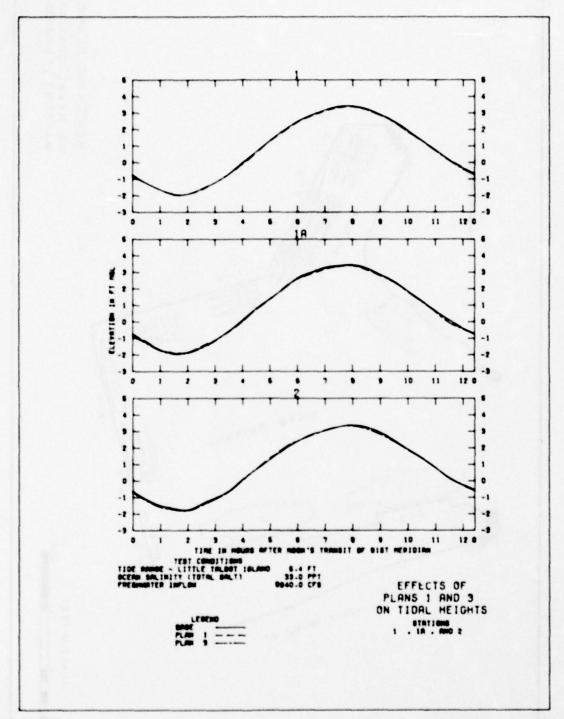


PLATE 4

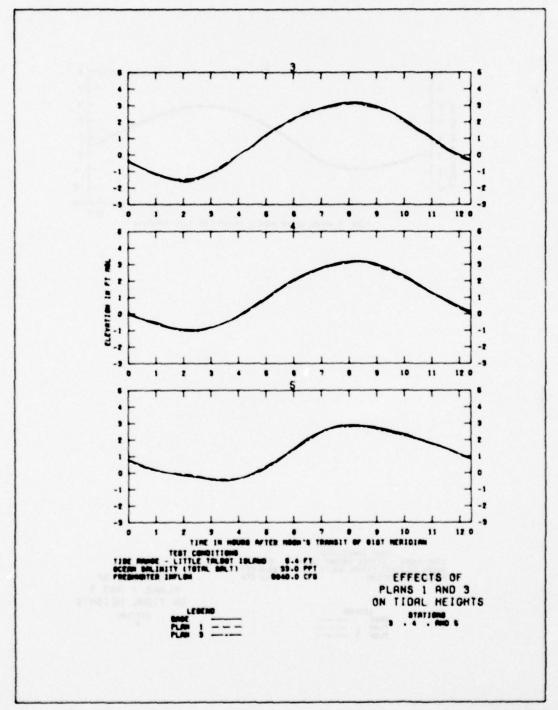


PLATE 5

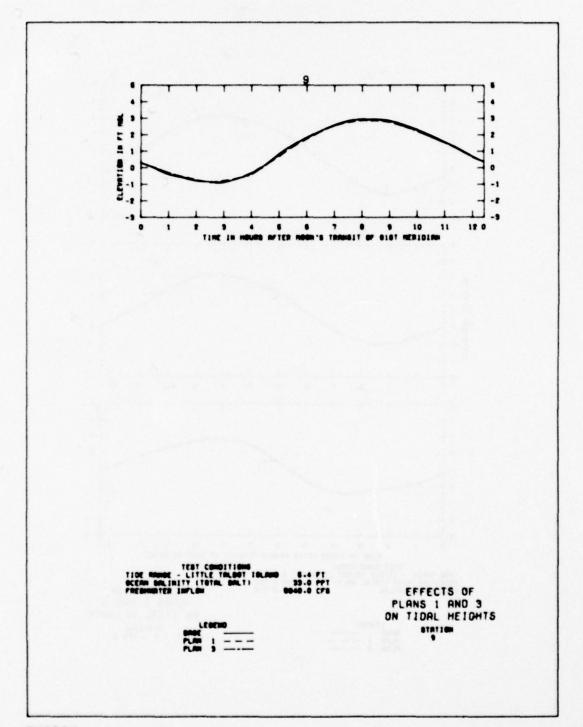


PLATE 6

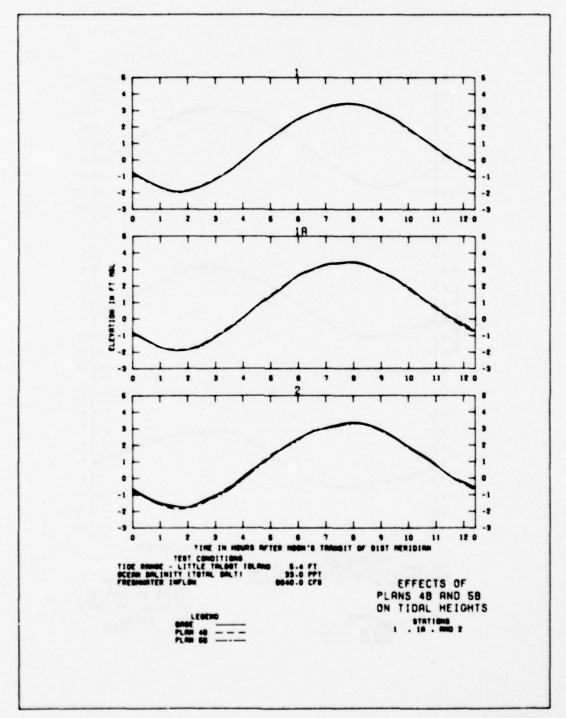


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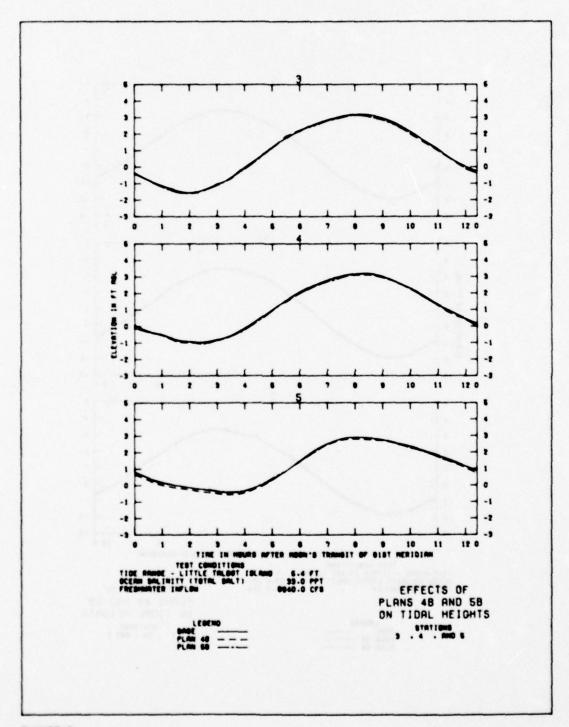
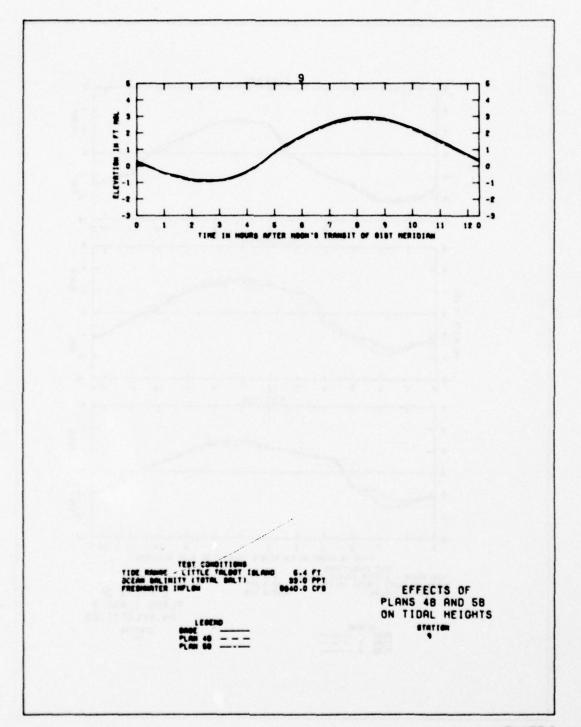


PLATE 8



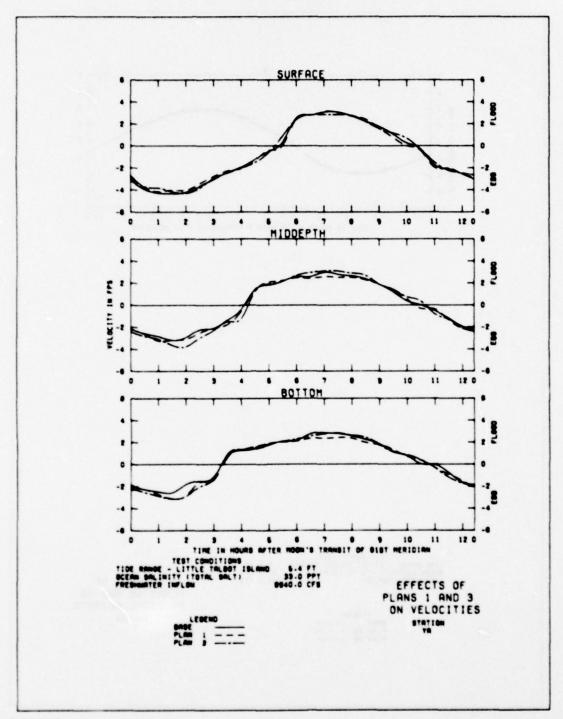


PLATE 10

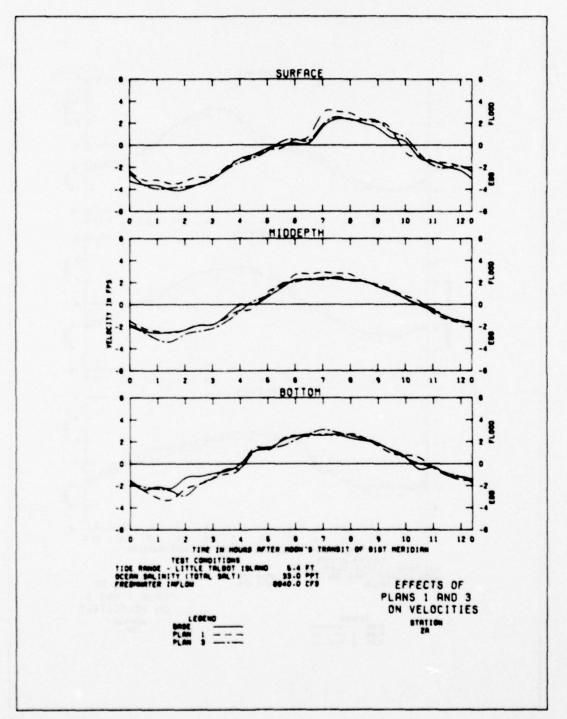


PLATE 11

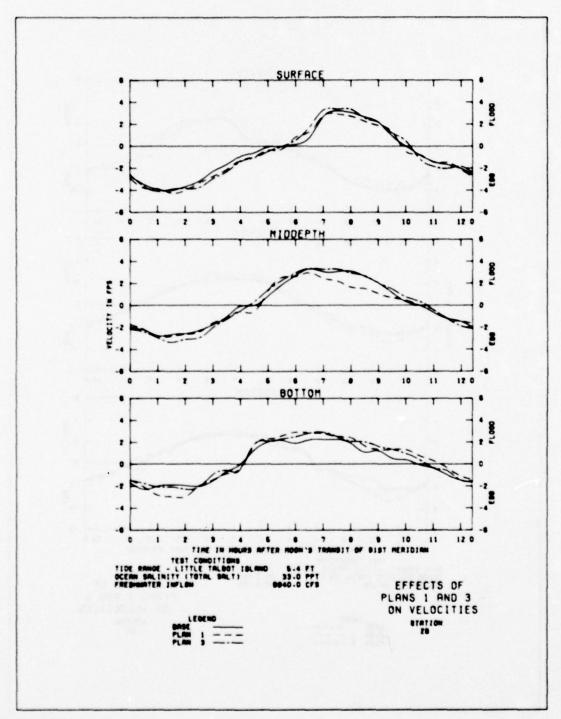


PLATE 12

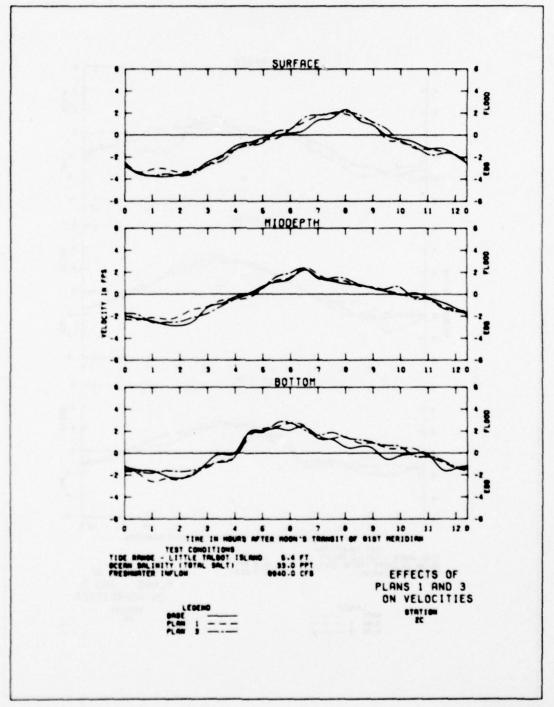


PLATE 13

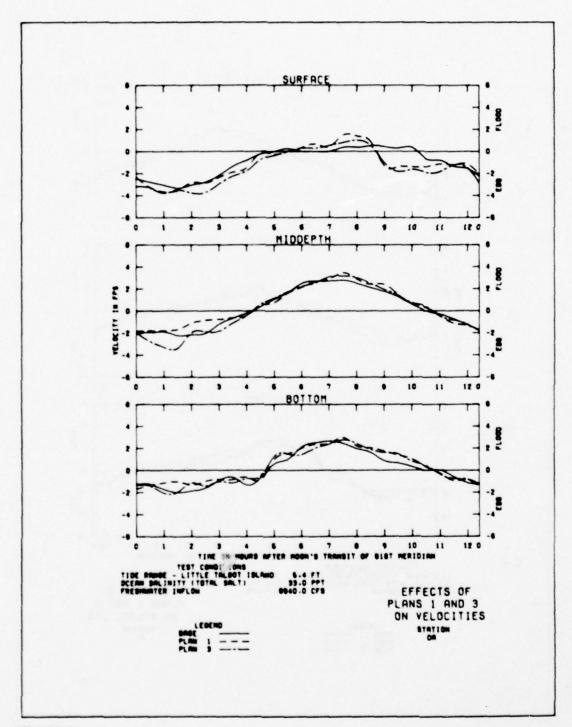


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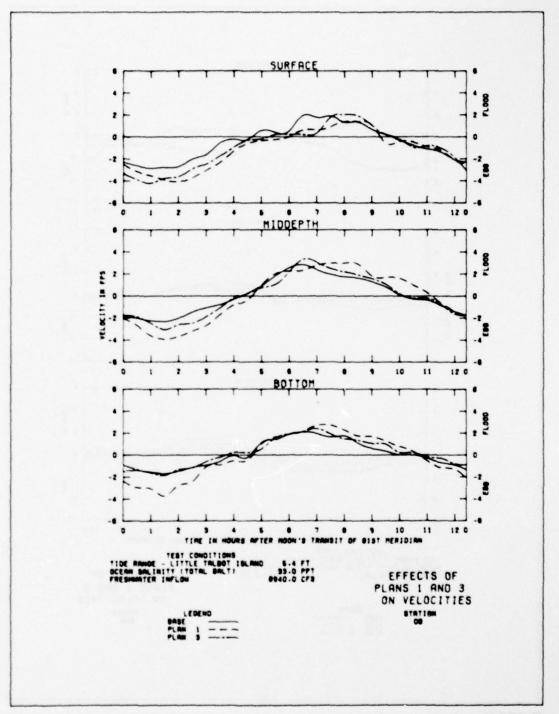


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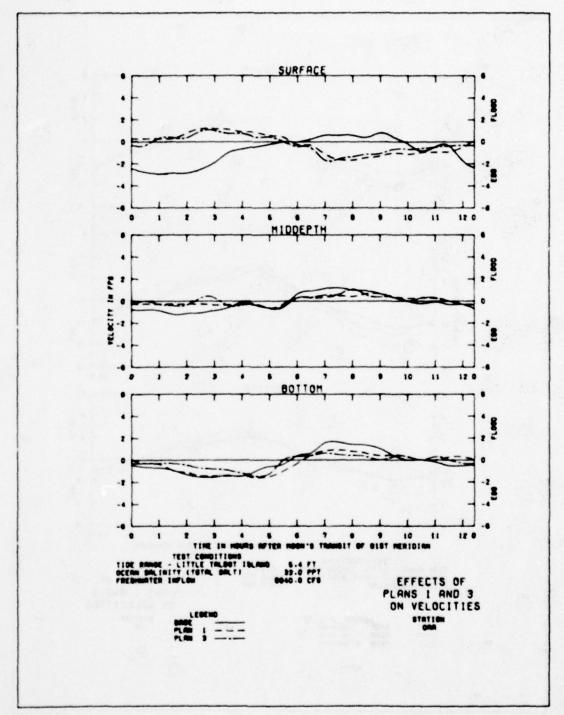
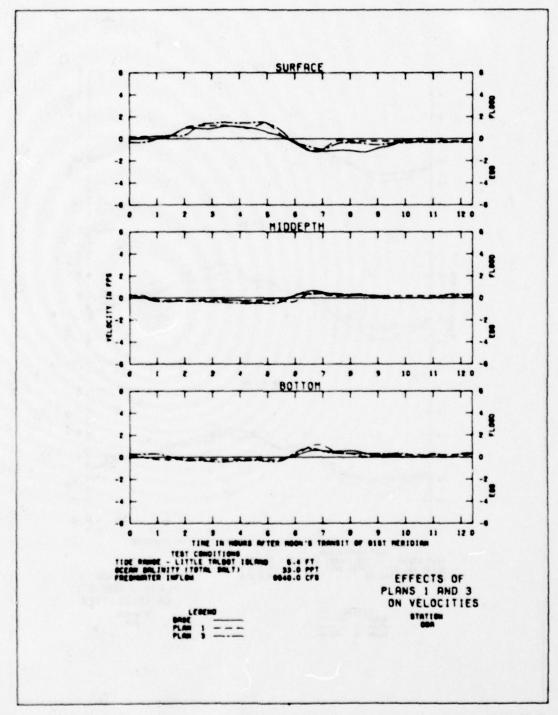


PLATE 16



. 11.12

PLATE 17

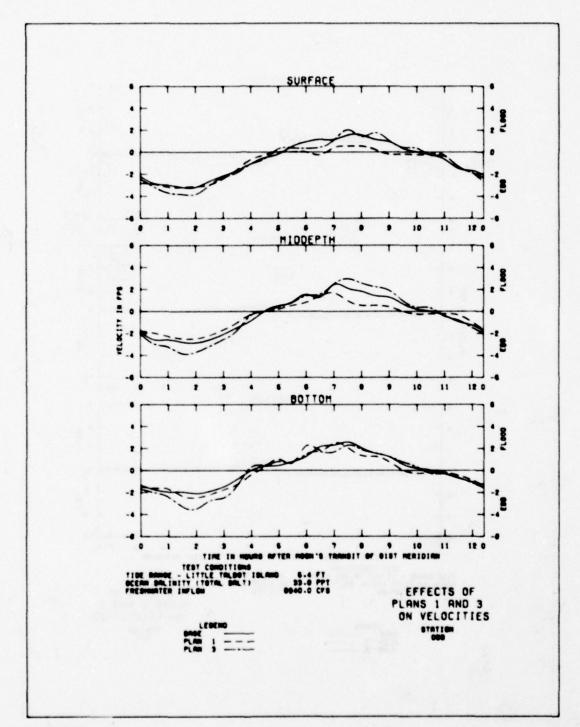


PLATE 18

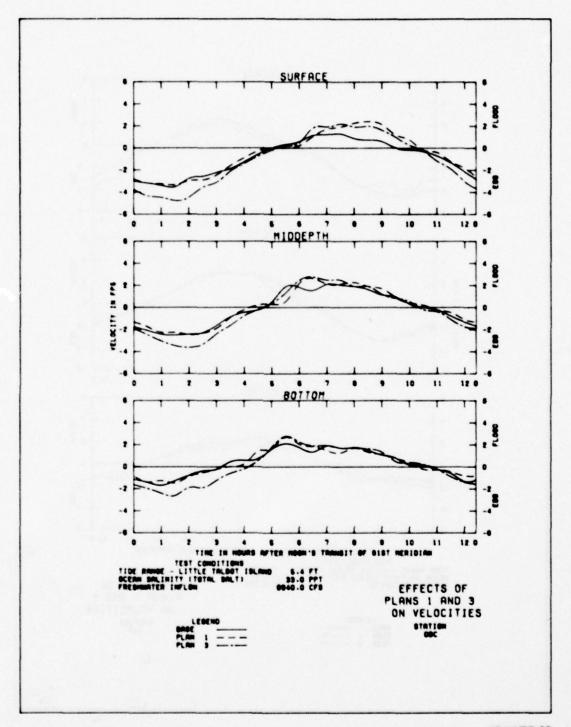


PLATE 19

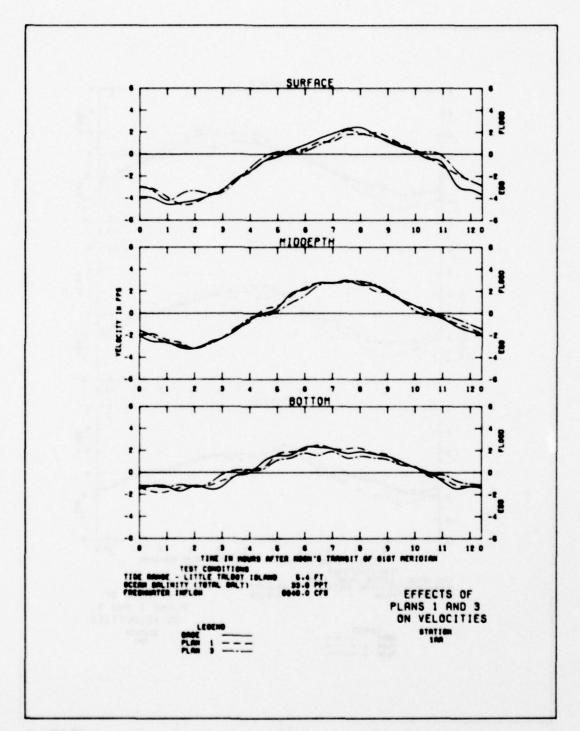


PLATE 20

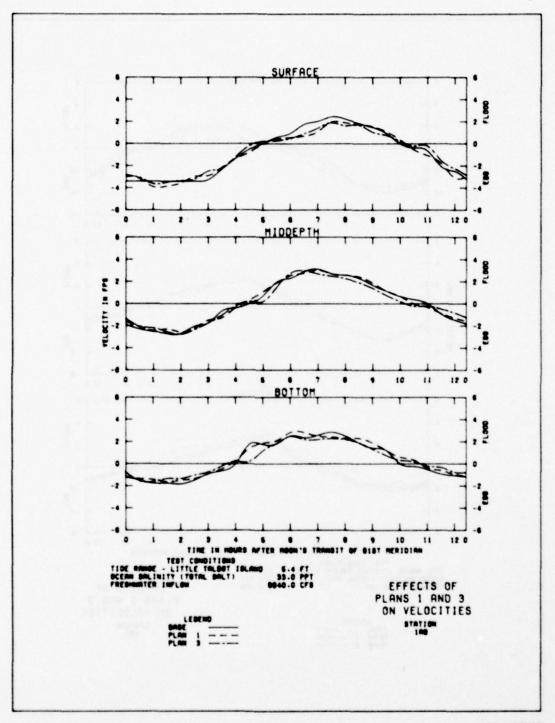


PLATE 21

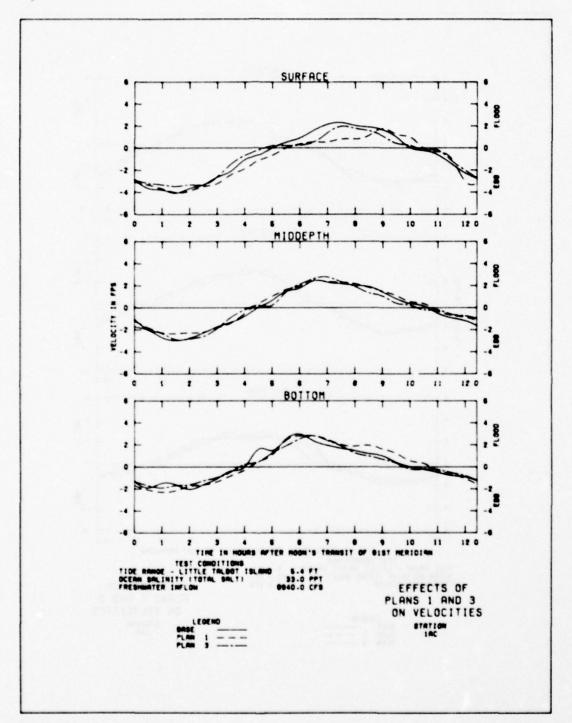


PLATE 22

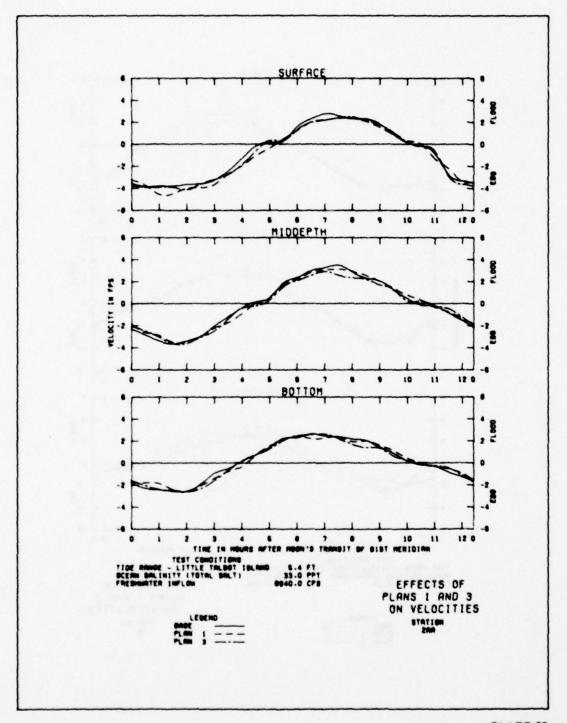


PLATE 23

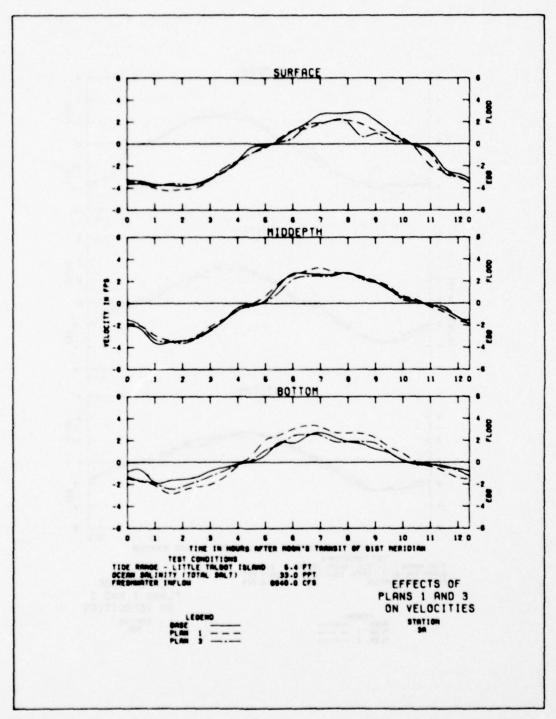


PLATE 24

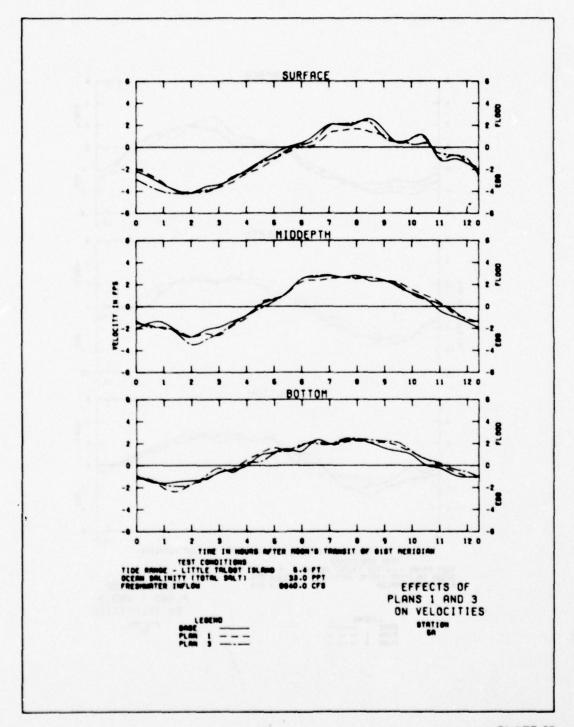


PLATE 25

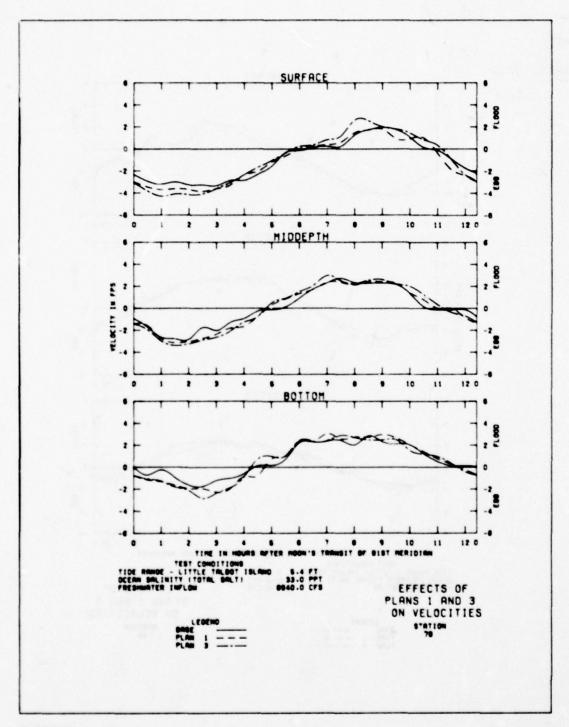


PLATE 26

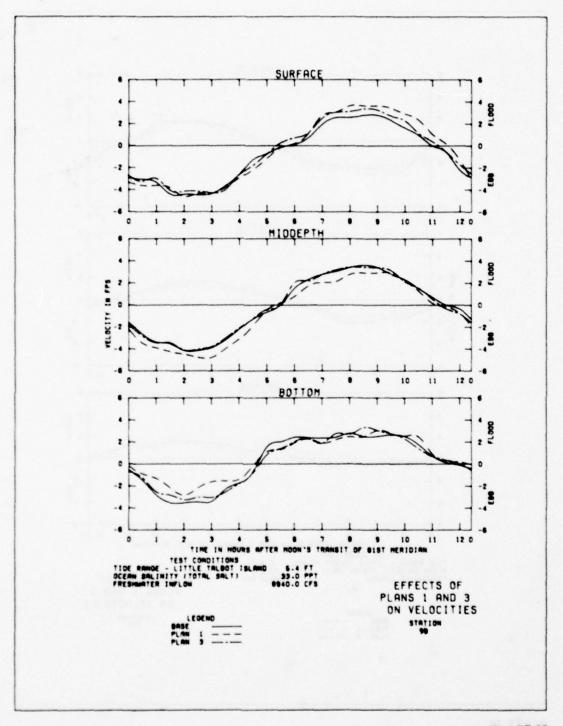


PLATE 27

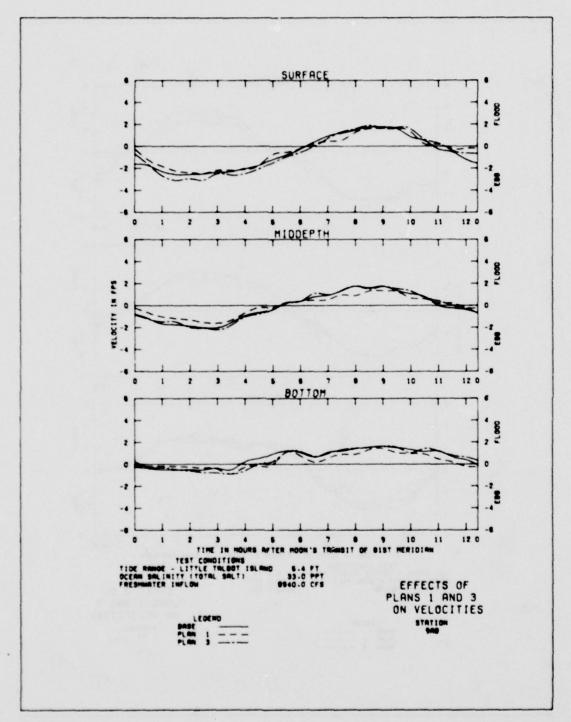


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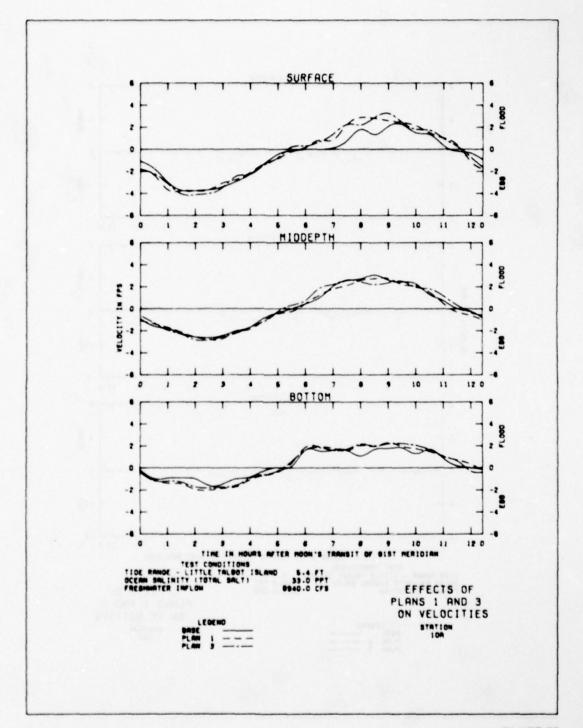


PLATE 29

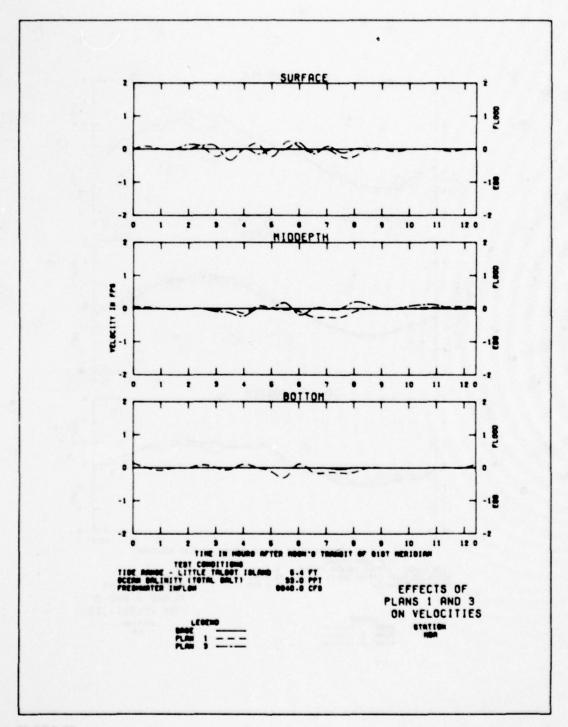


PLATE 30

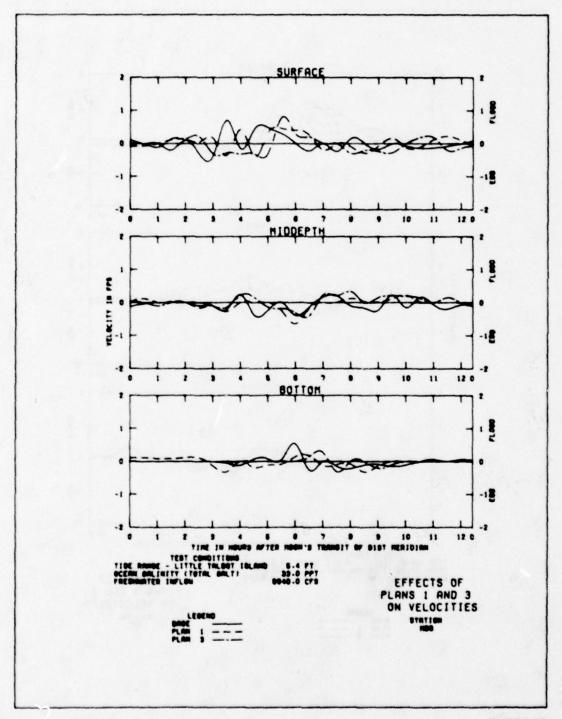


PLATE 31

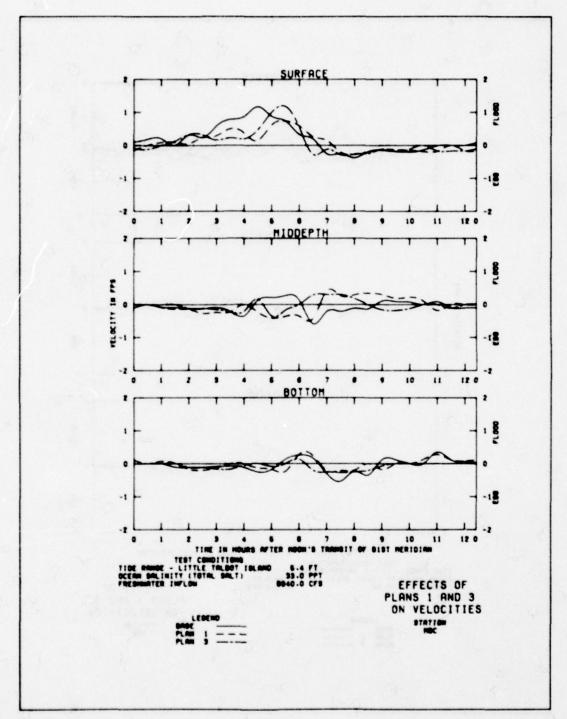


PLATE 32

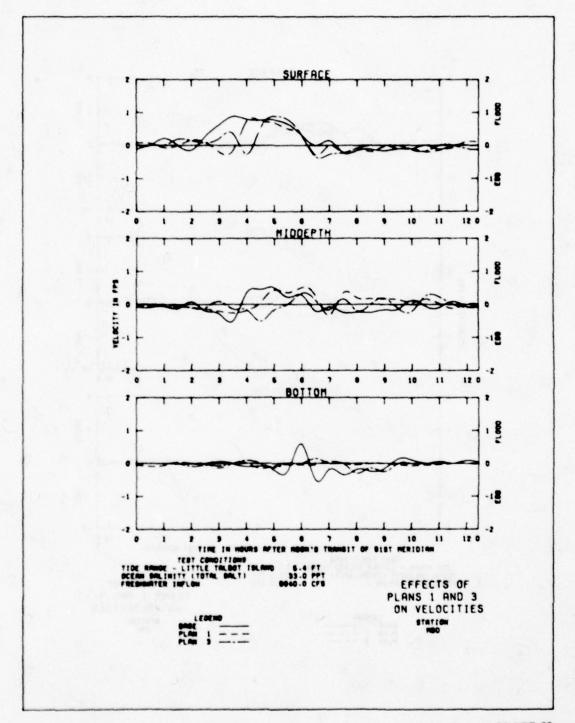


PLATE 33

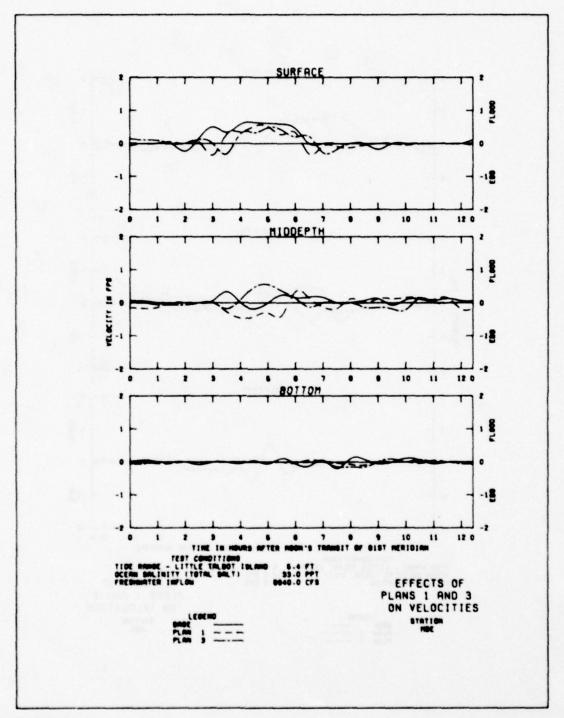


PLATE 34

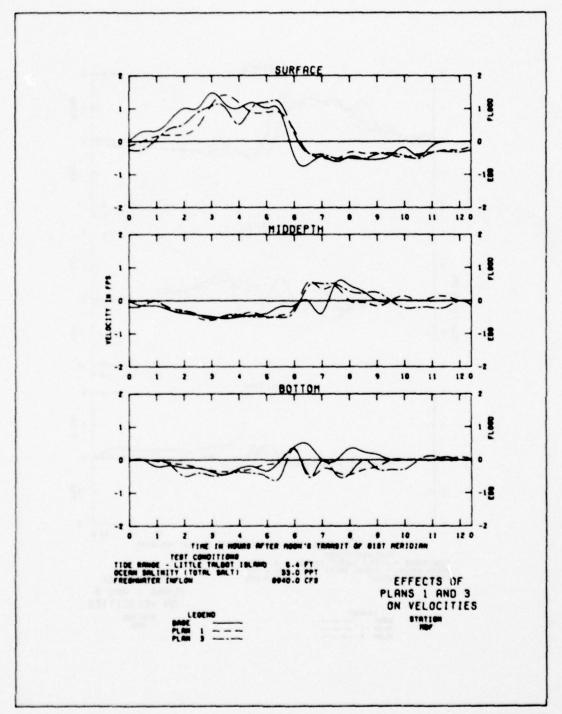


PLATE 35

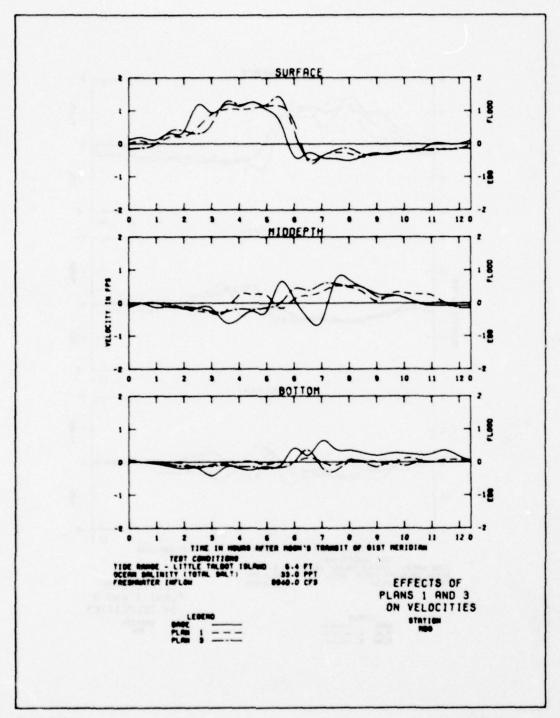


PLATE 36

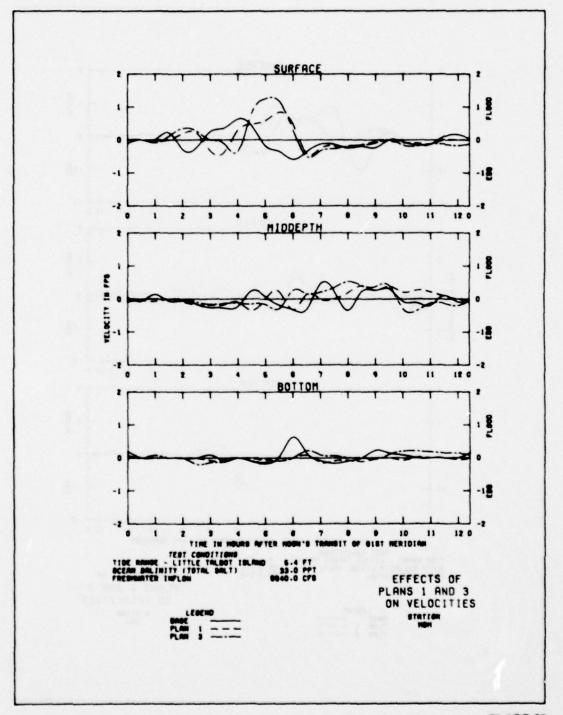


PLATE 37

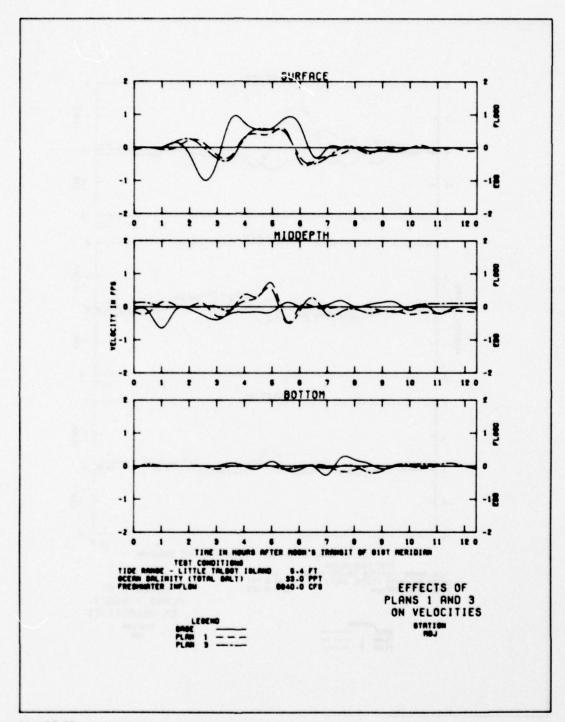
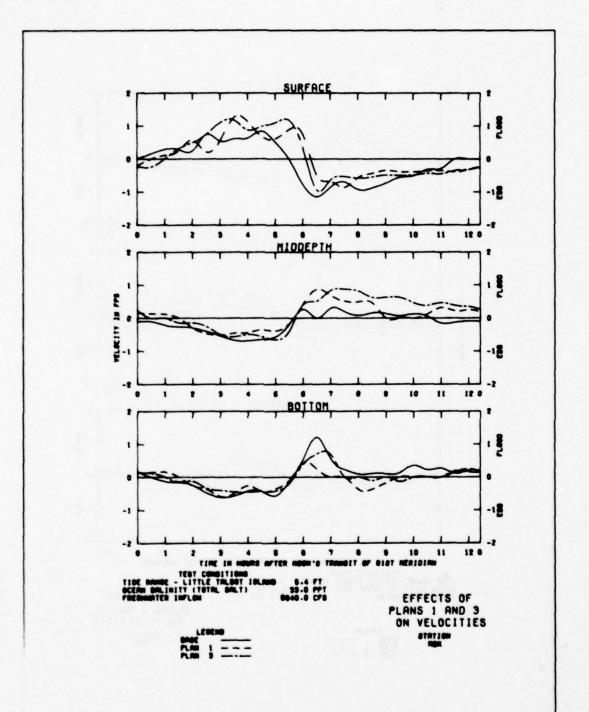


PLATE 38



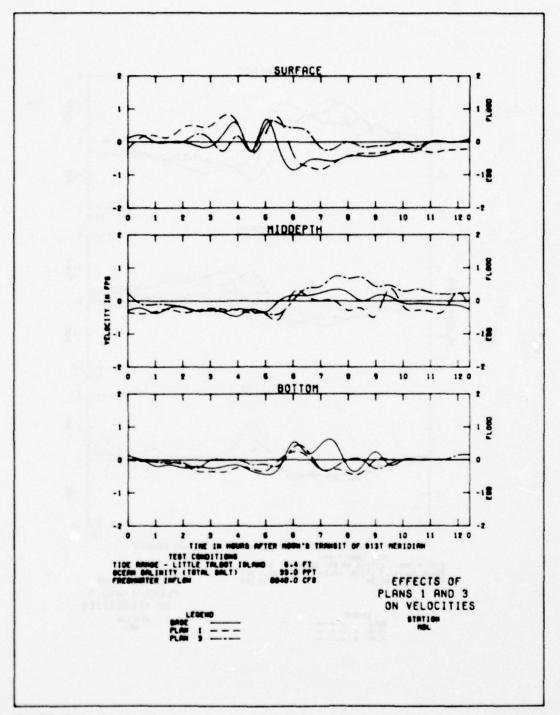


PLATE 40

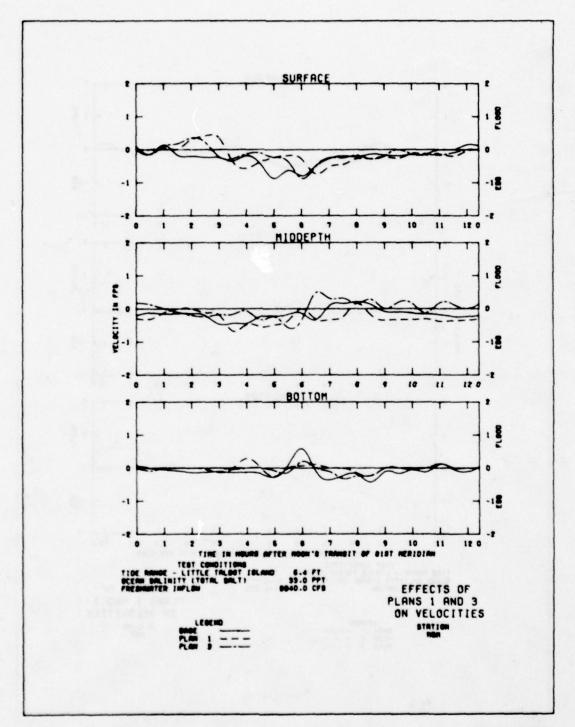


PLATE 41

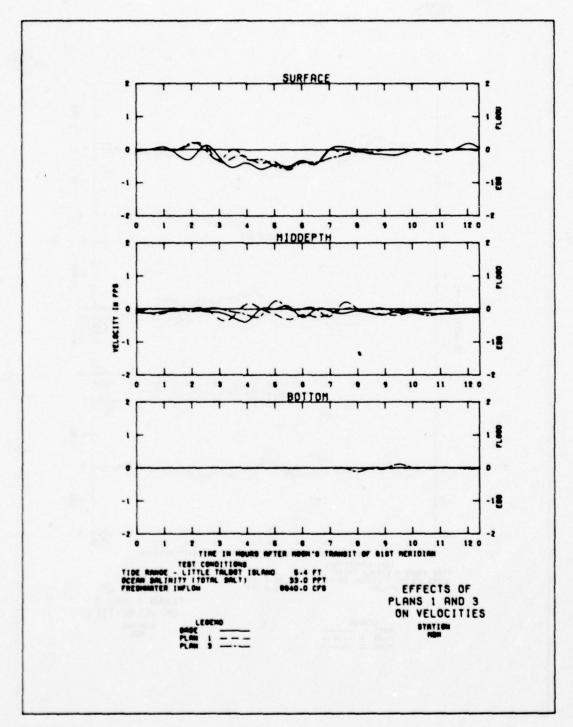


PLATE 42

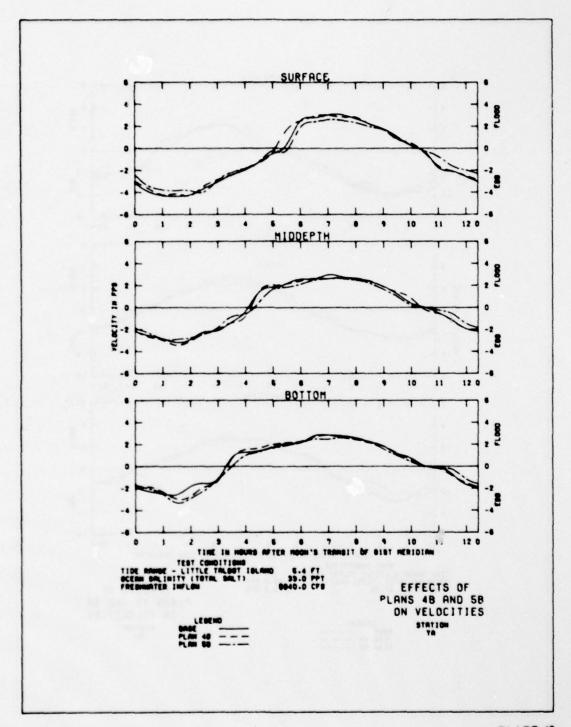


PLATE 43

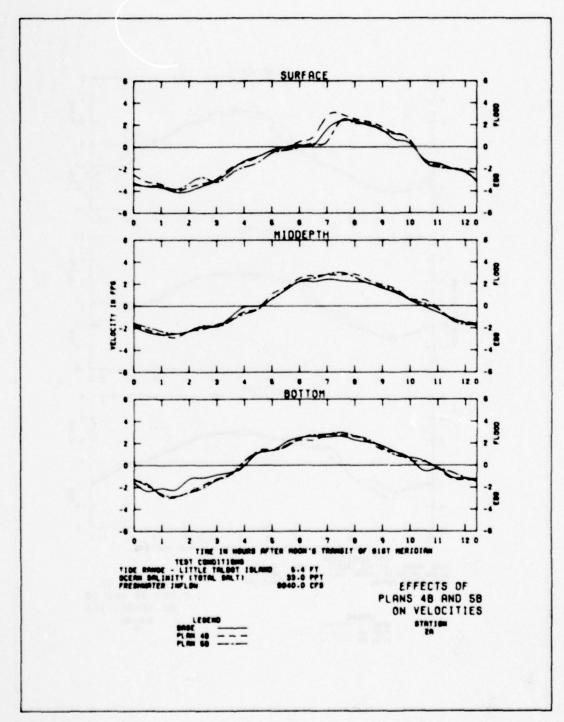


PLATE 44

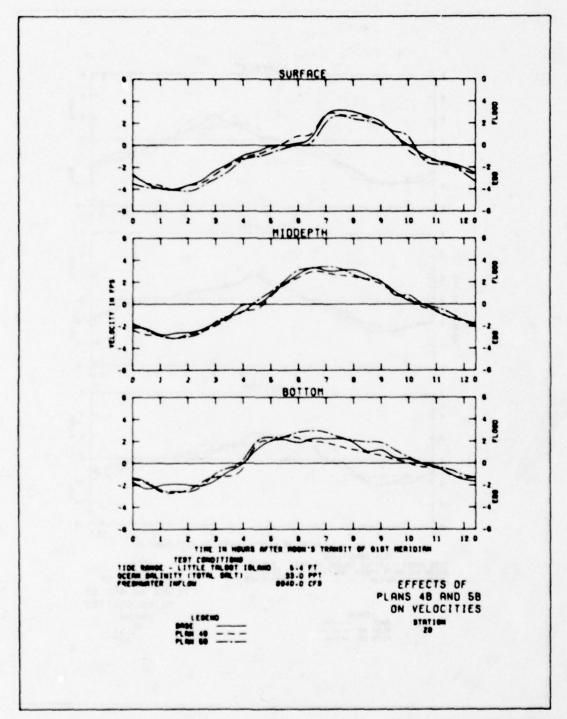


PLATE 45

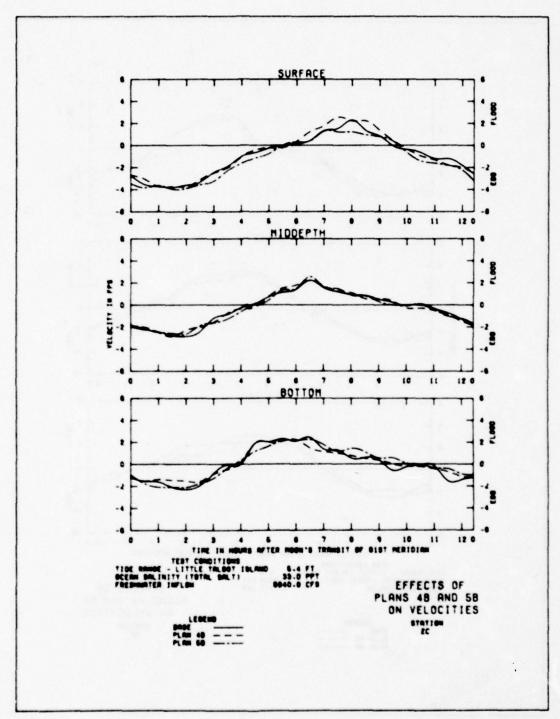


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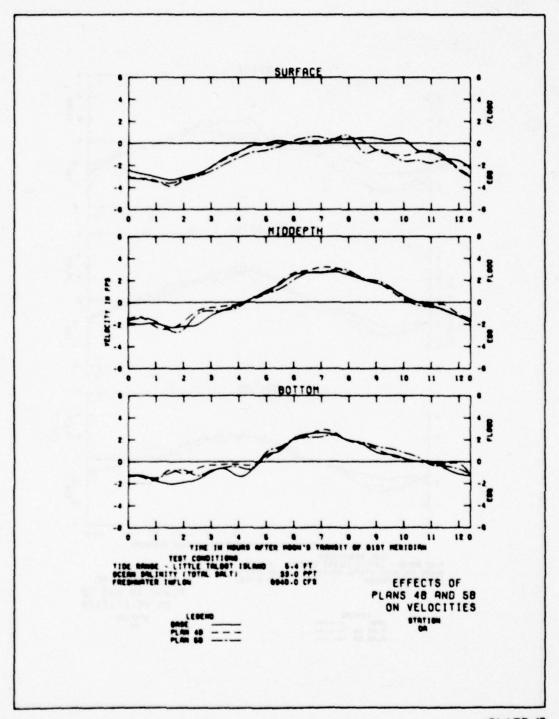


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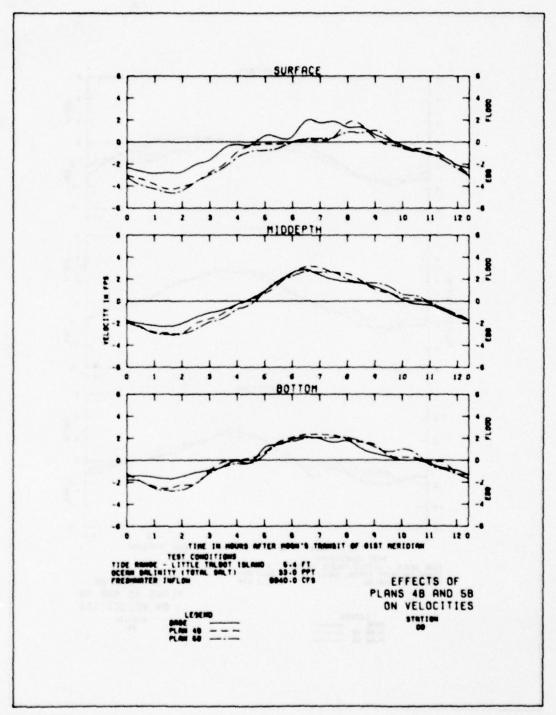


PLATE 48

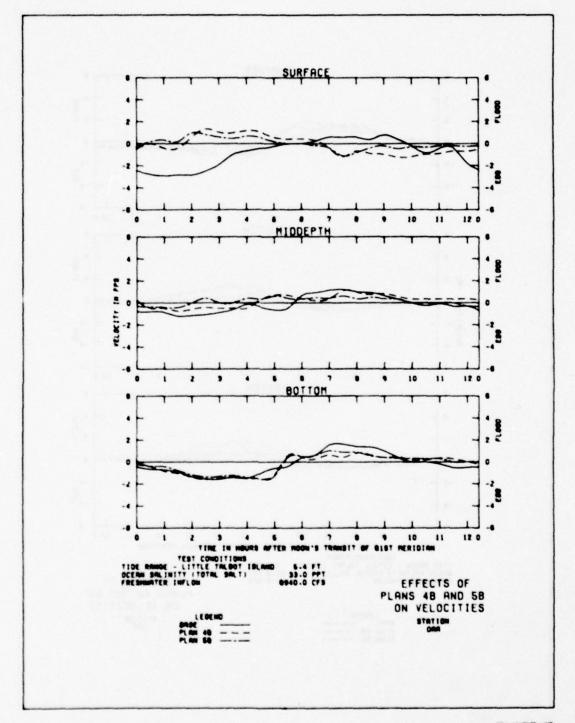


PLATE 49

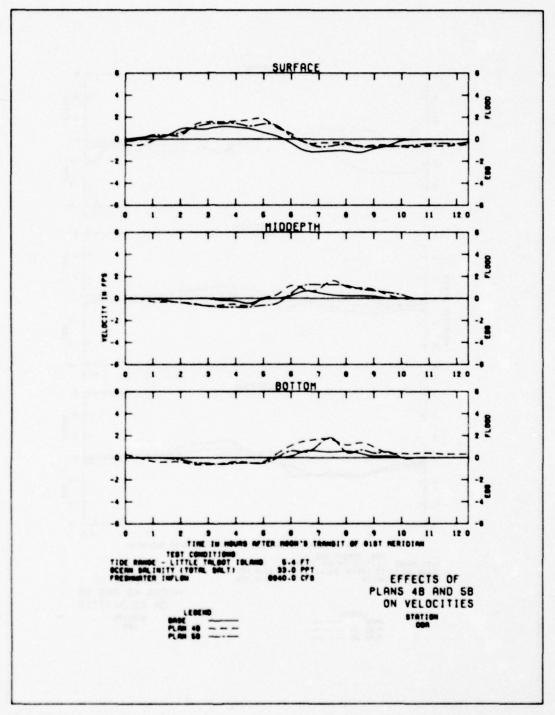


PLATE 50

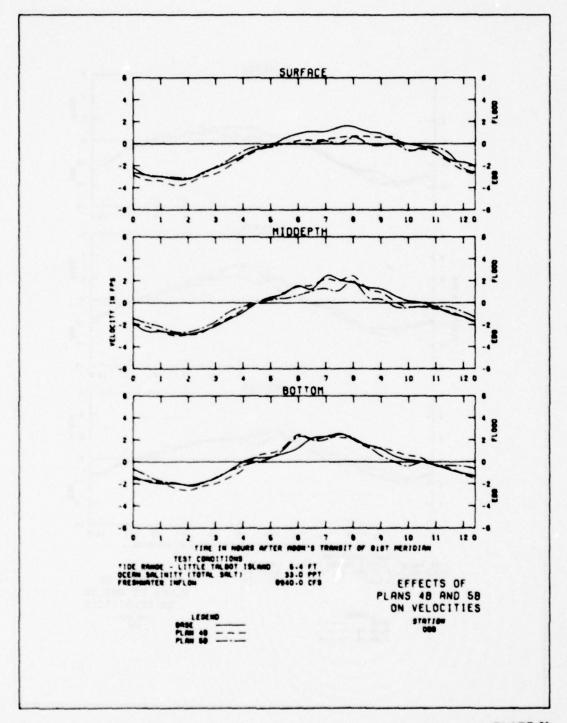


PLATE 51

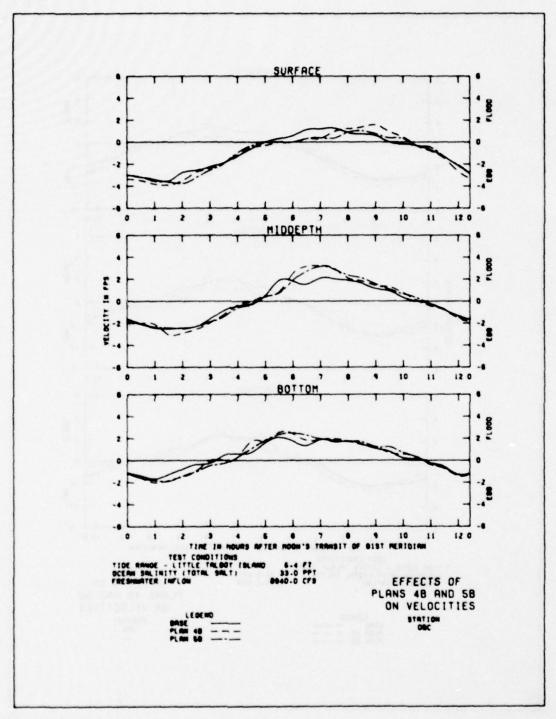


PLATE 52

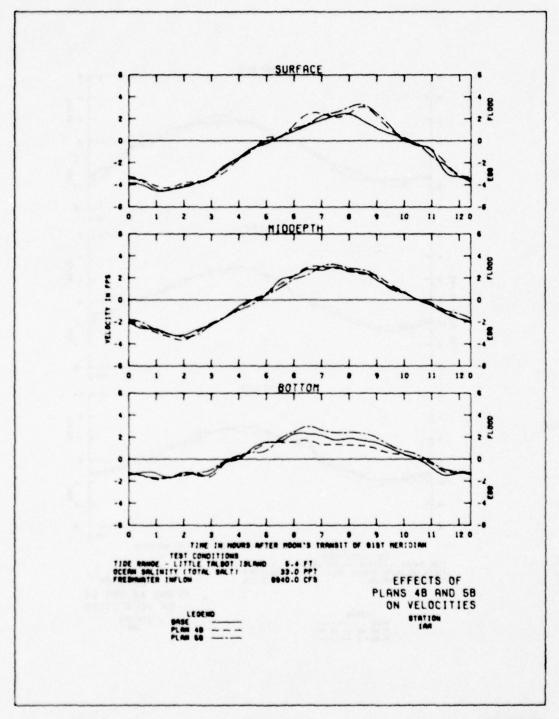


PLATE 53

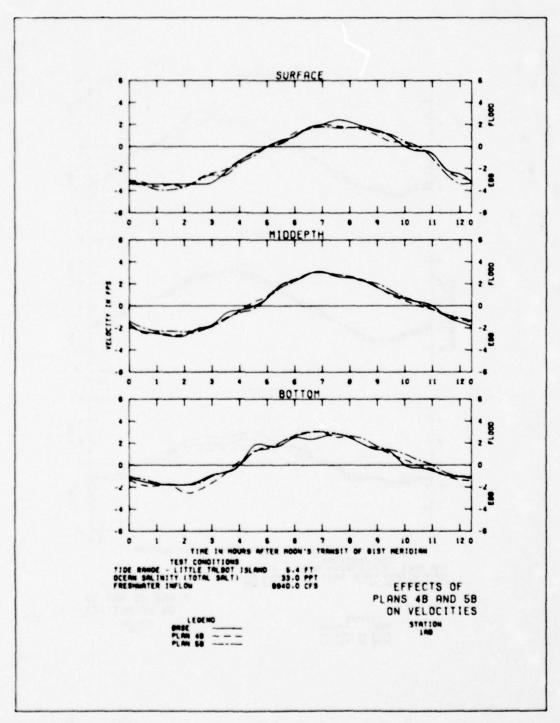


PLATE 54

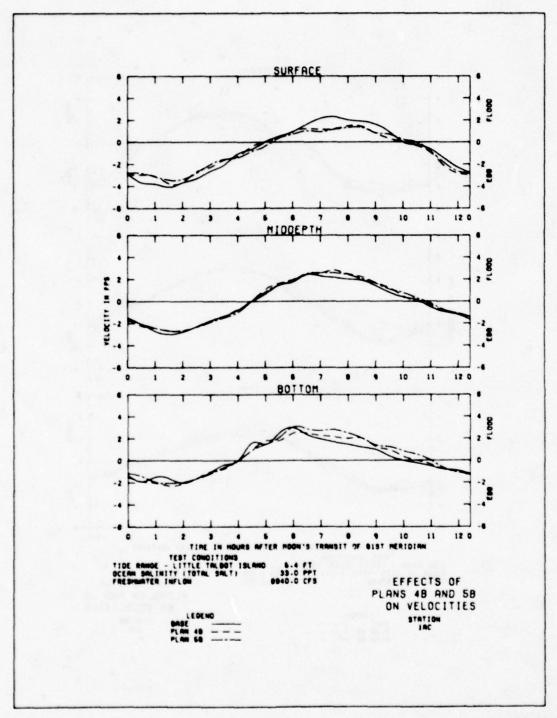


PLATE 55

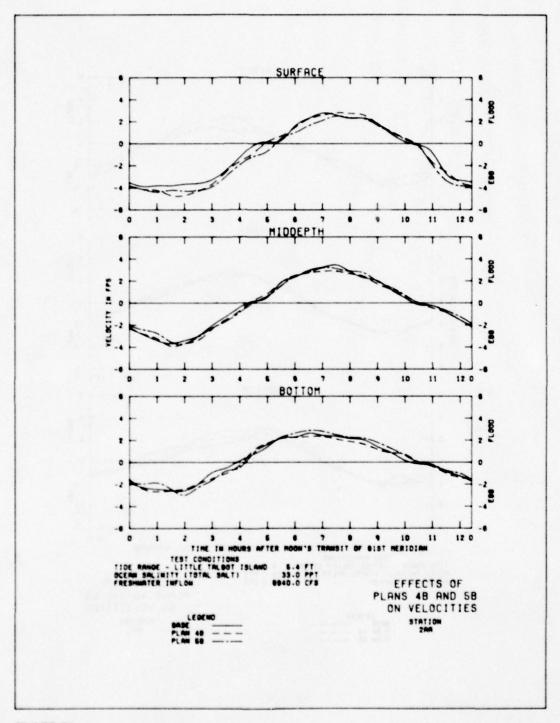
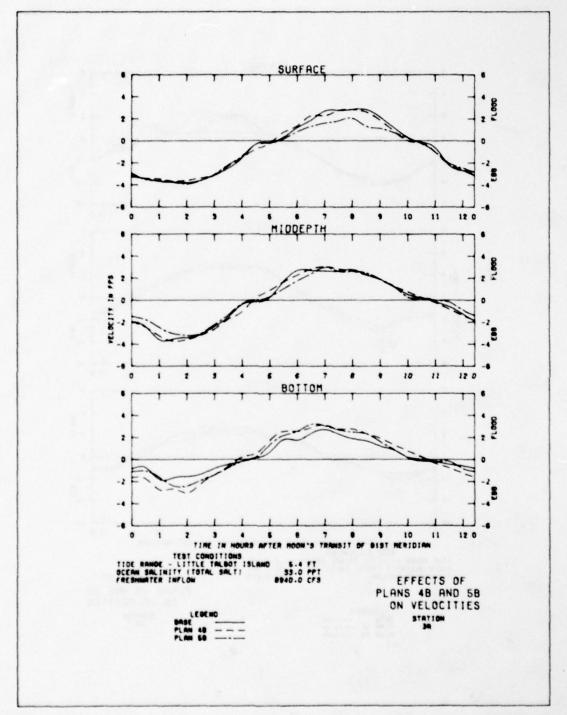


PLATE 56



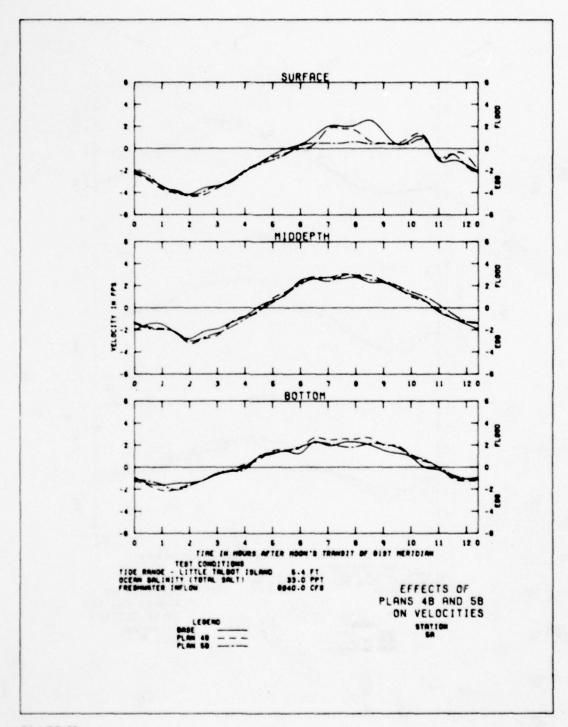


PLATE 58

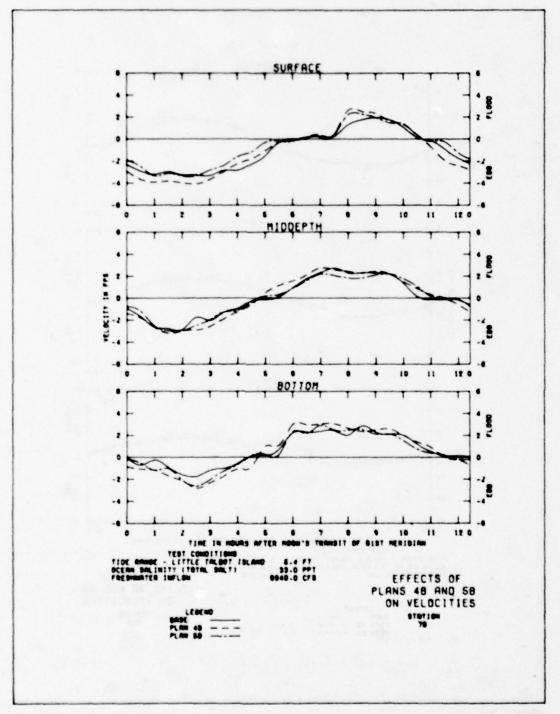


PLATE 59

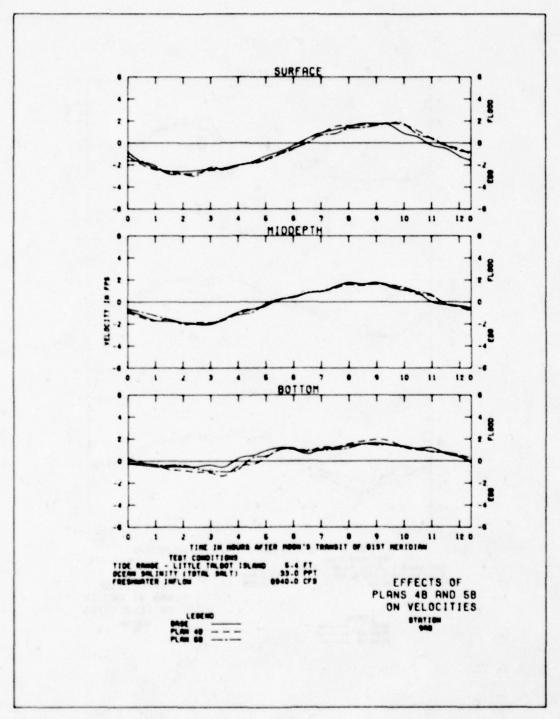


PLATE 60

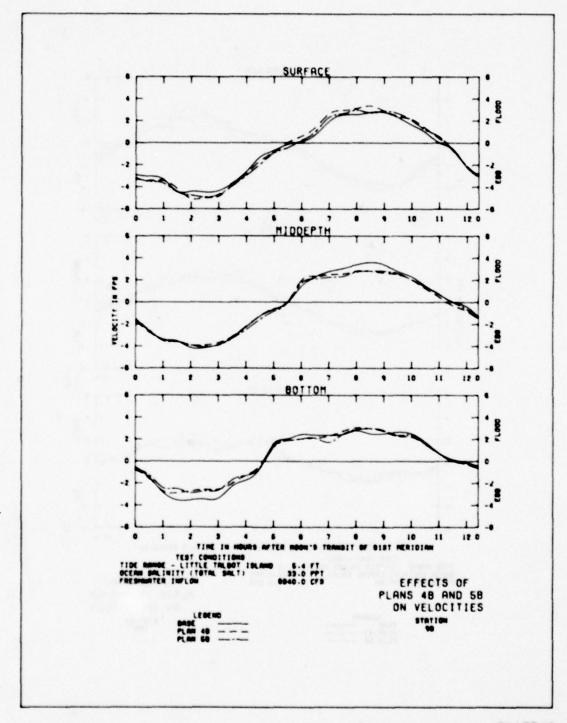


PLATE 61

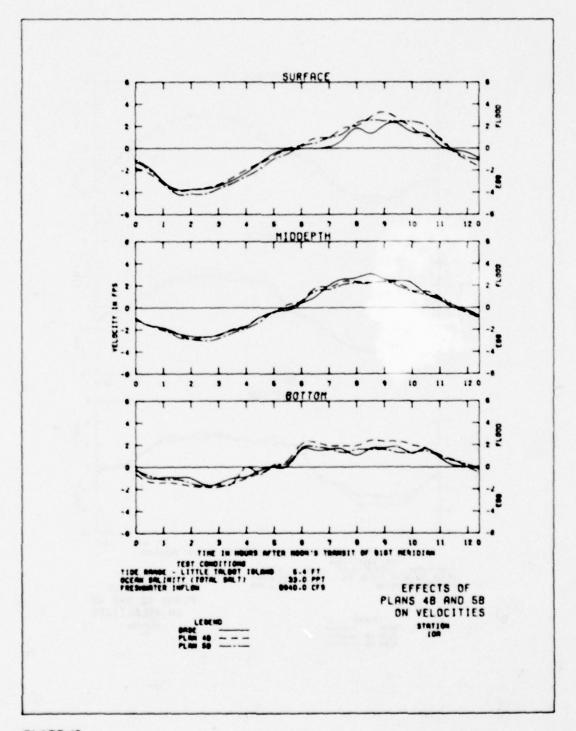


PLATE 62

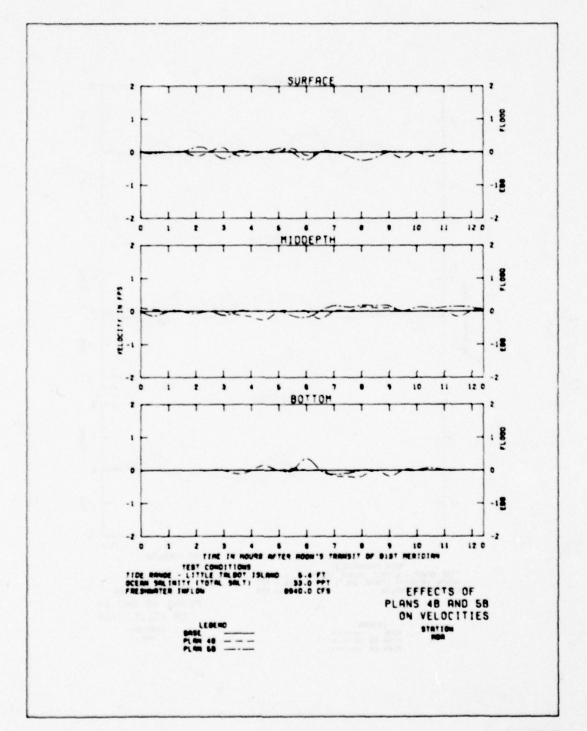


PLATE 63

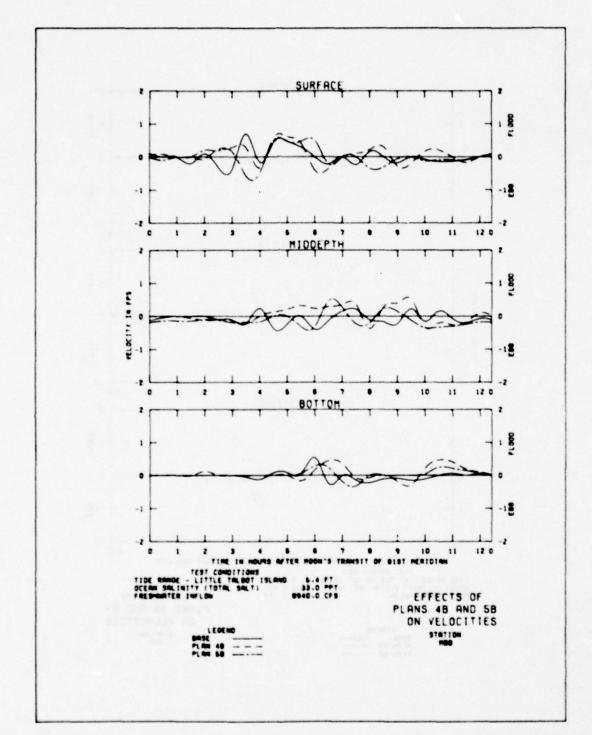


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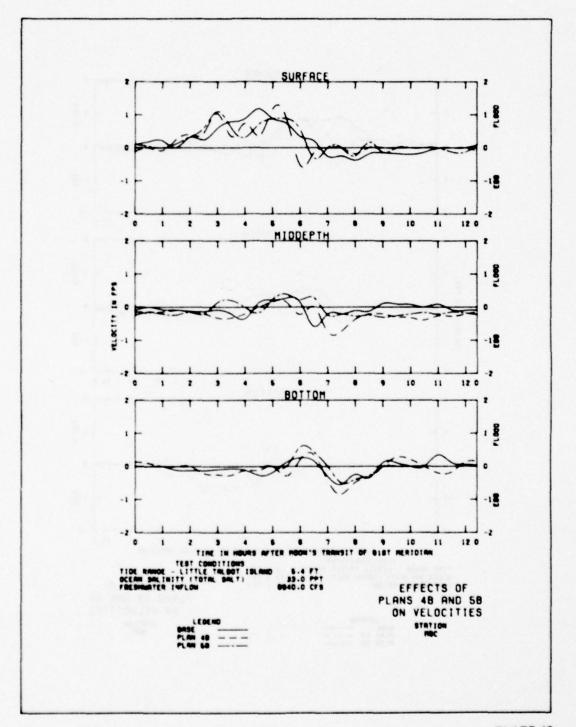


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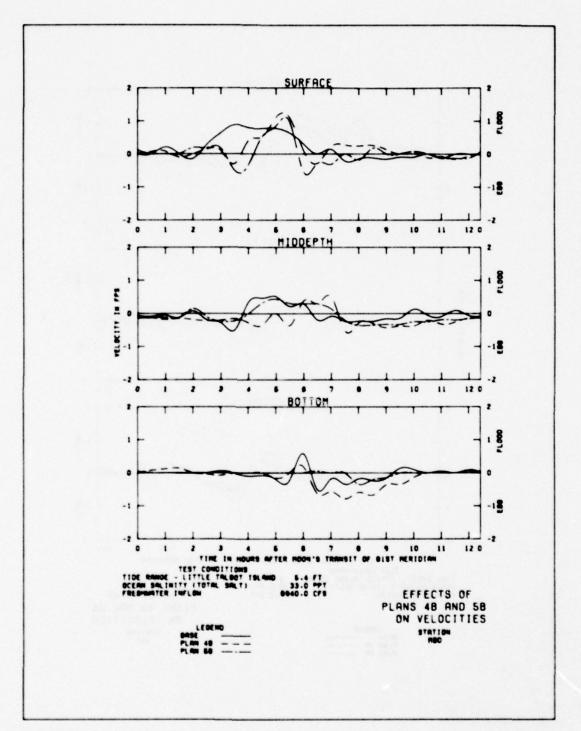


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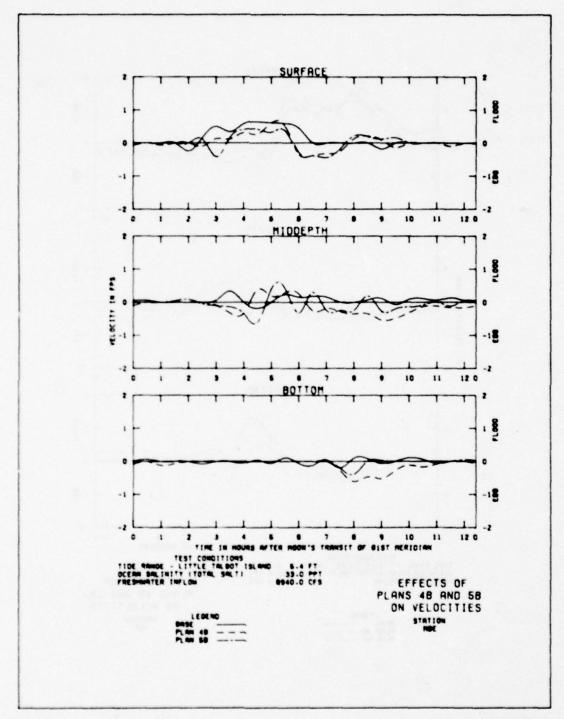


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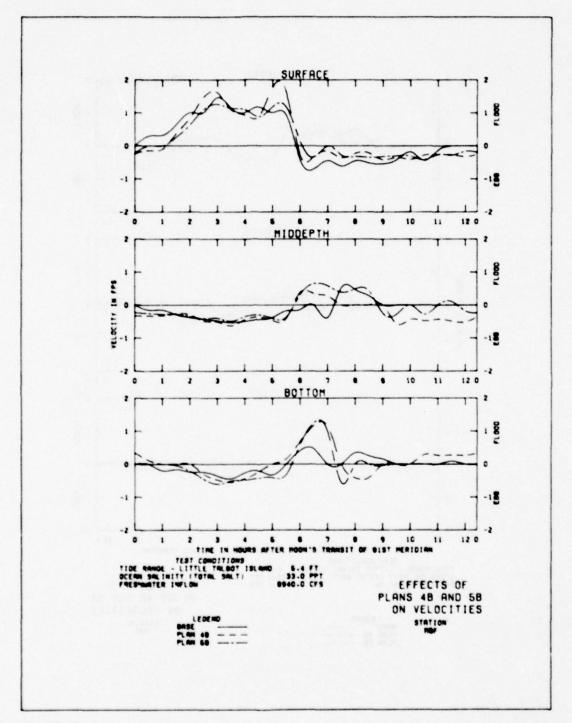


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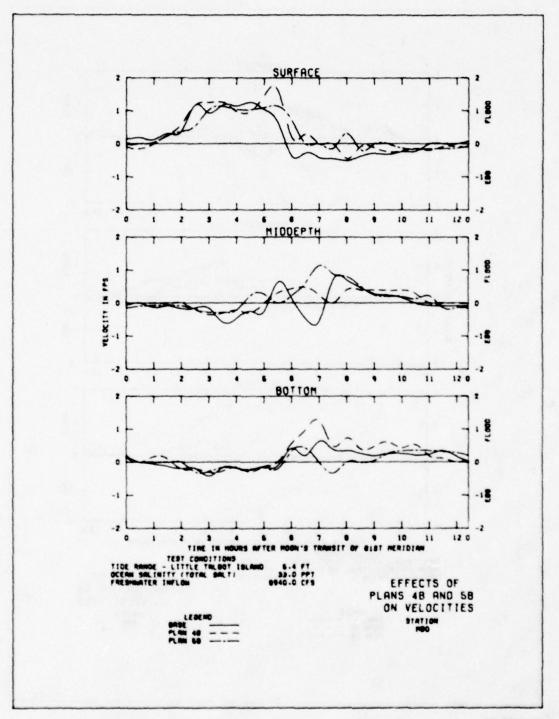


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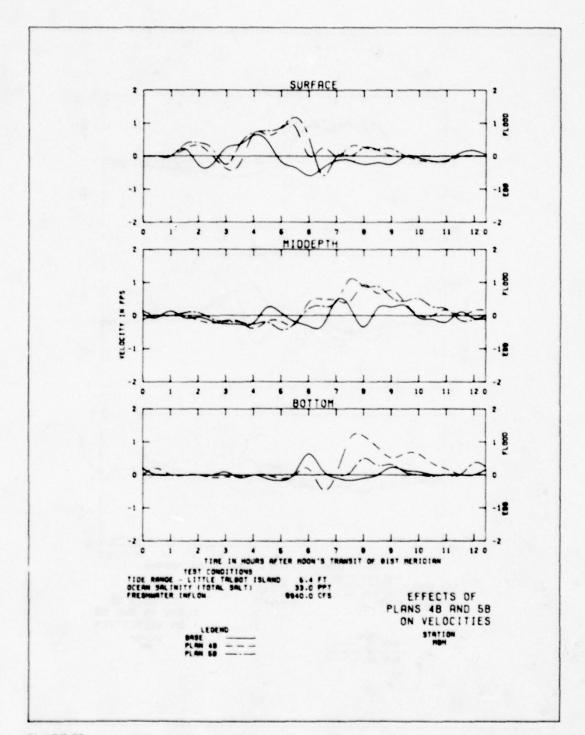


PLATE 70

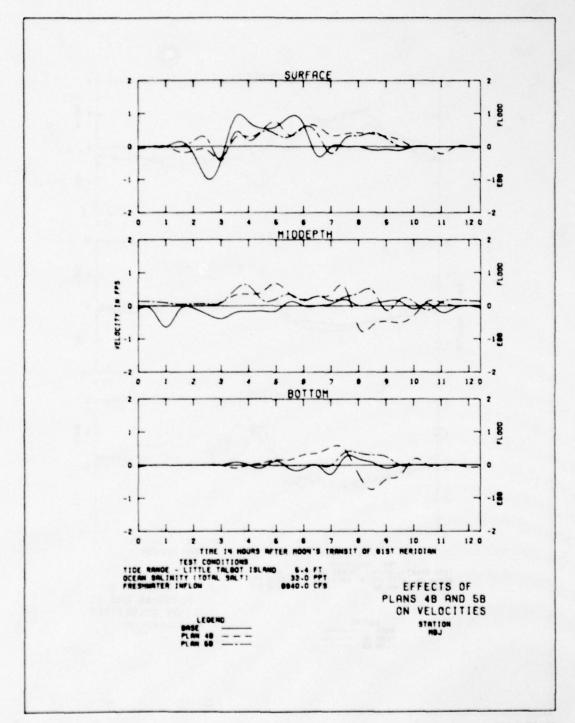


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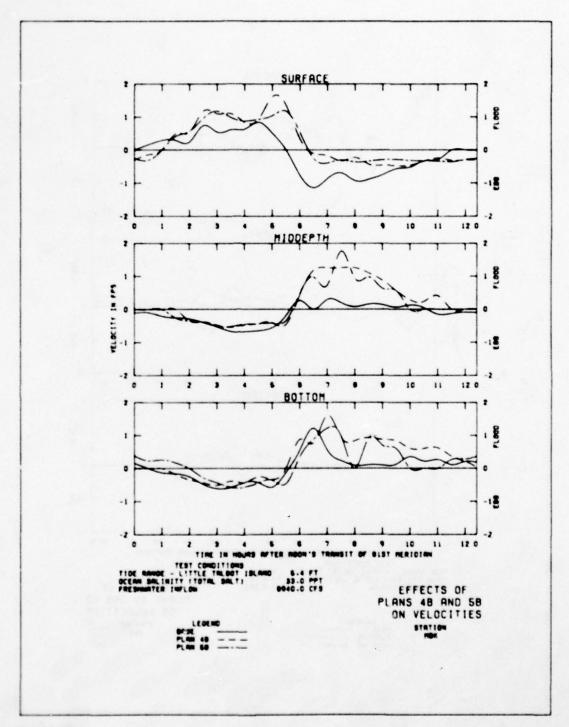


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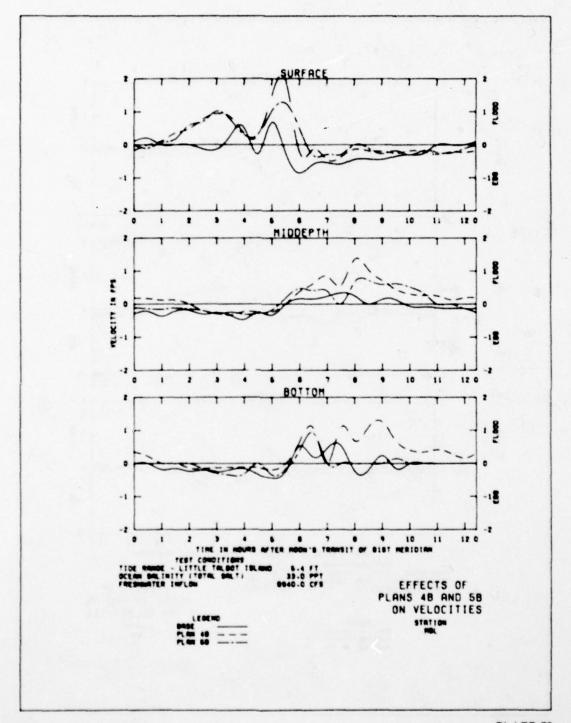


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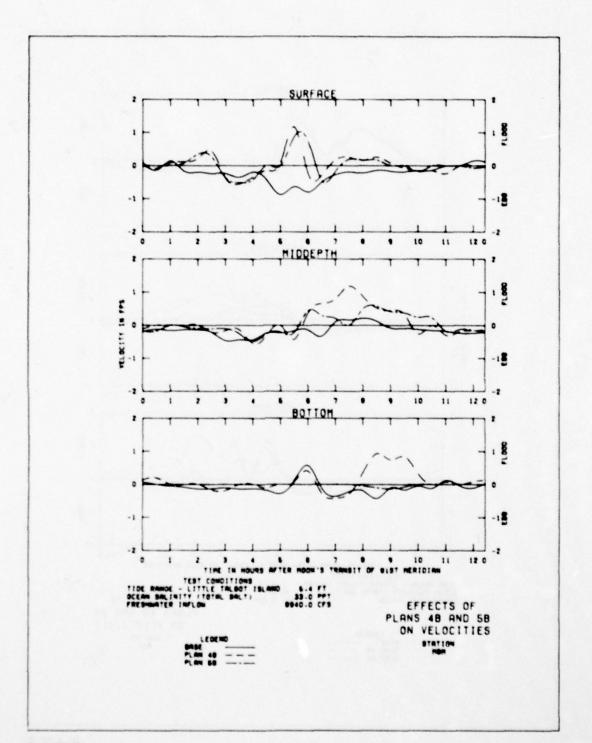


PLATE 74

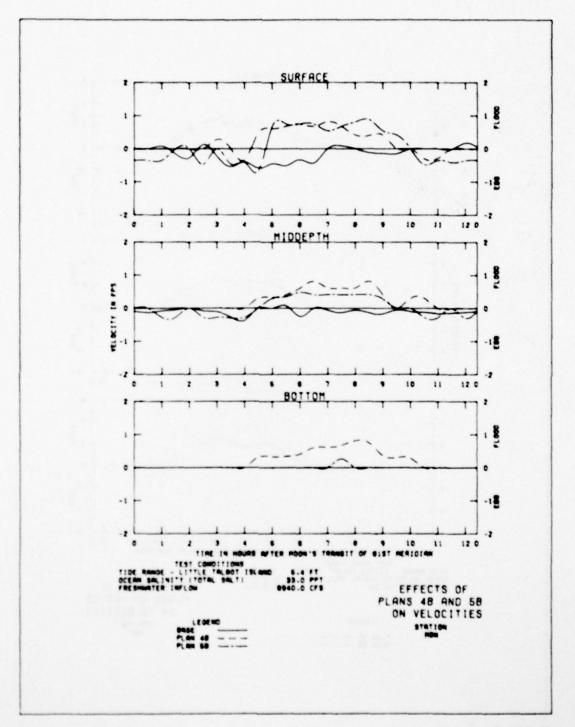


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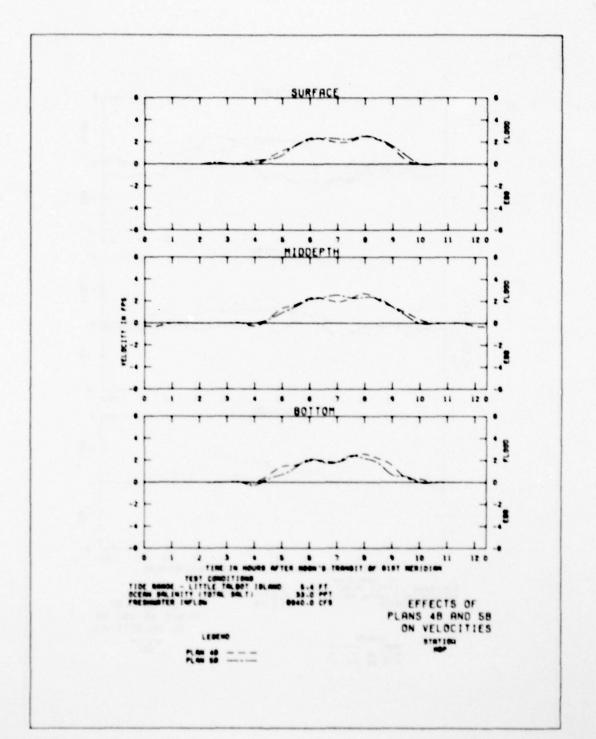


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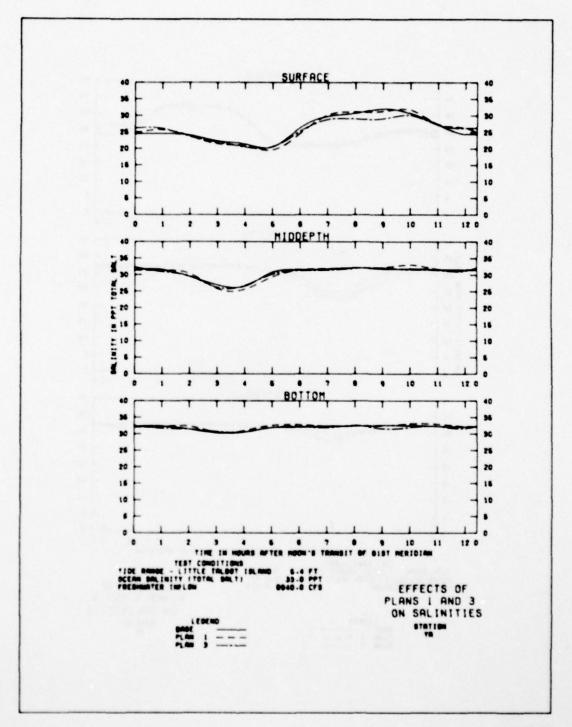


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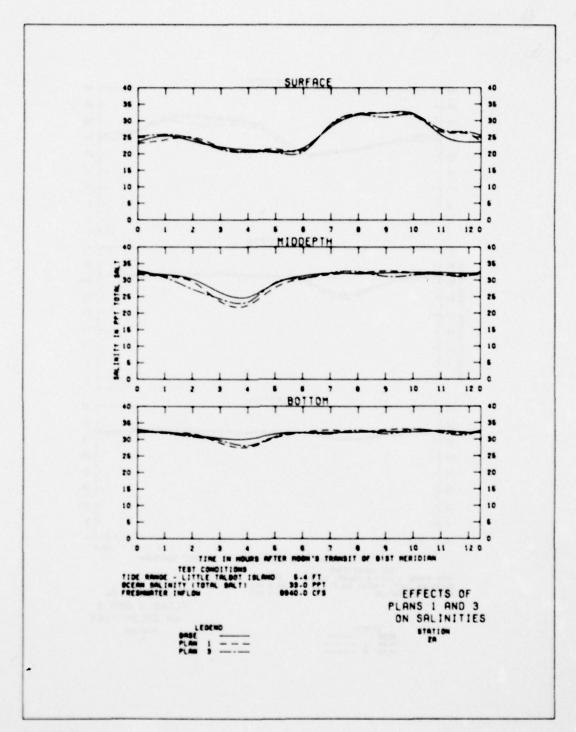


PLATE 78

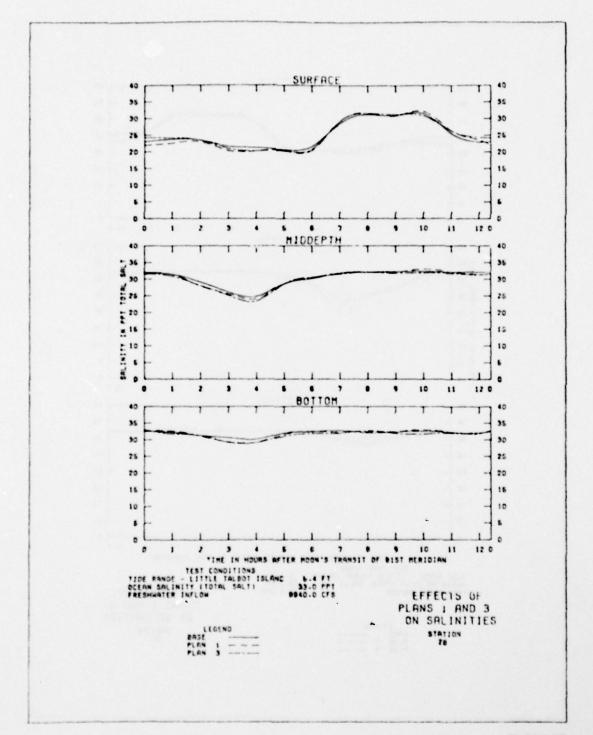


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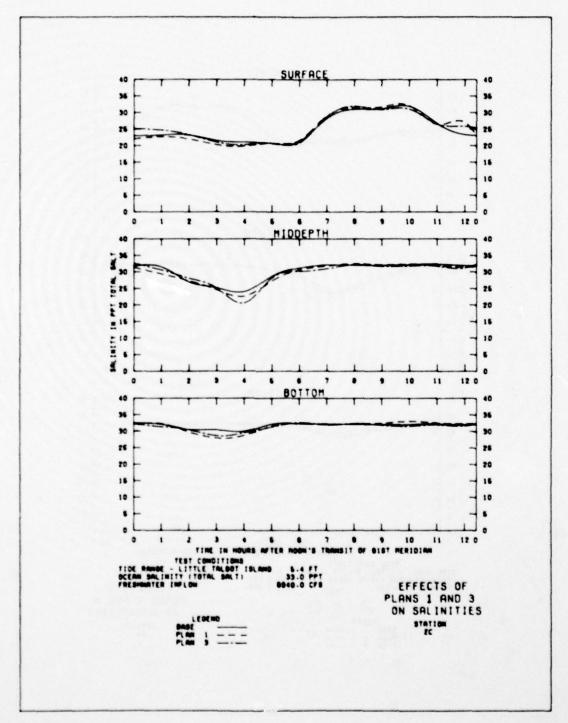


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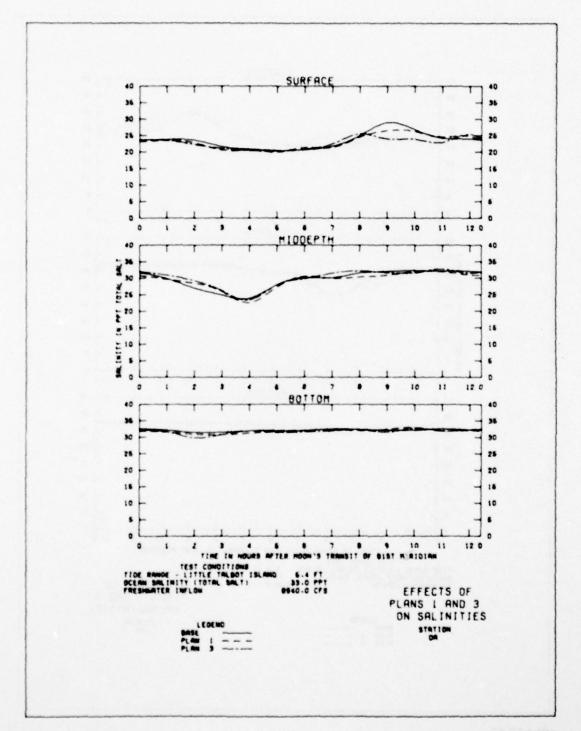


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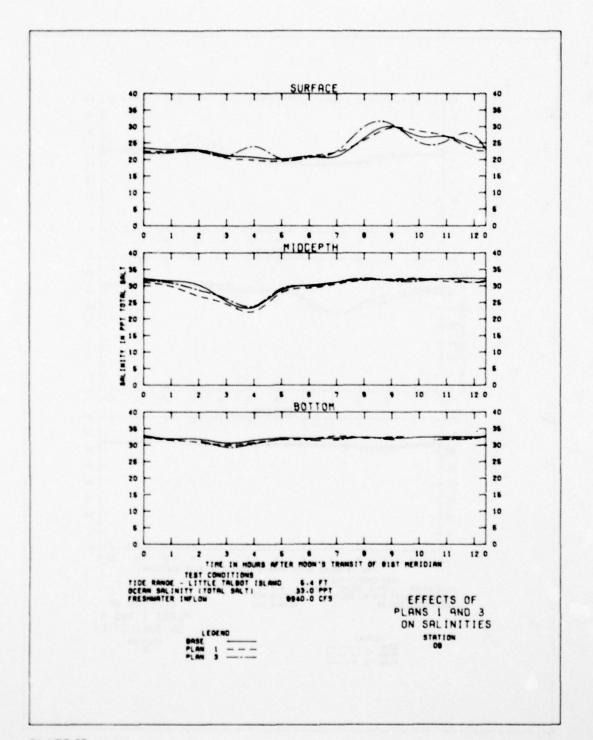


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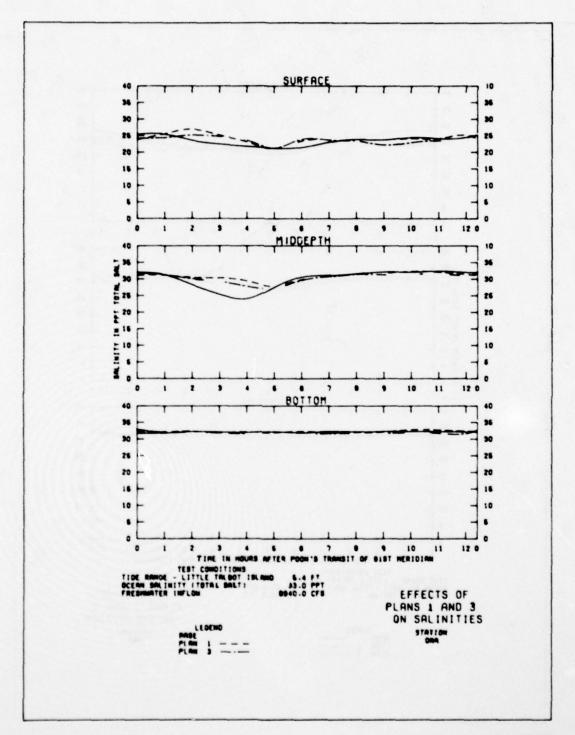


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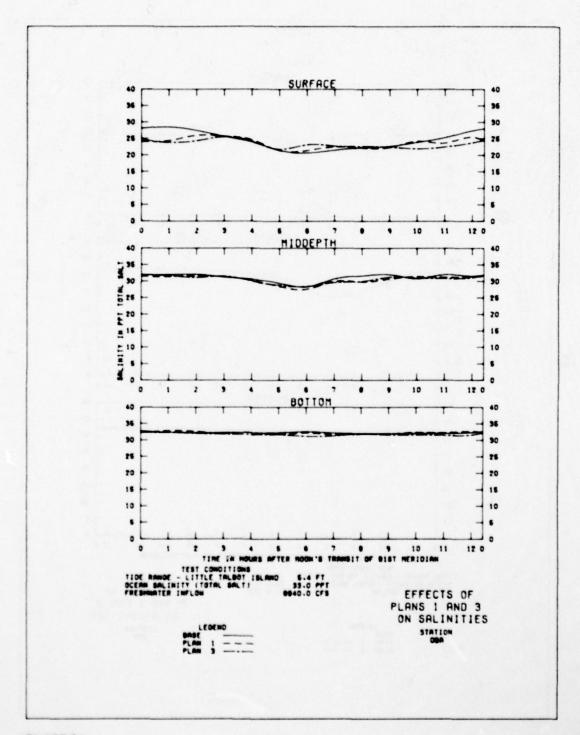


PLATE 84

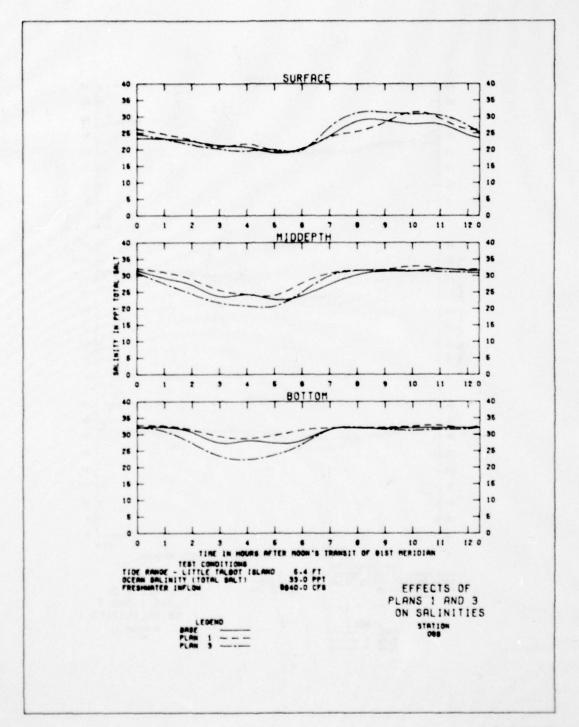
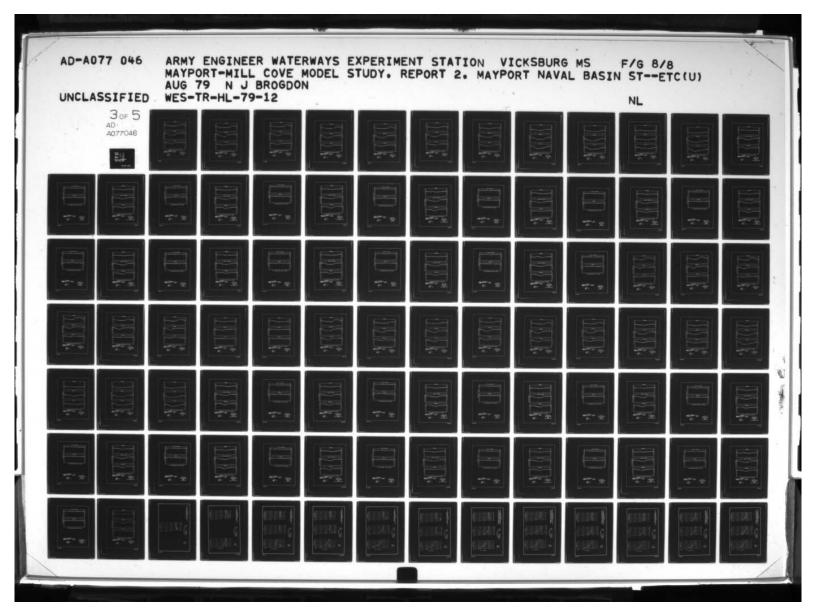


PLATE 85



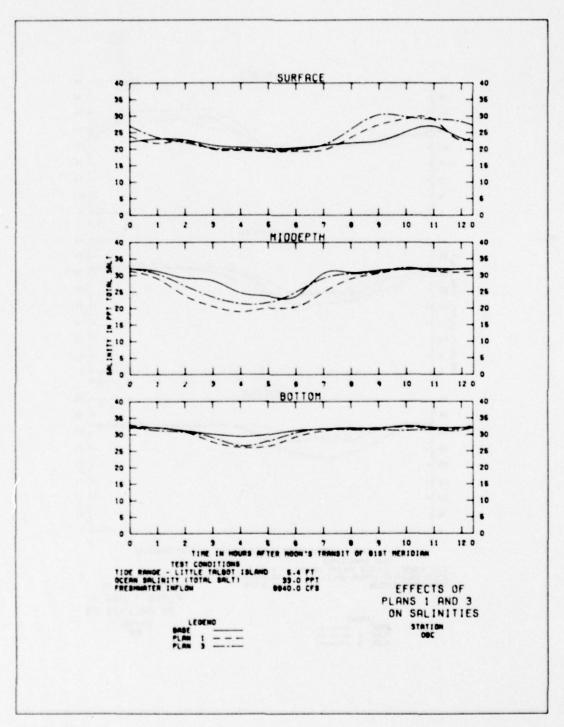
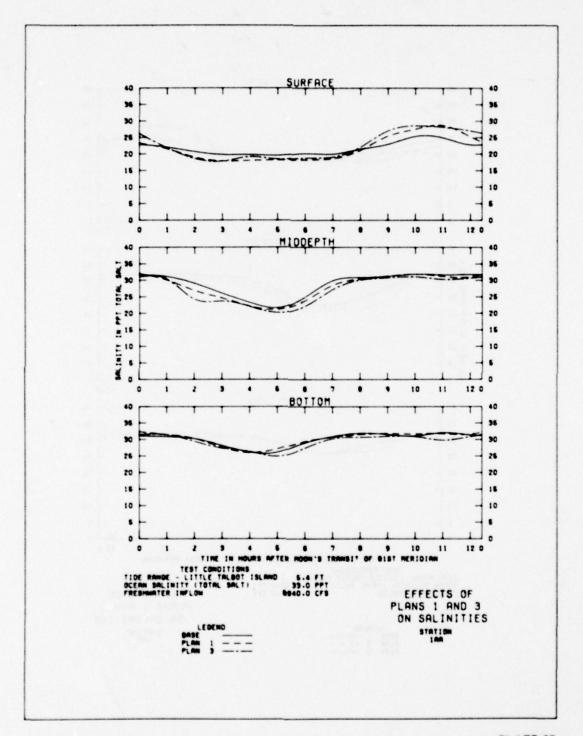
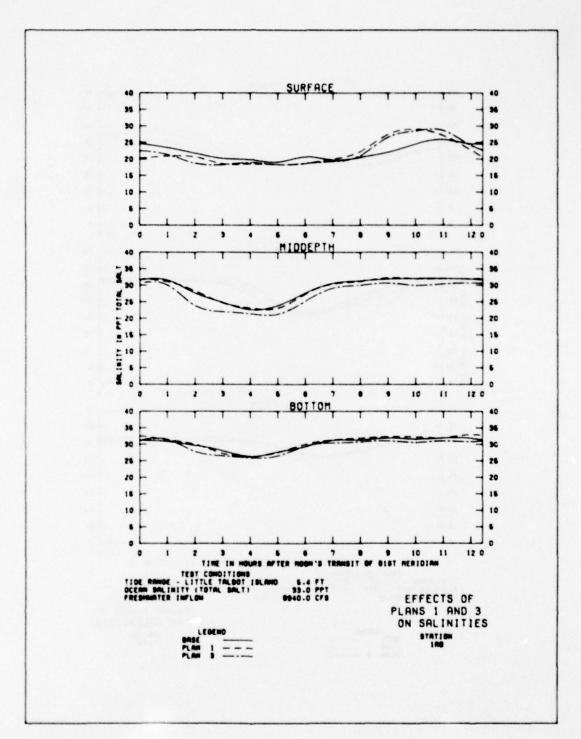


PLATE 86





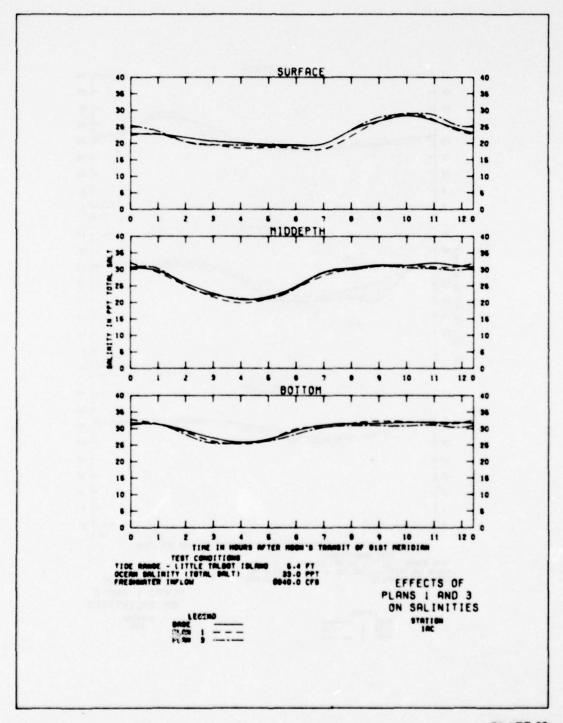


PLATE 89

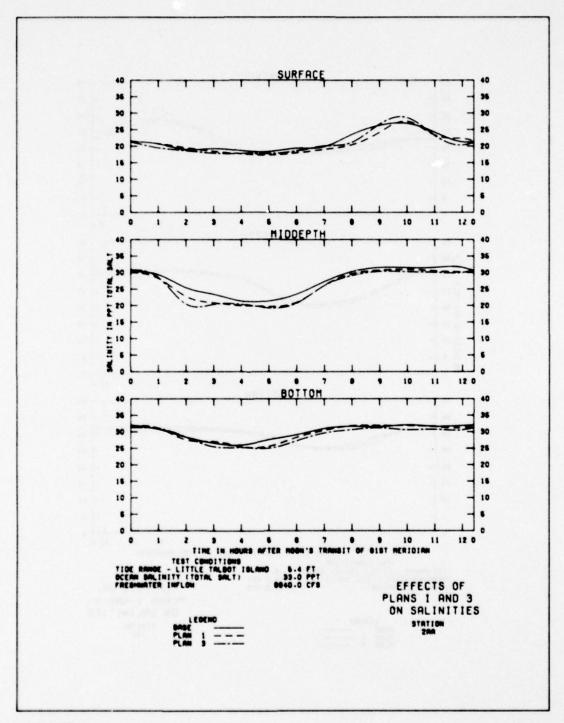


PLATE 90

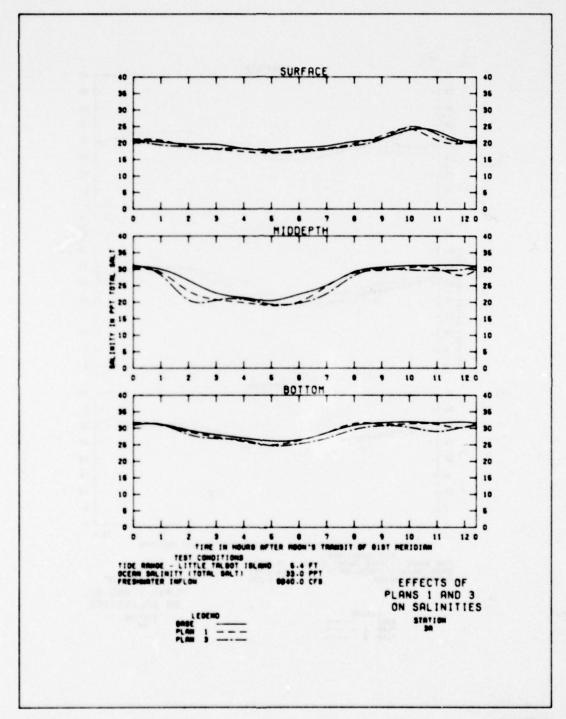


PLATE 91

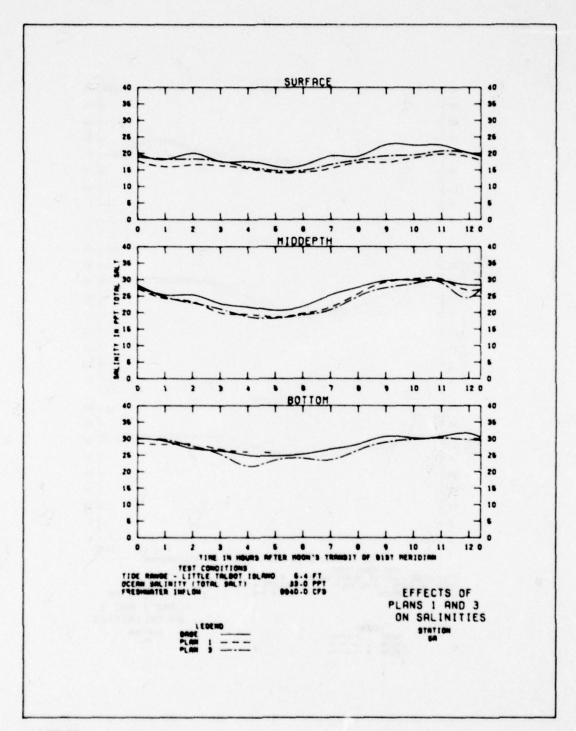


PLATE 92

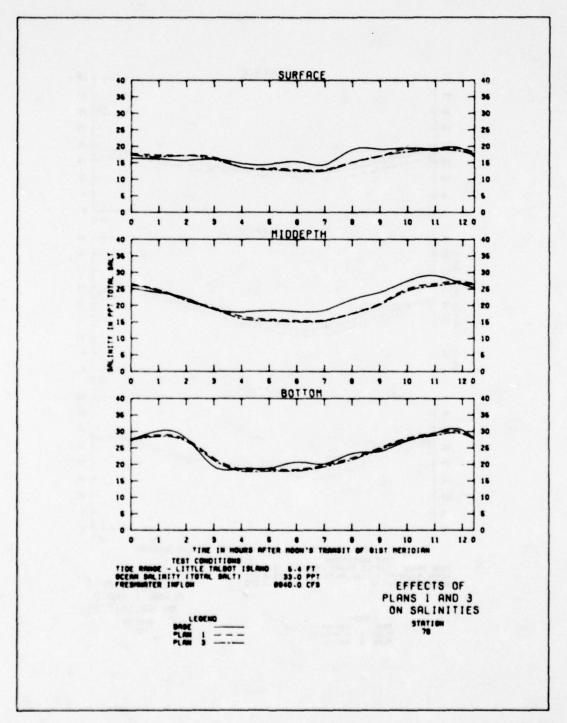


PLATE 93

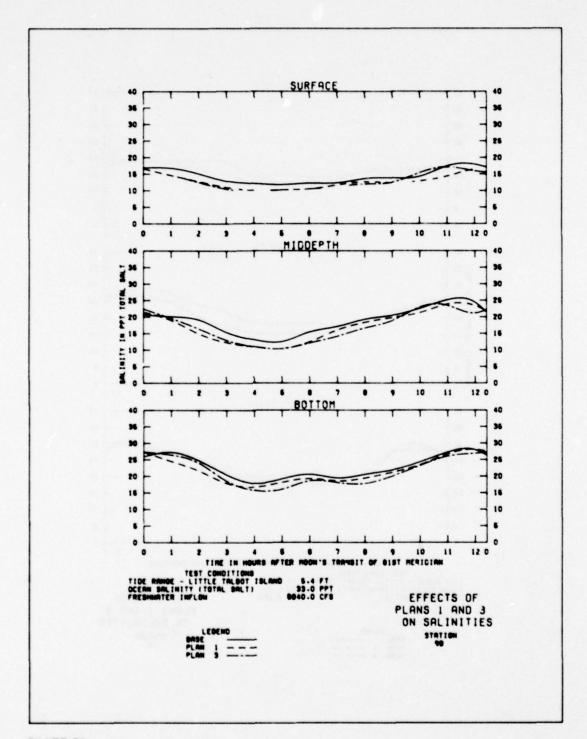
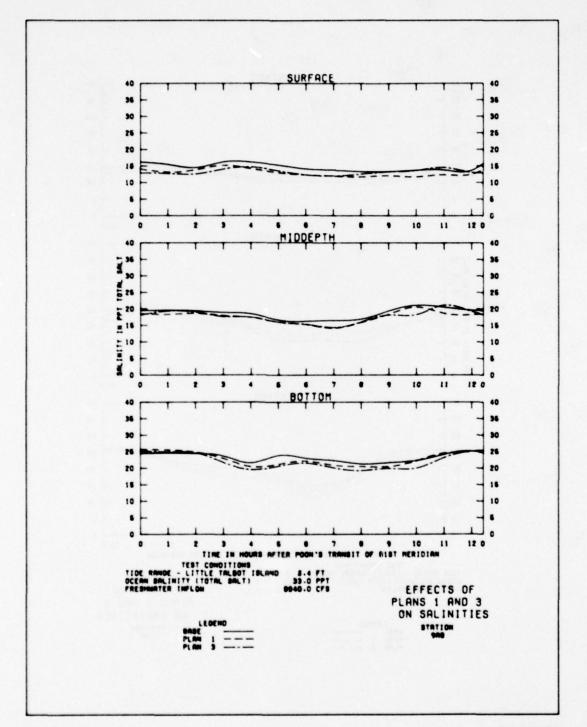


PLATE 94



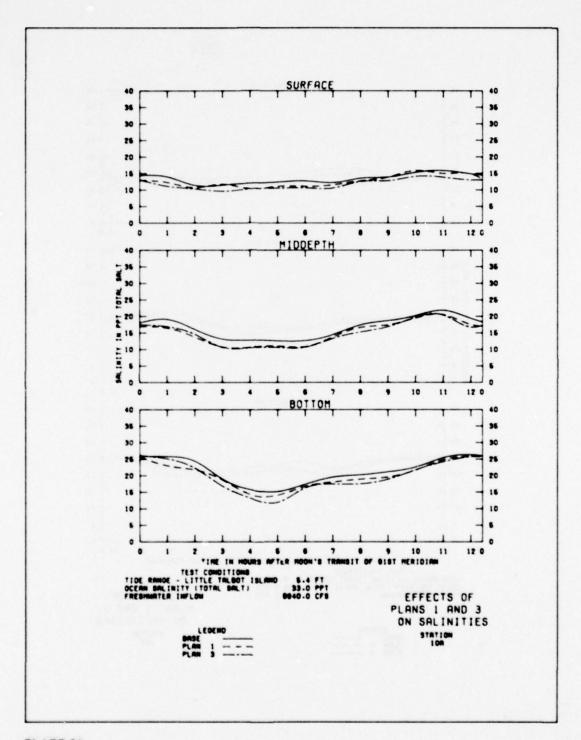


PLATE 96

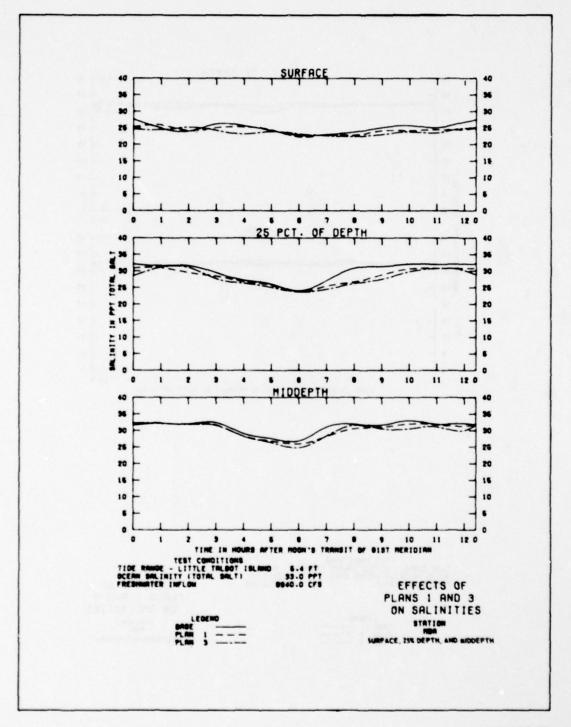
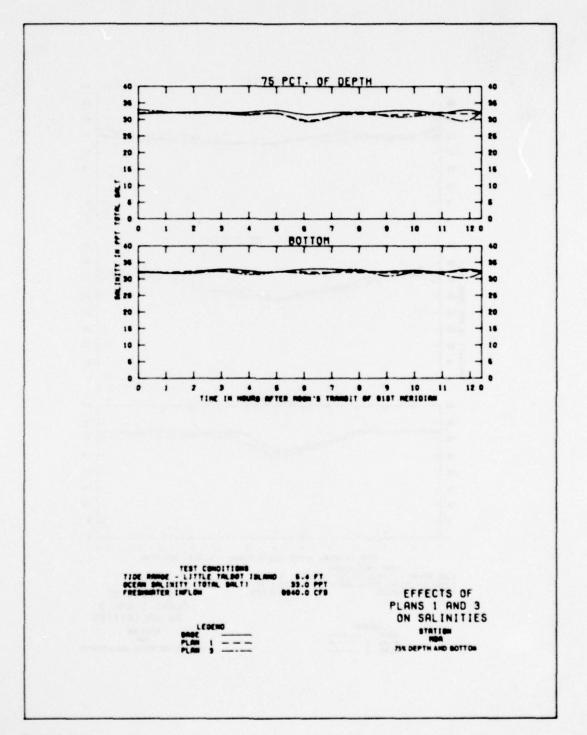


PLATE 97



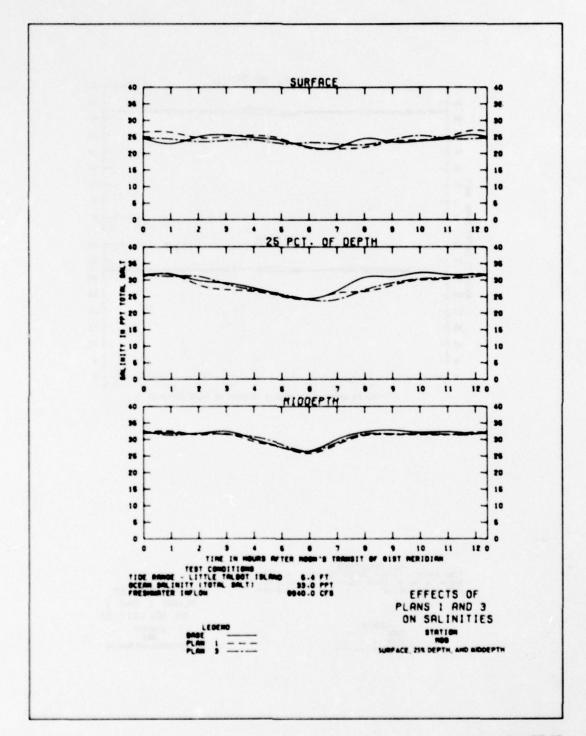
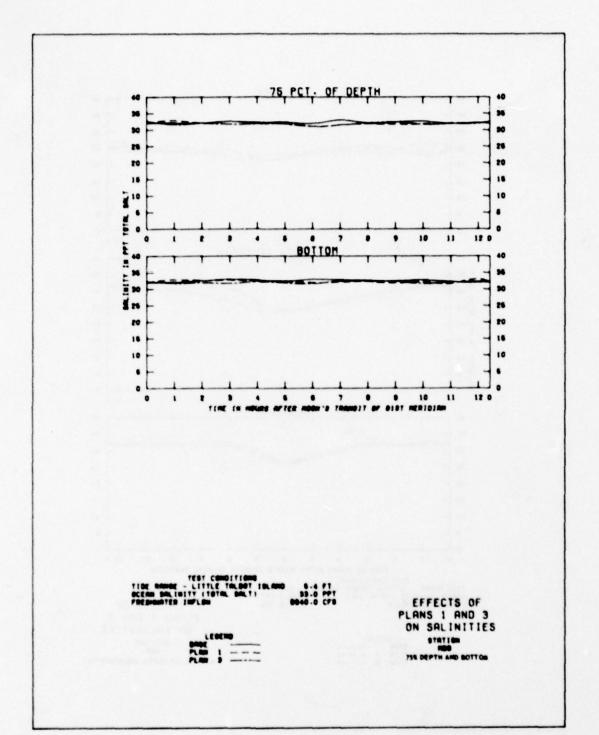


PLATE 99



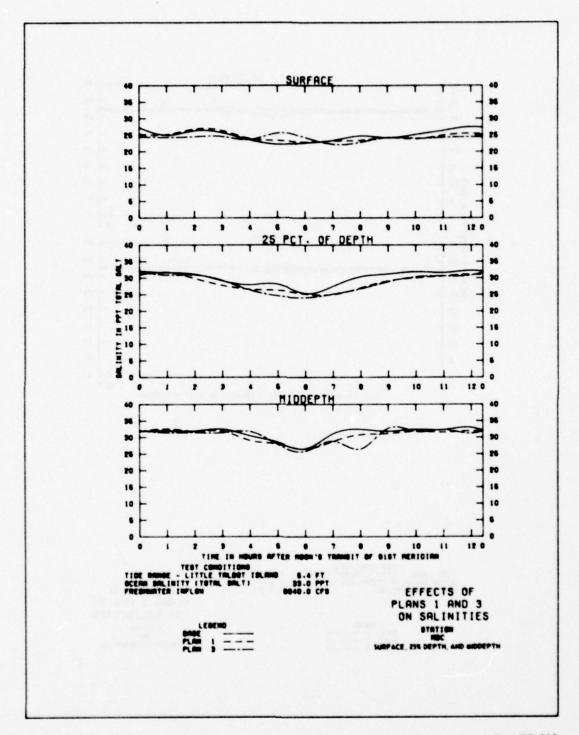


PLATE 101

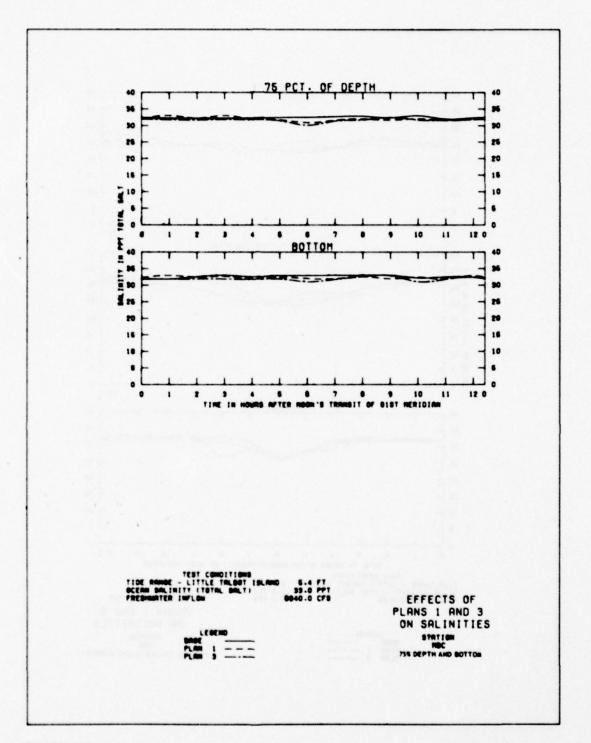


PLATE 102

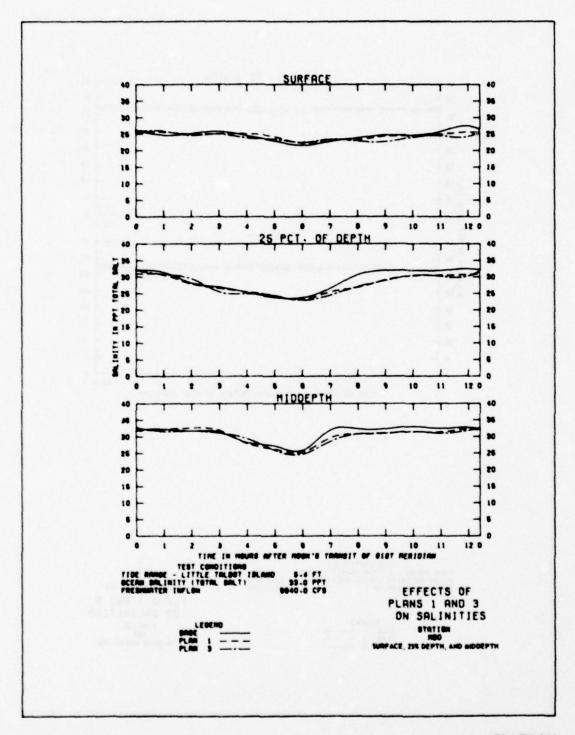
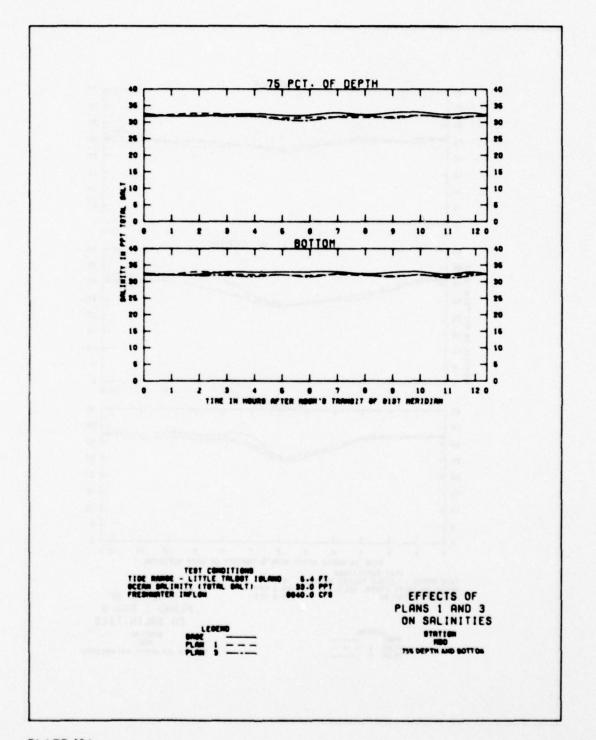


PLATE 103



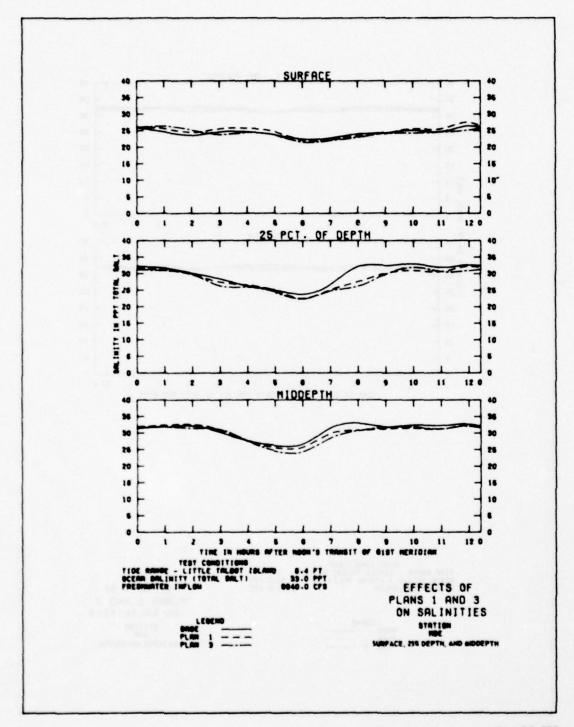


PLATE 105

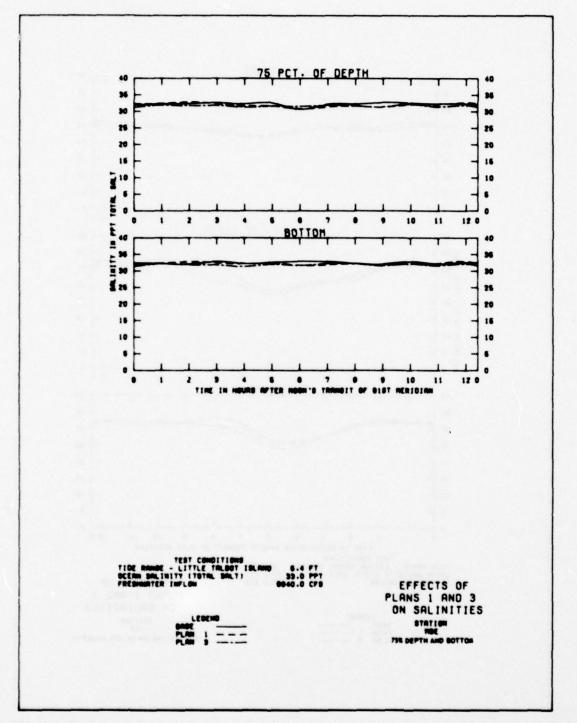
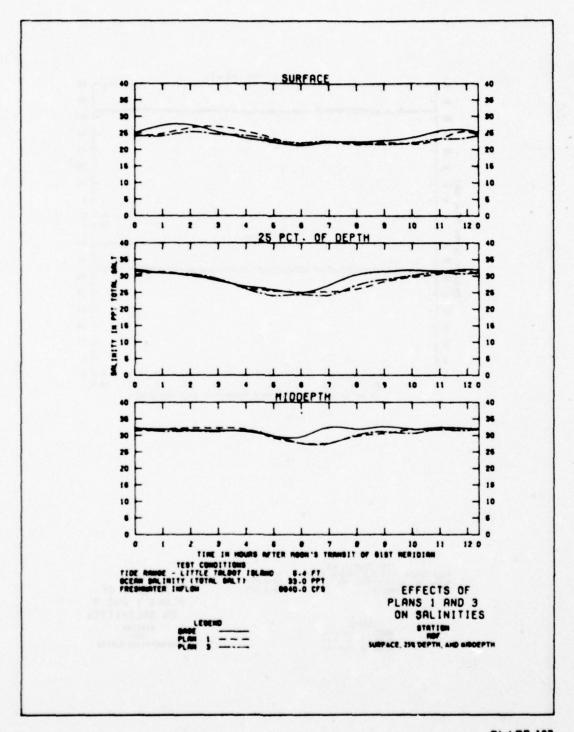
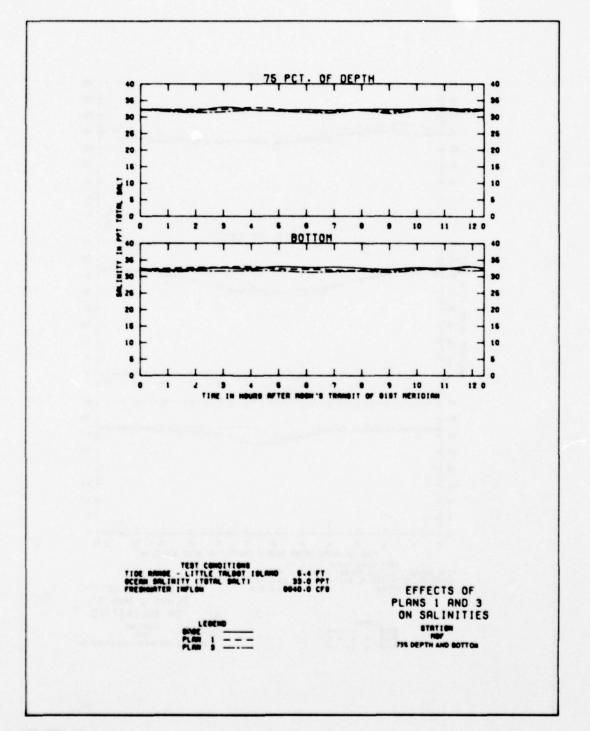


PLATE 106





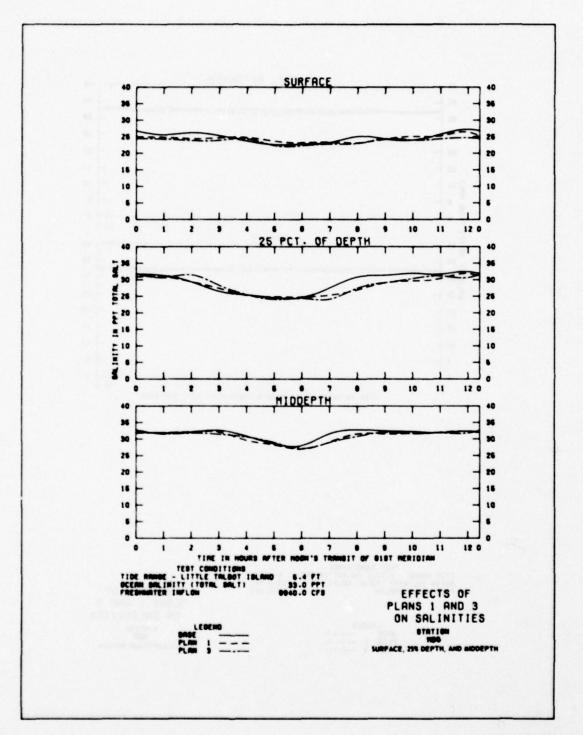


PLATE 109

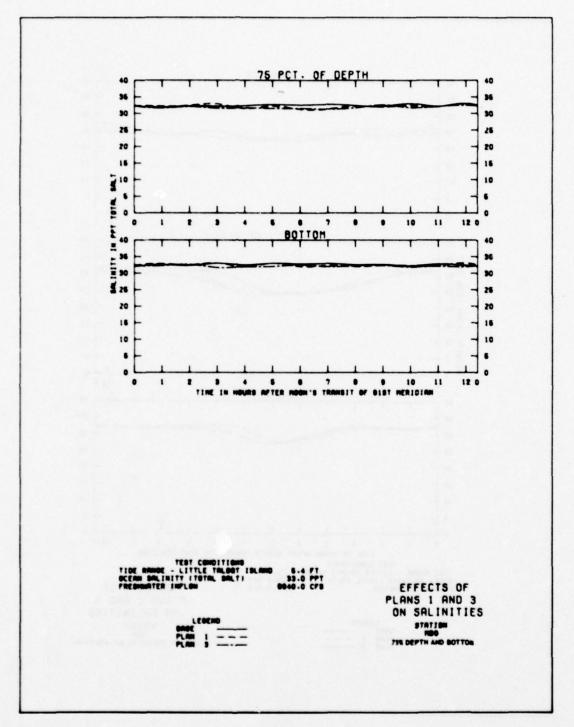


PLATE 110

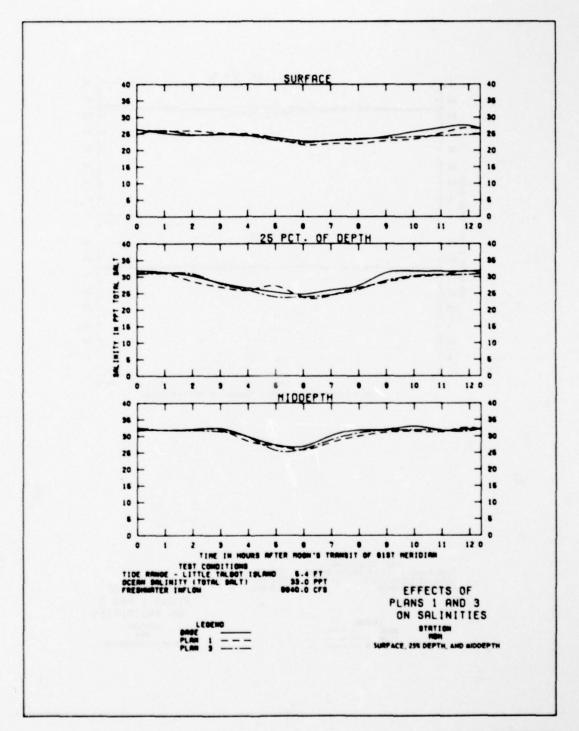
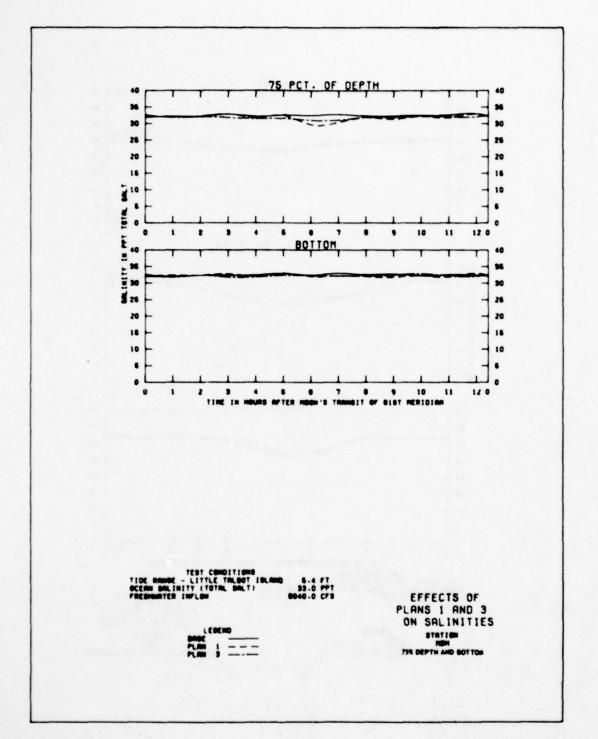


PLATE 111



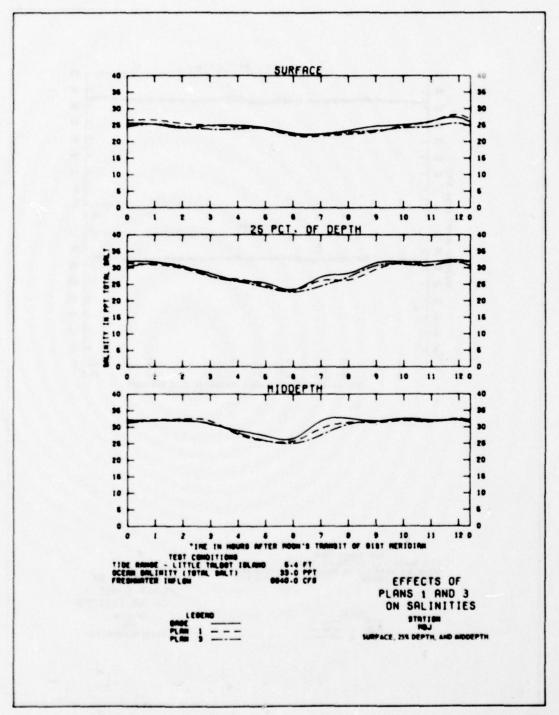


PLATE 113

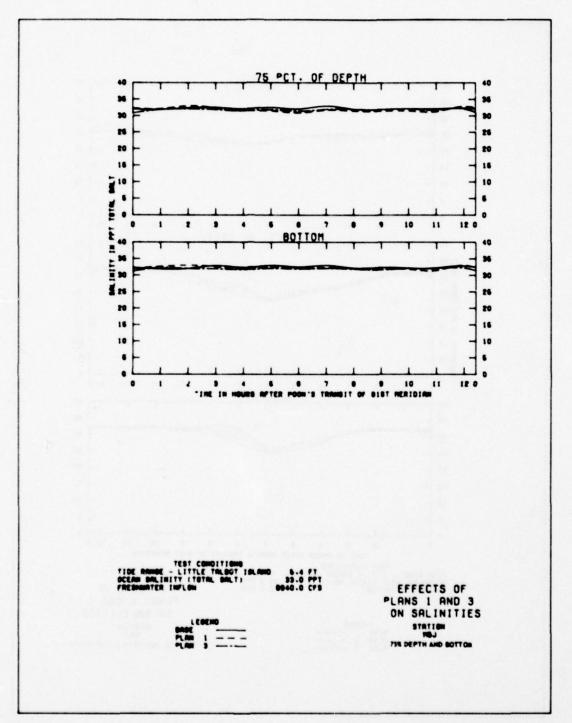


PLATE 114

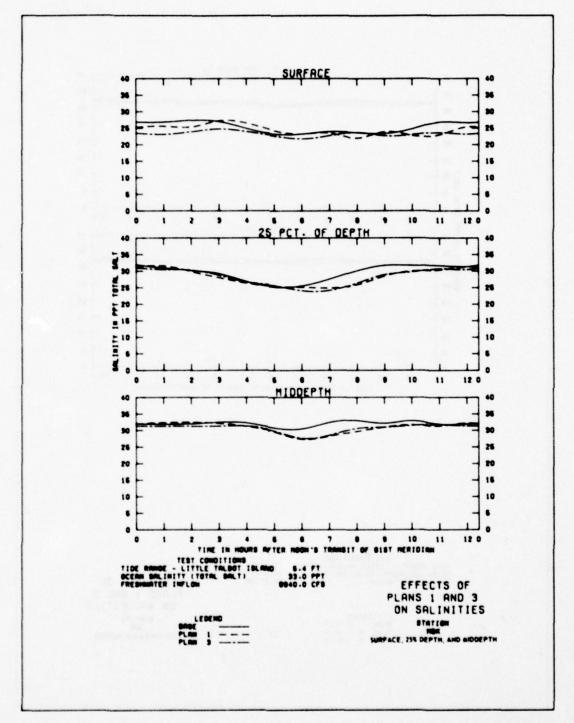
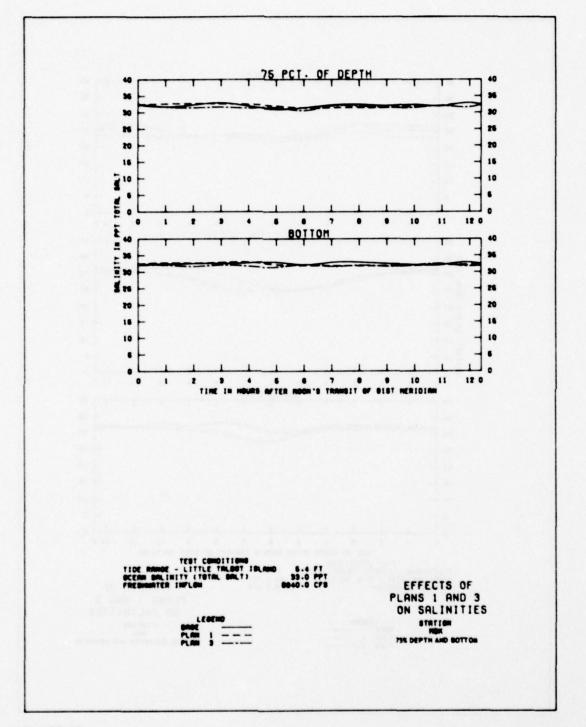


PLATE 115



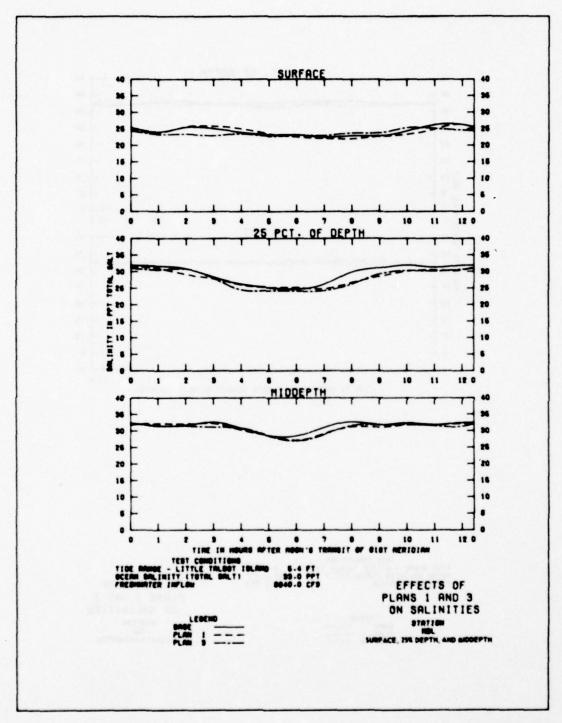


PLATE 117

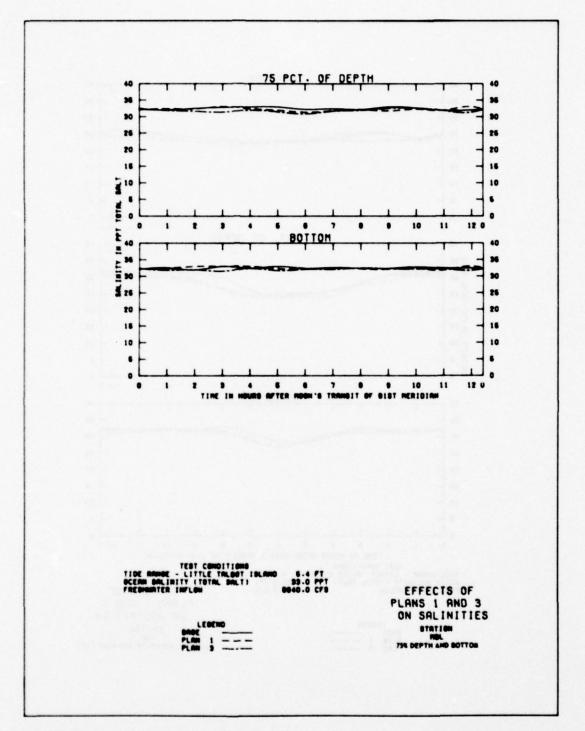


PLATE 118

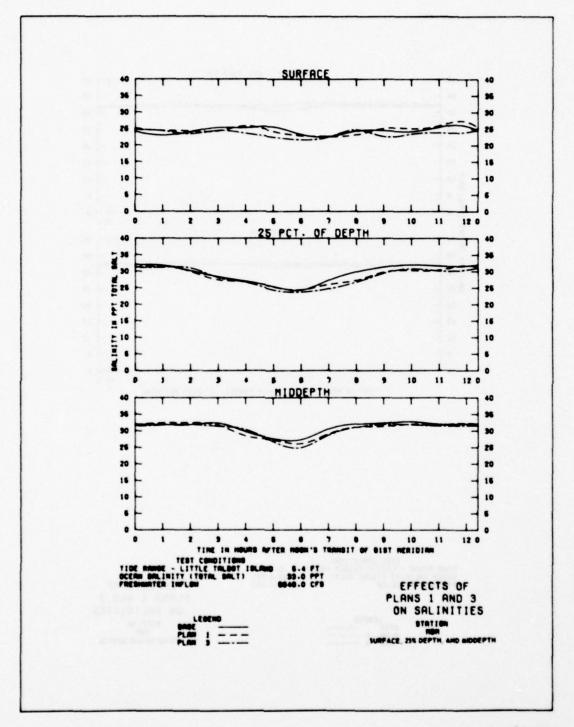


PLATE 119

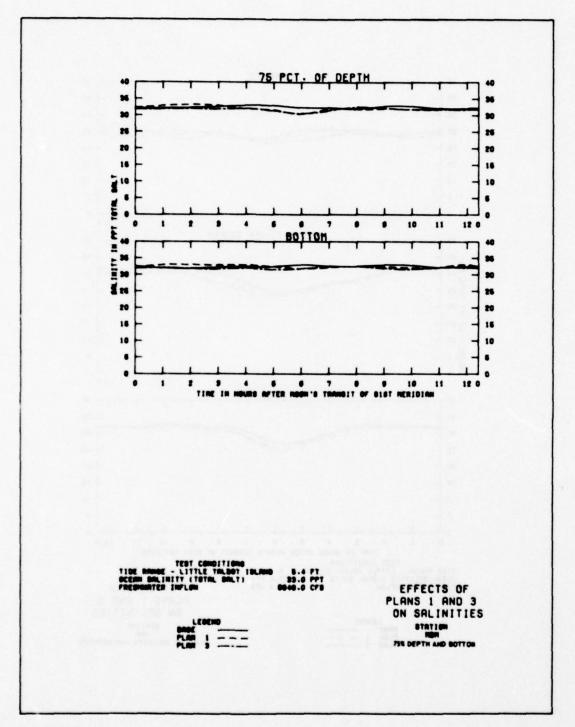


PLATE 120

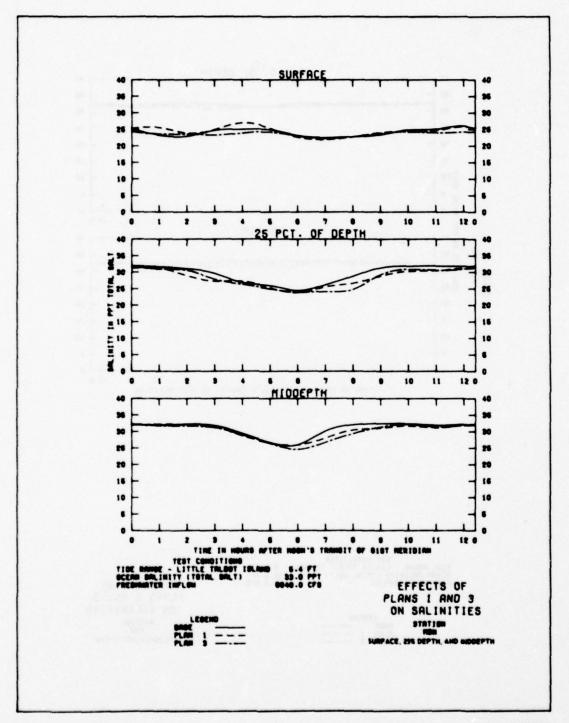
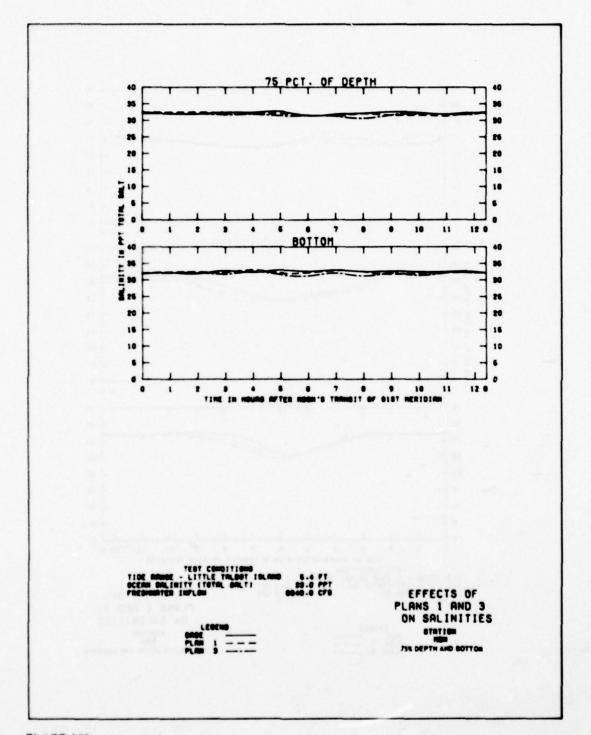


PLATE 121



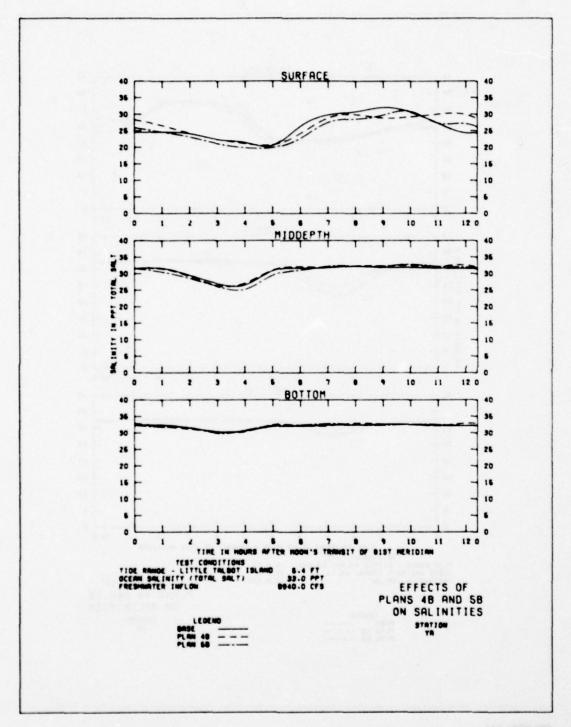


PLATE 123

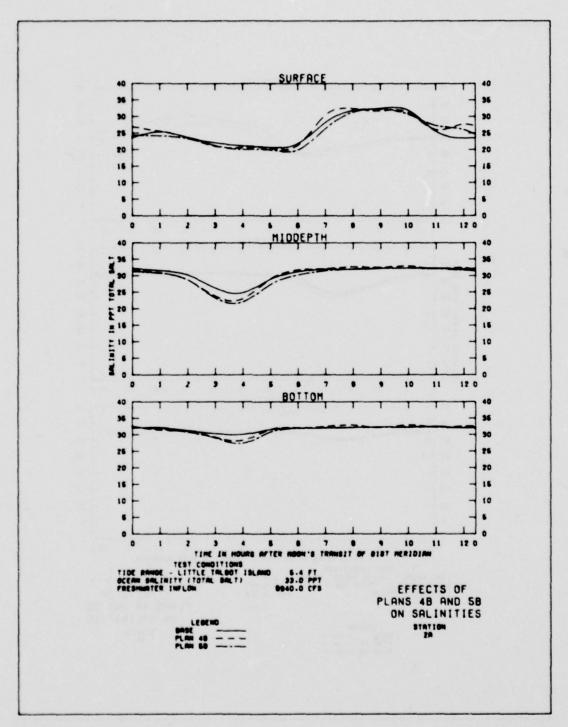
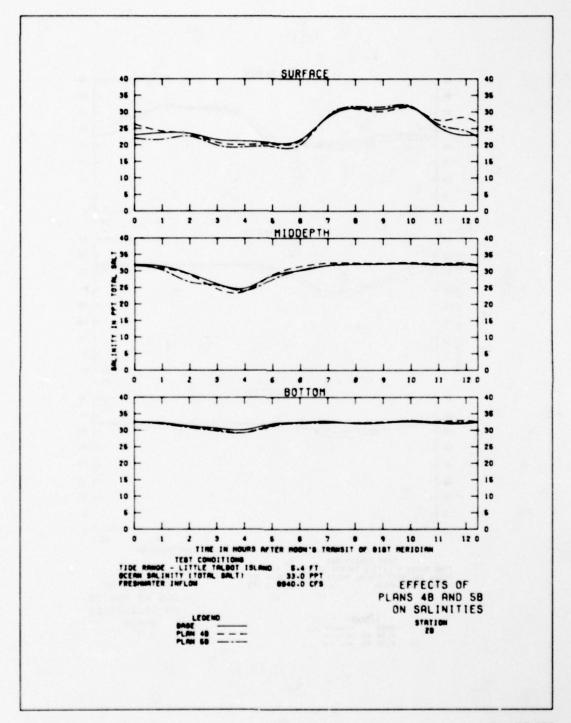


PLATE 124



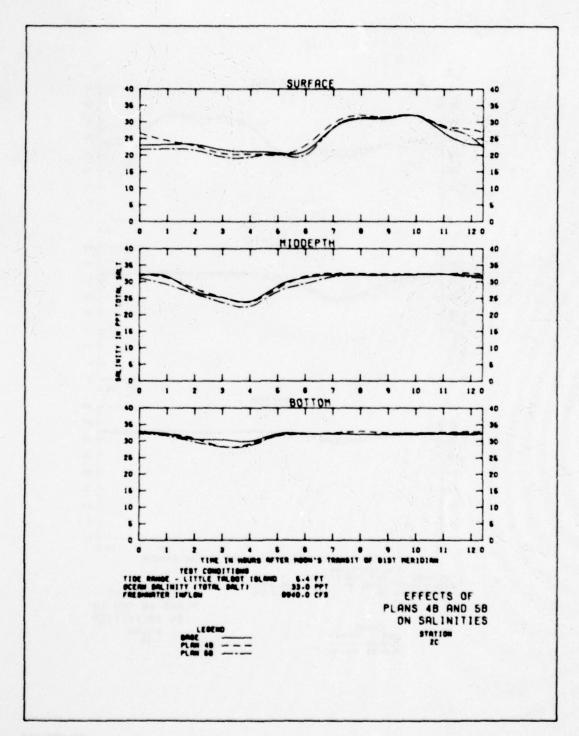


PLATE 126

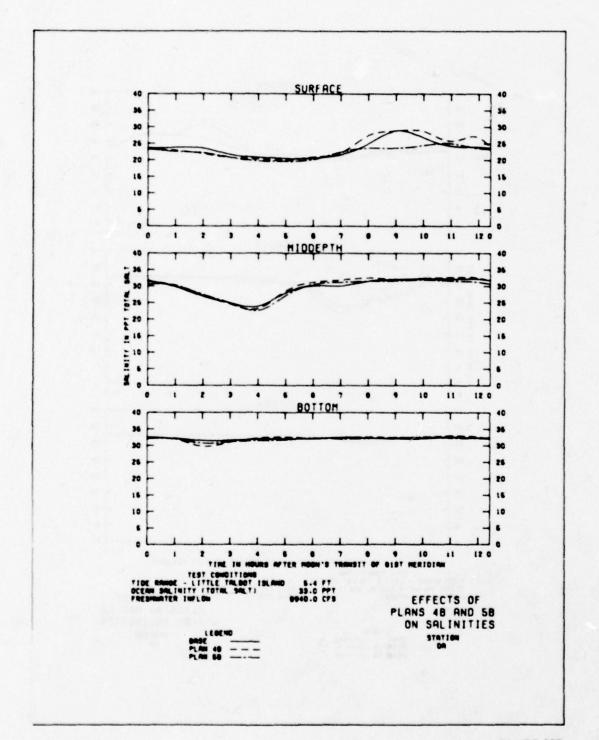


PLATE 127

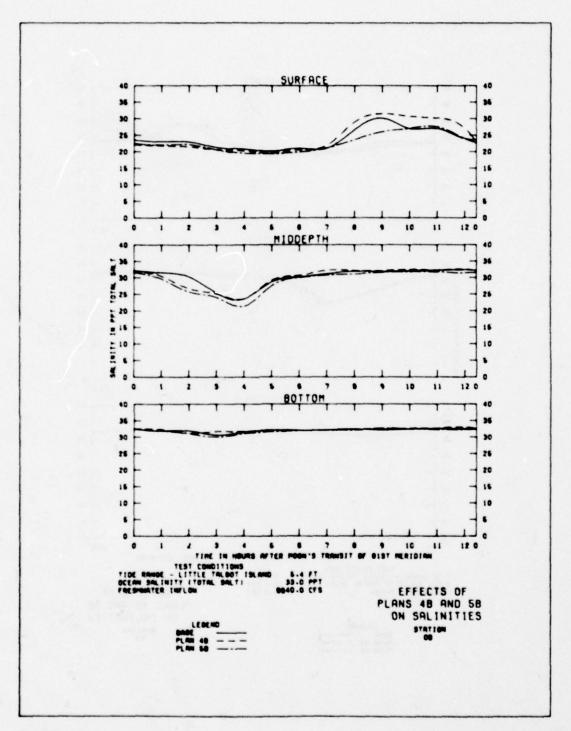


PLATE 128

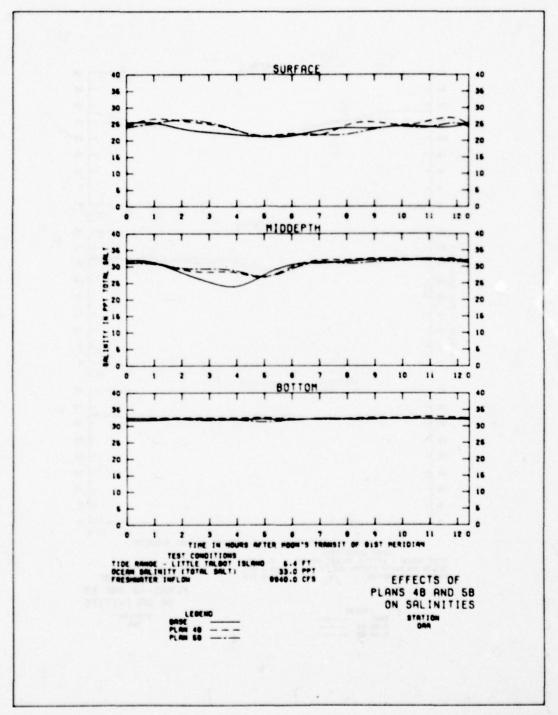


PLATE 129

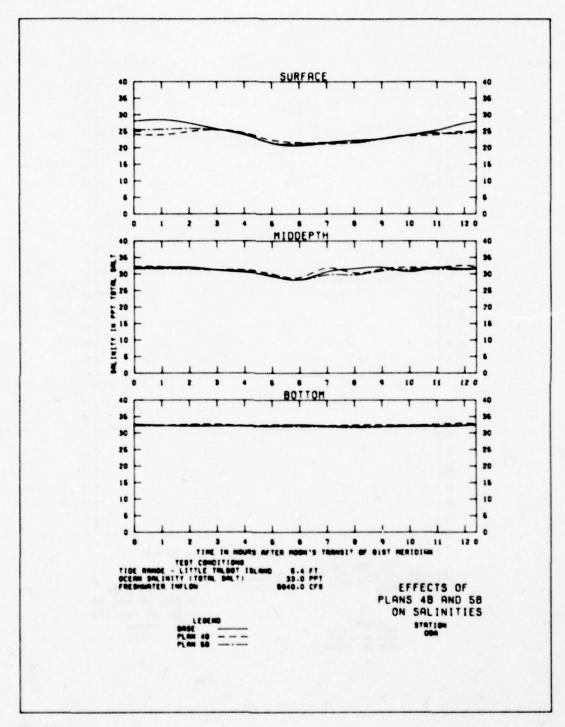


PLATE 130

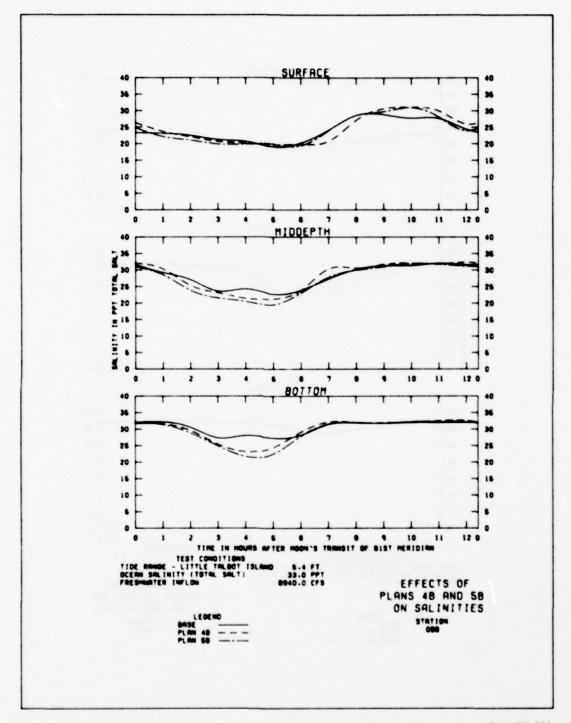


PLATE 131

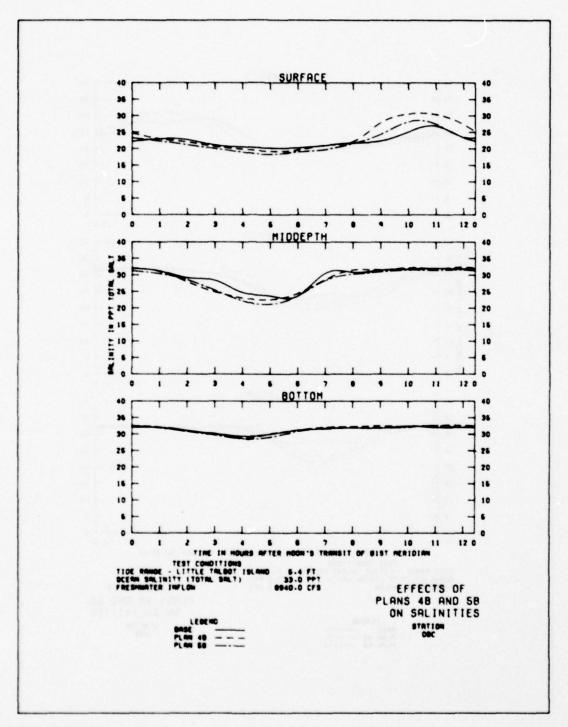


PLATE 132

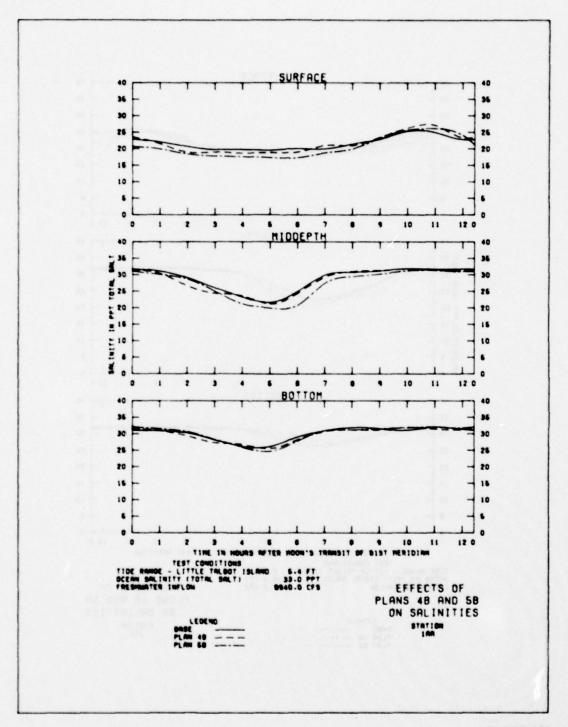


PLATE 133

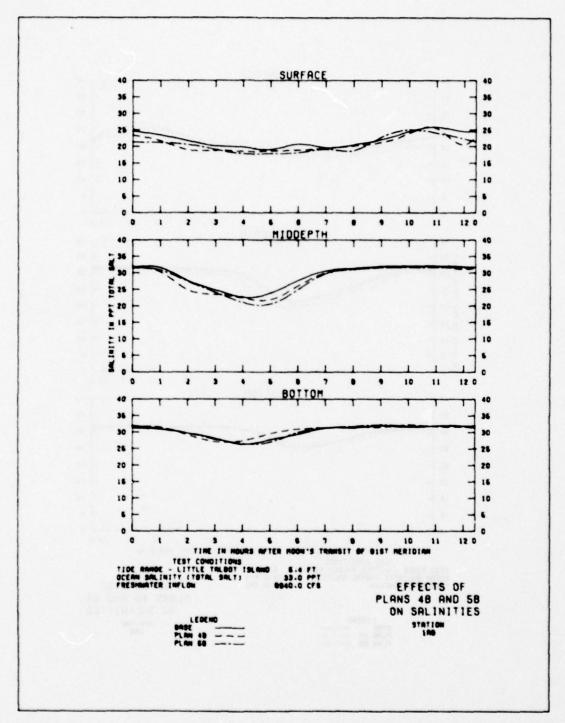


PLATE 134

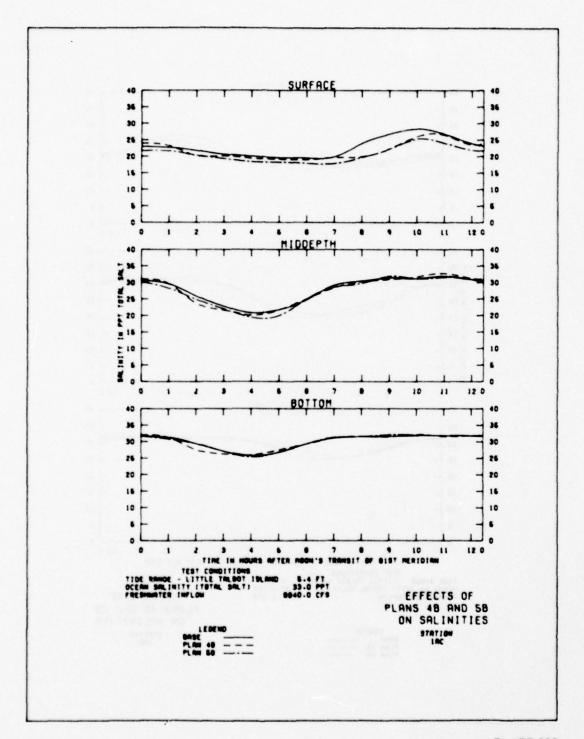


PLATE 135

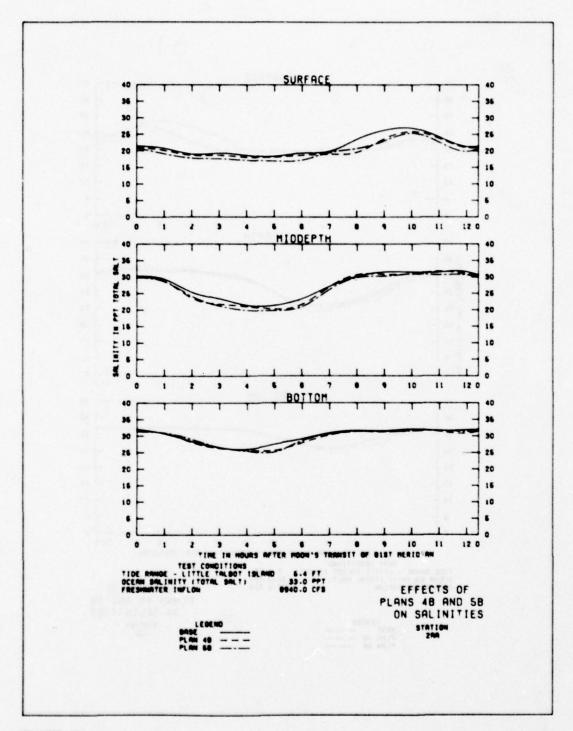


PLATE 136

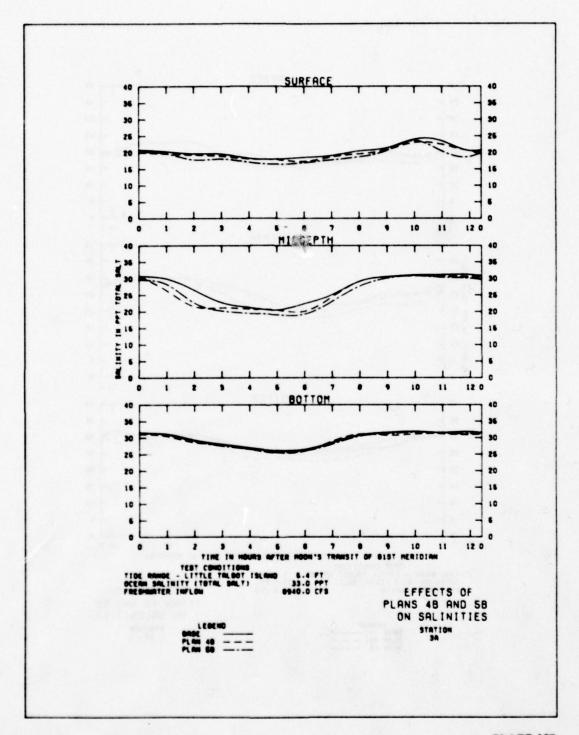


PLATE 137

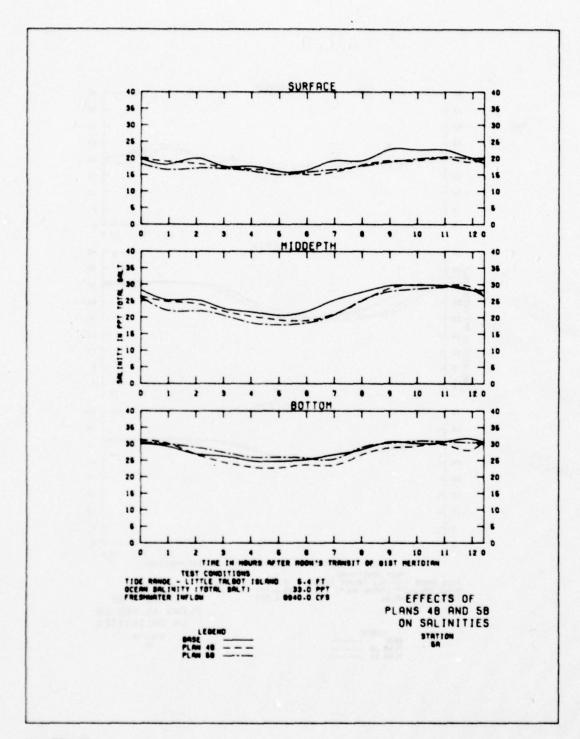
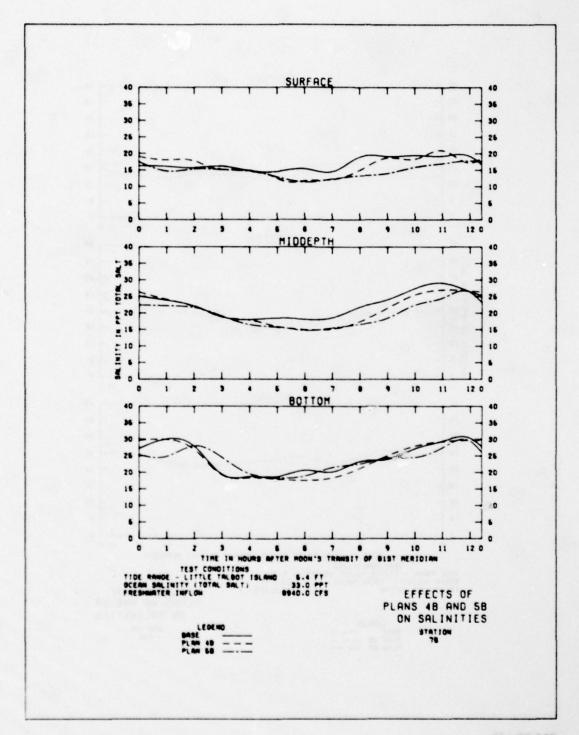


PLATE 138



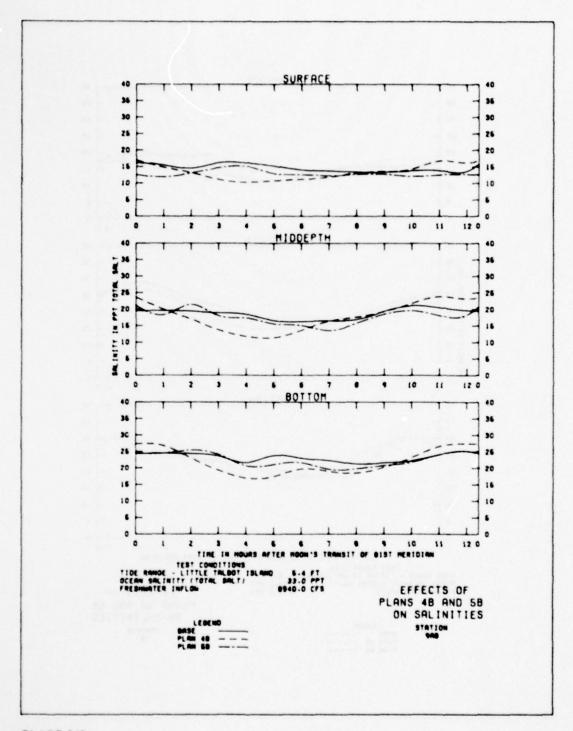
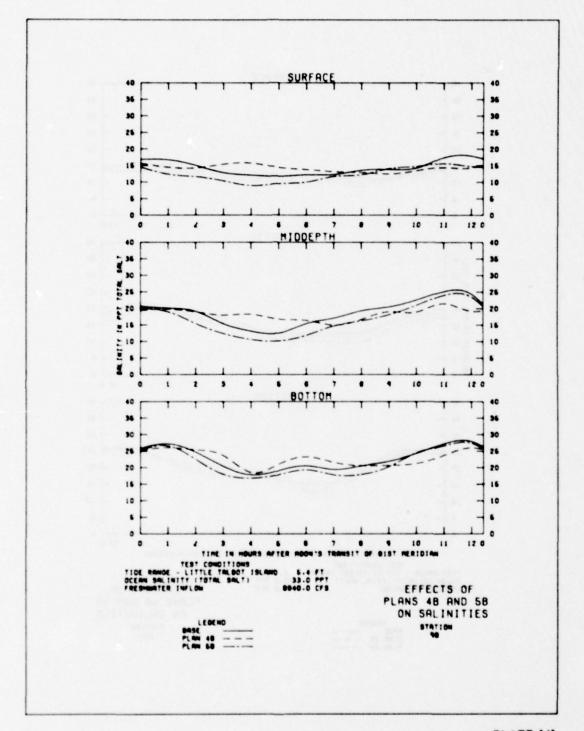


PLATE 140



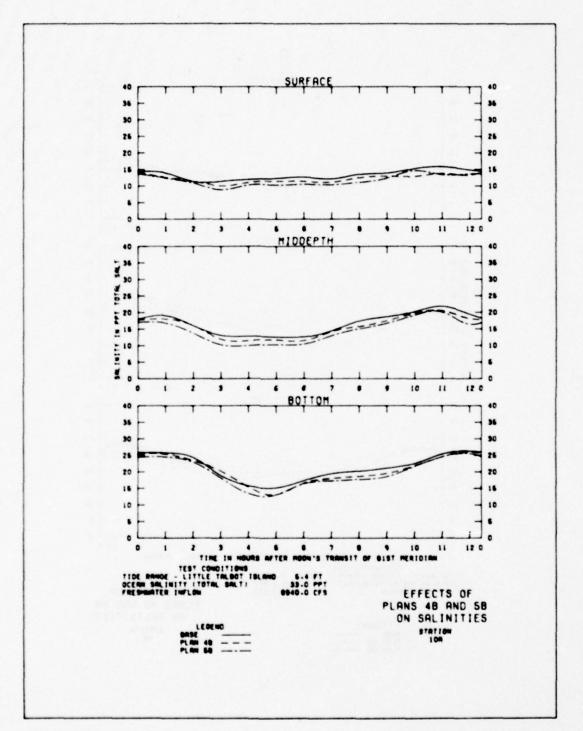
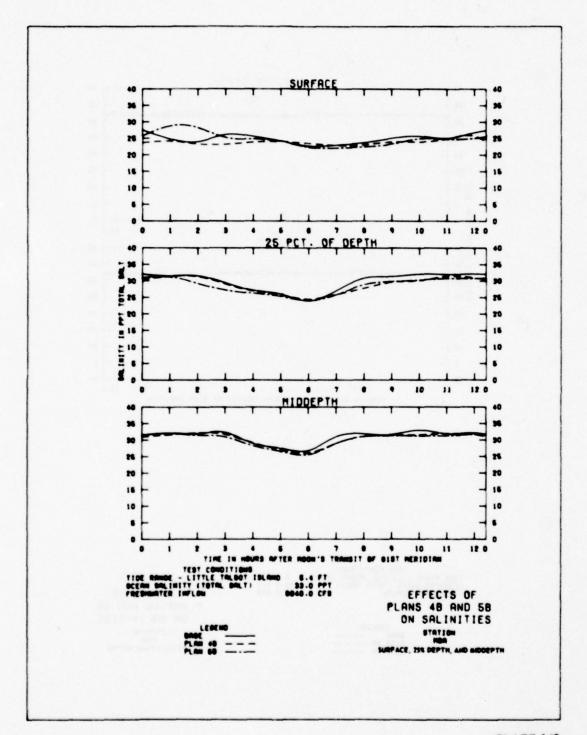
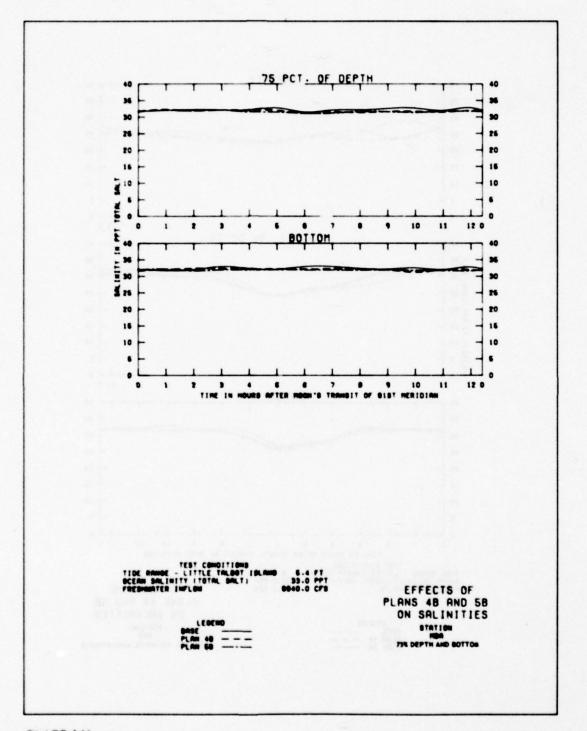


PLATE 142





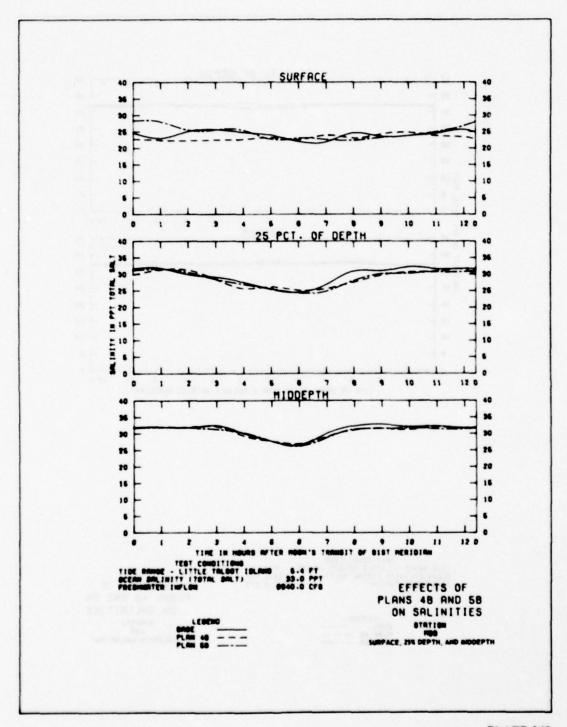


PLATE 145

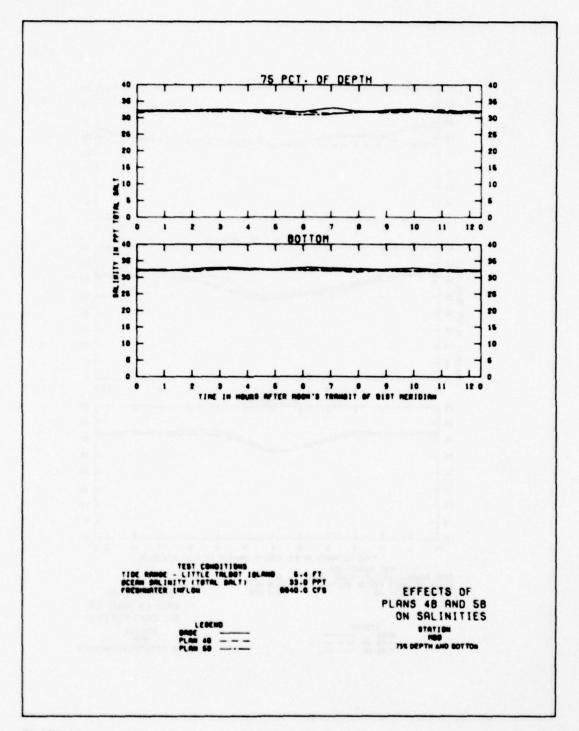
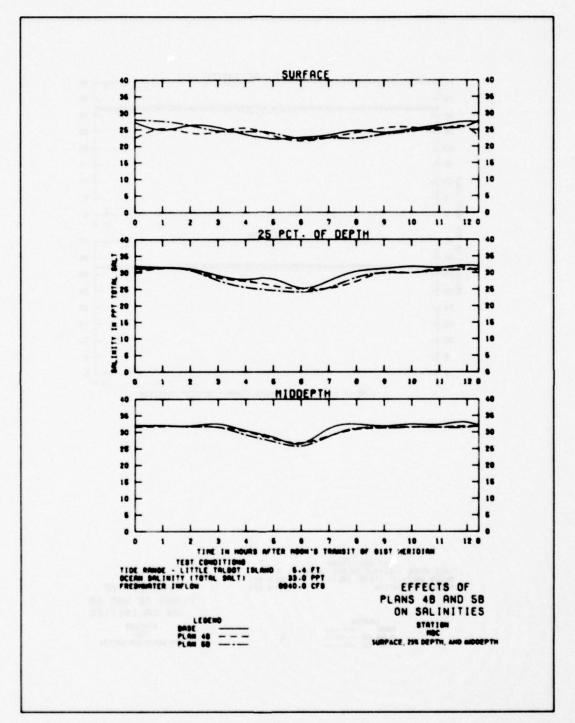


PLATE 146



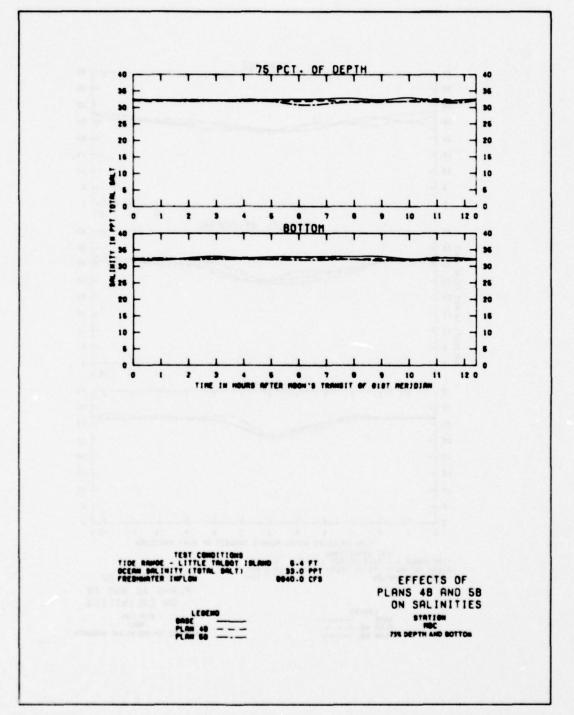


PLATE 148

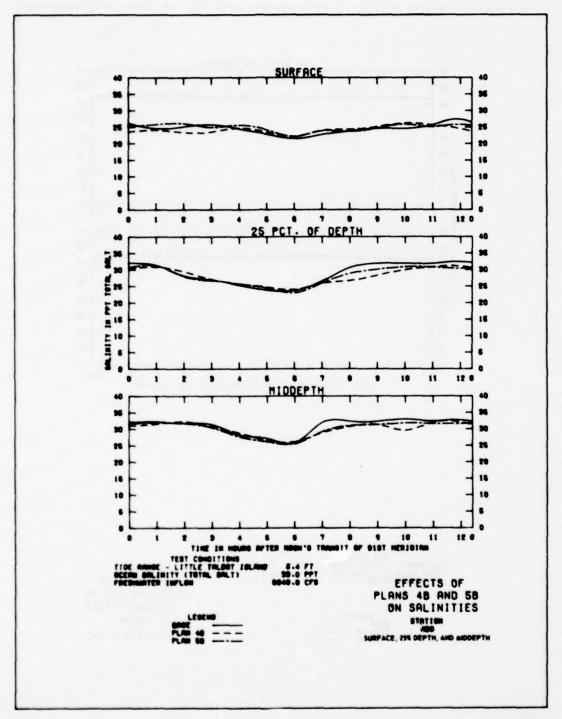
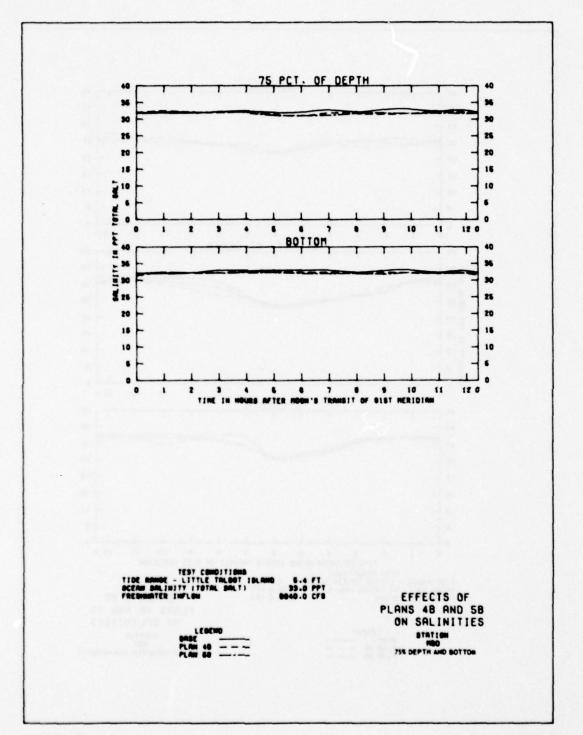


PLATE 149



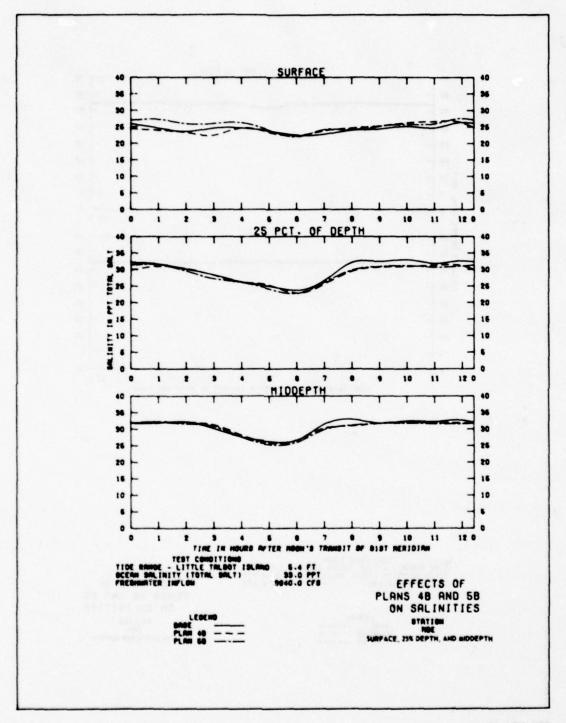
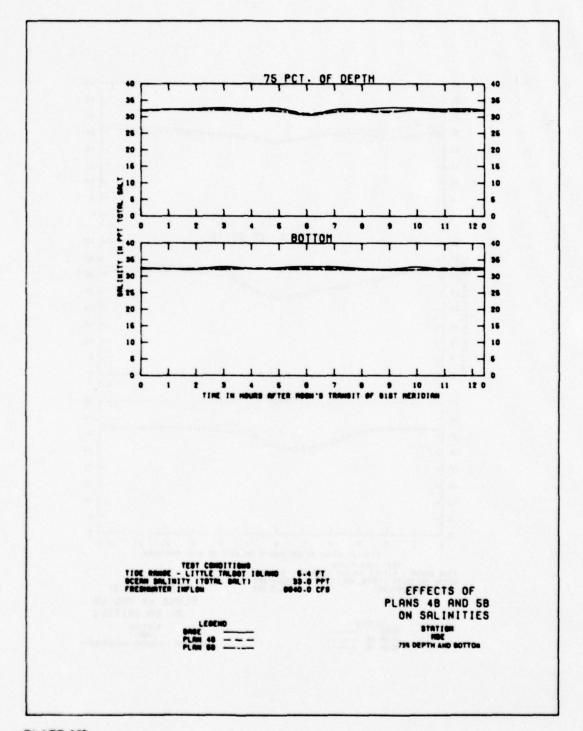


PLATE 151



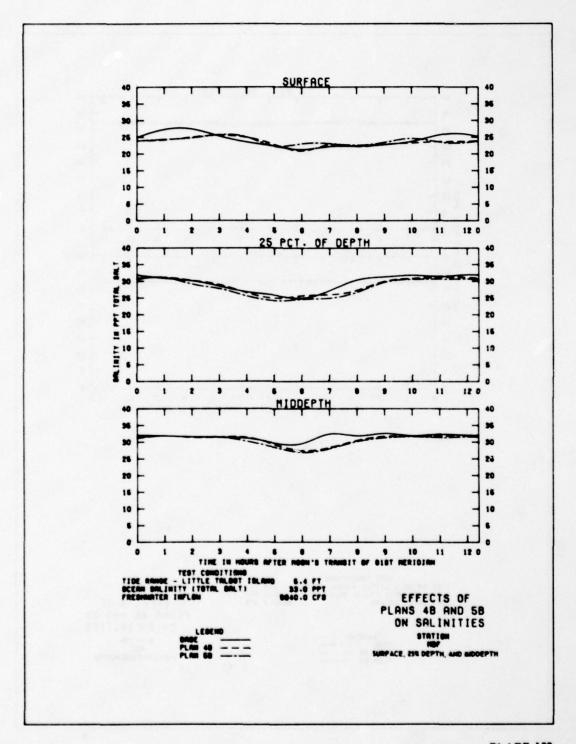
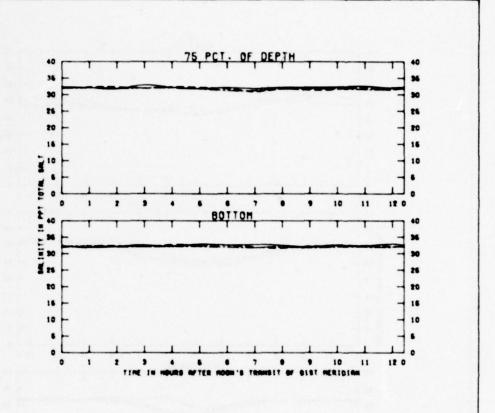


PLATE 153



TIDE RRINGE - LITTLE TRADOT IBLAND 6.4 FT OCEAN SALINITY (TOTAL SALT) 93.0 PPT PRESMANTER INFLON 9040.0 CFB

PLM 48 ---

EFFECTS OF PLANS 48 AND 58 ON SALINITIES STATION TOP 735 DEPTH AND BOTTON

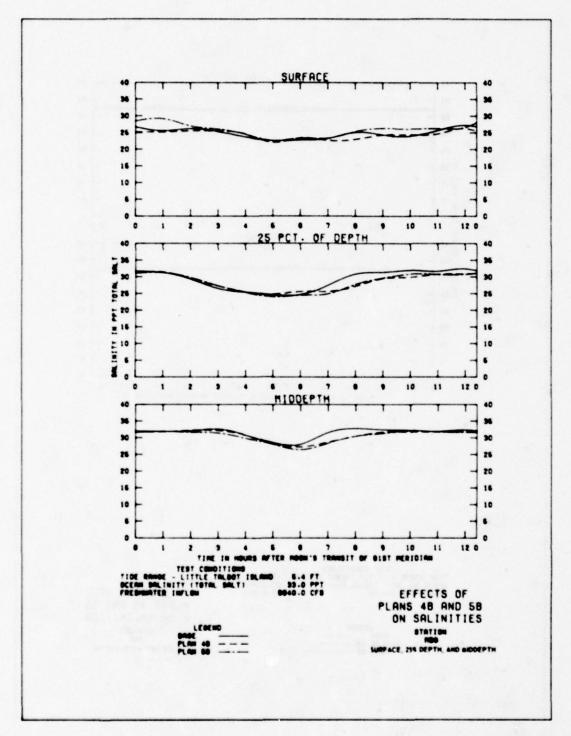


PLATE 155

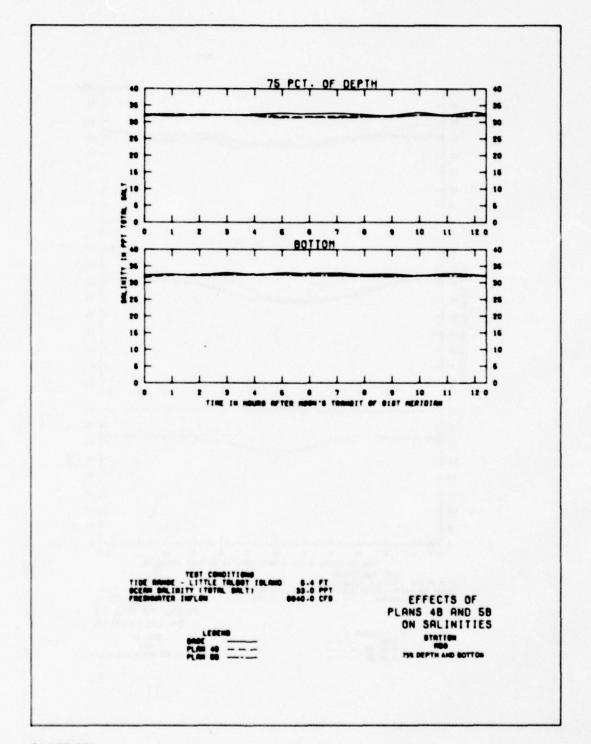


PLATE 156

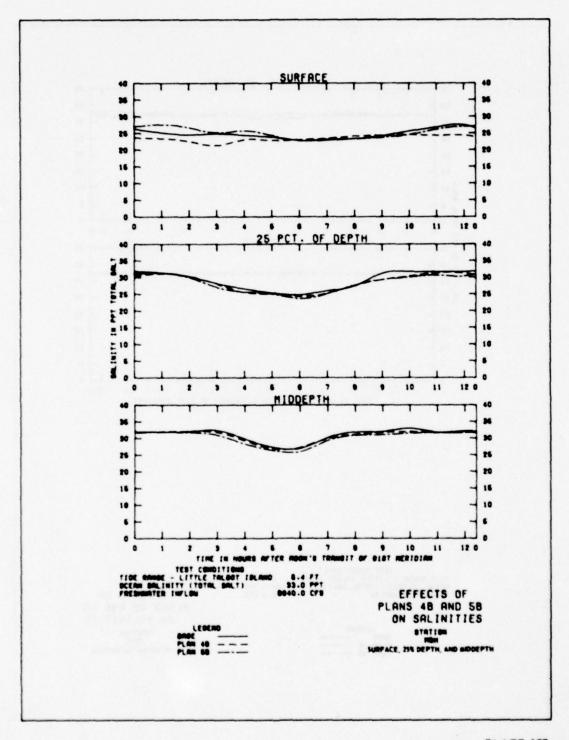


PLATE 157

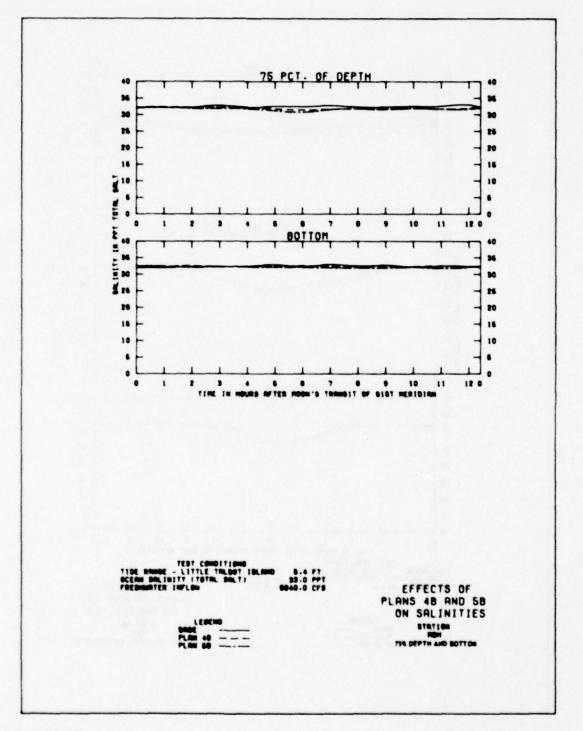


PLATE 158

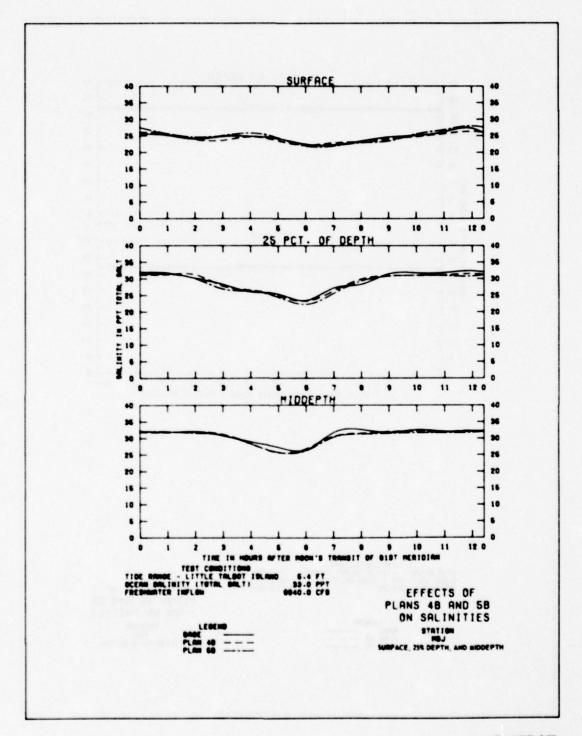
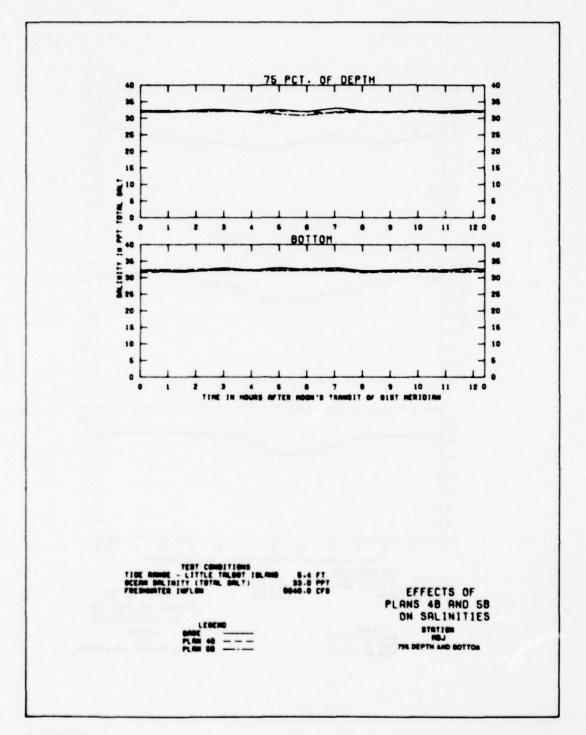


PLATE 159



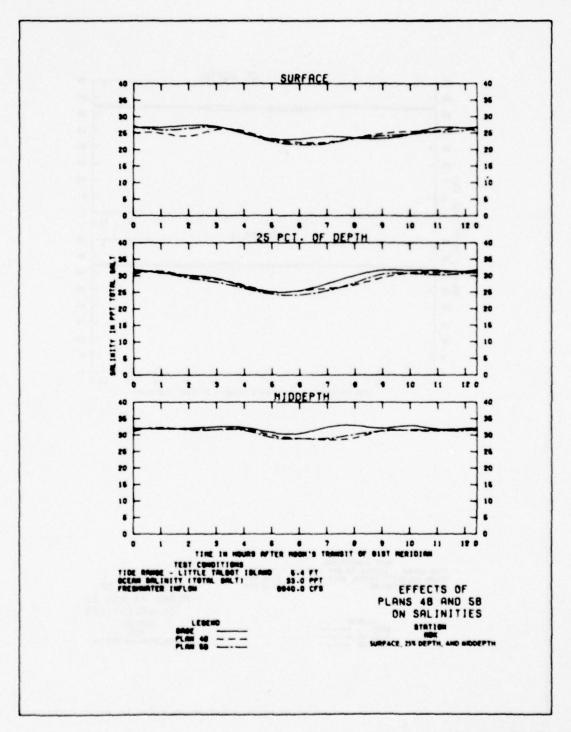


PLATE 161

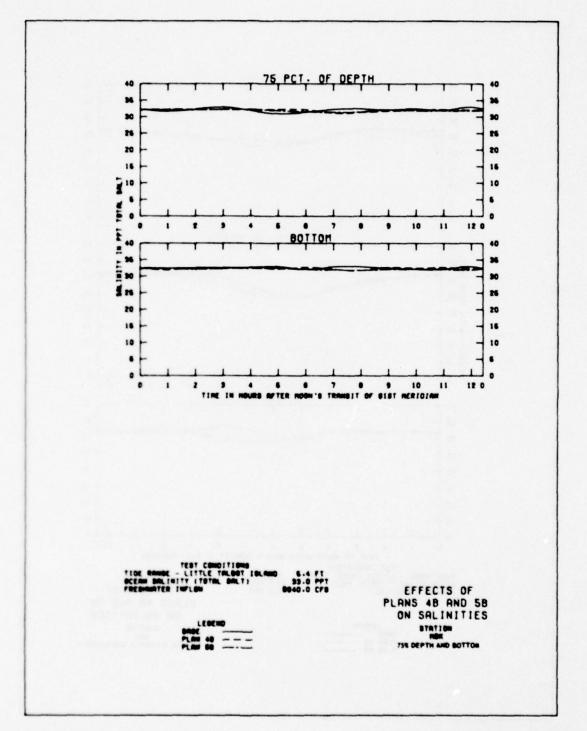


PLATE 162

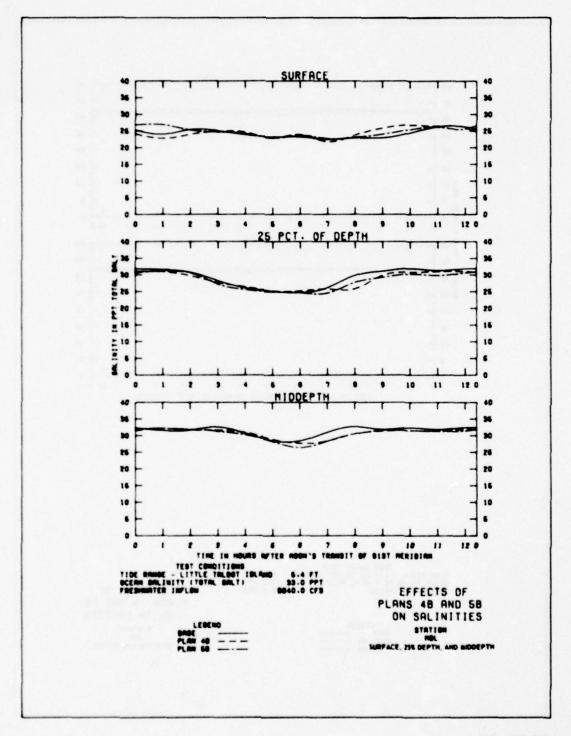


PLATE 163

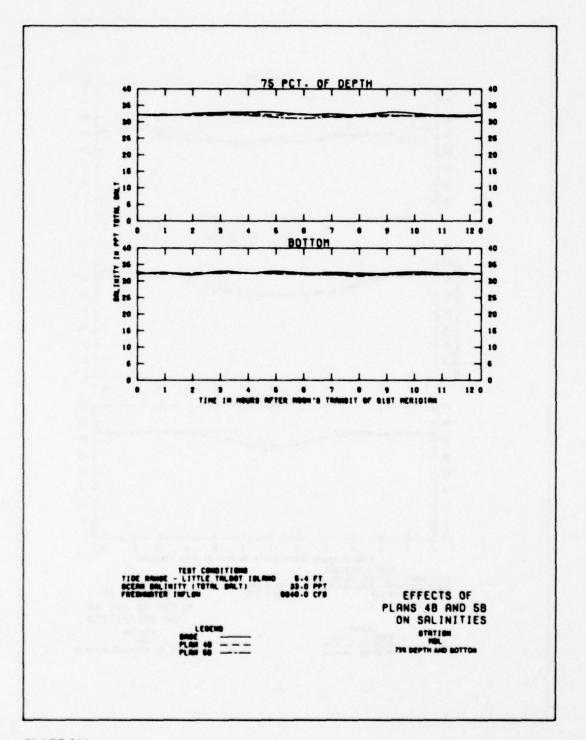


PLATE 164

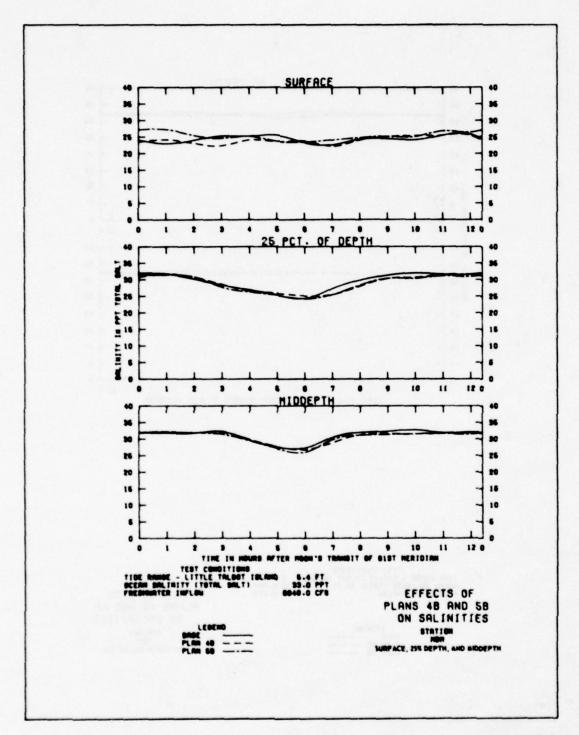
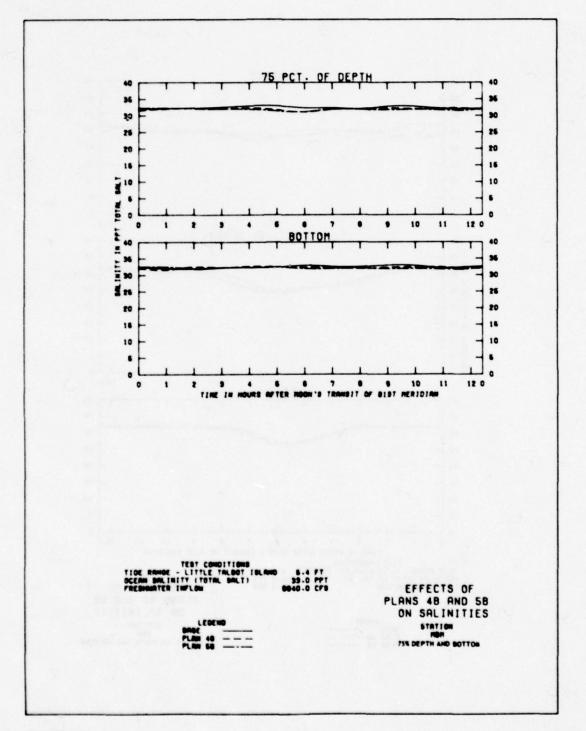


PLATE 165



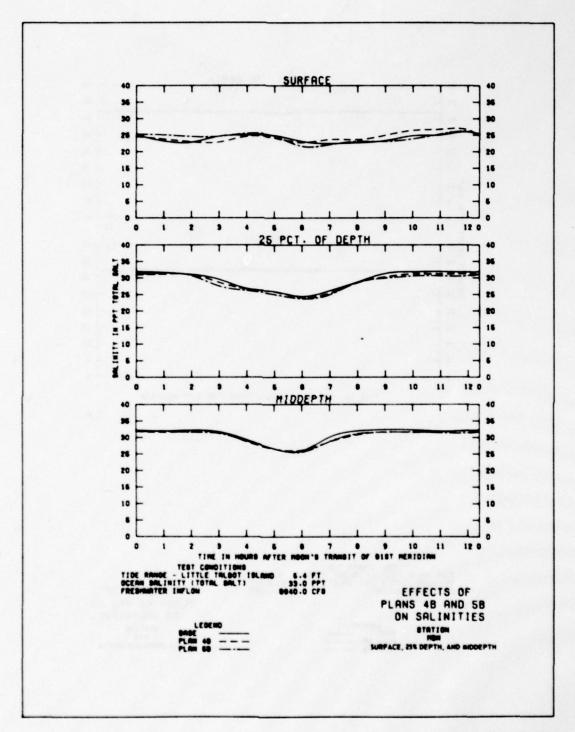


PLATE 167

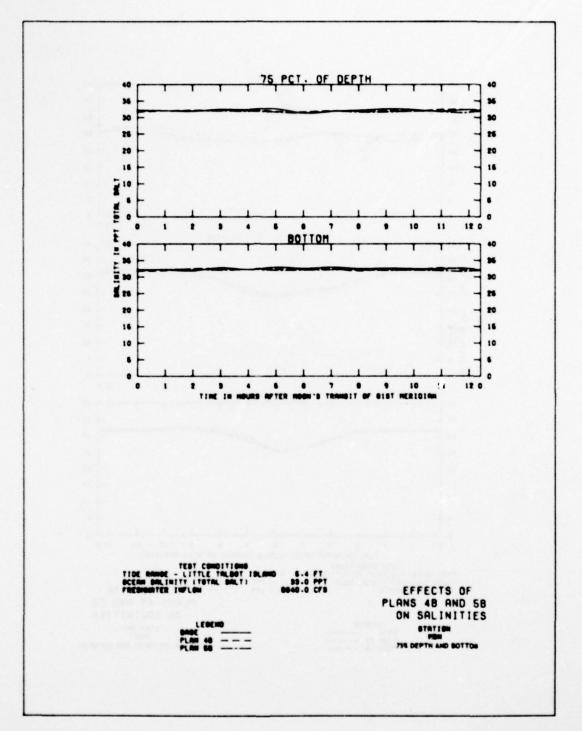


PLATE 168

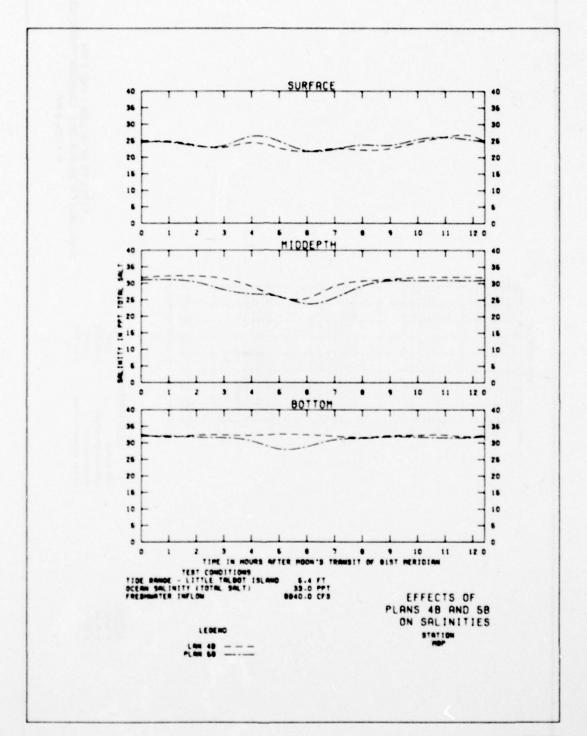
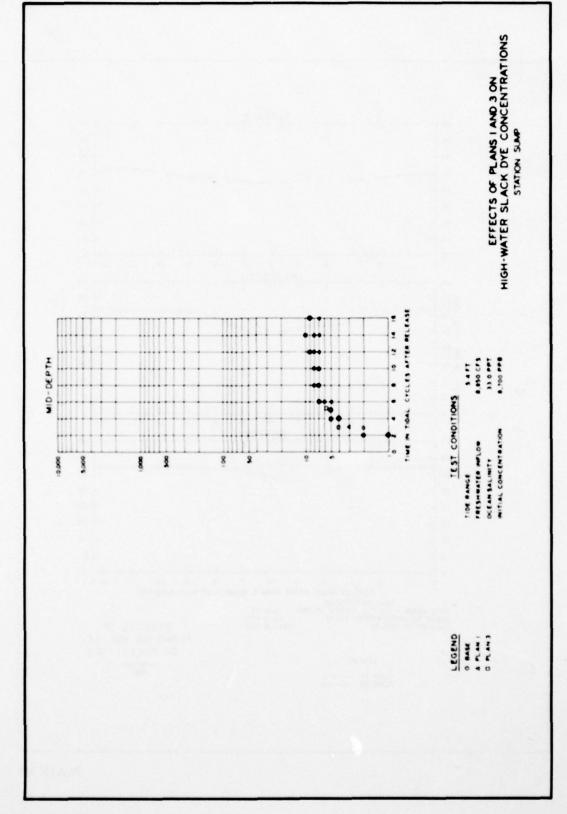
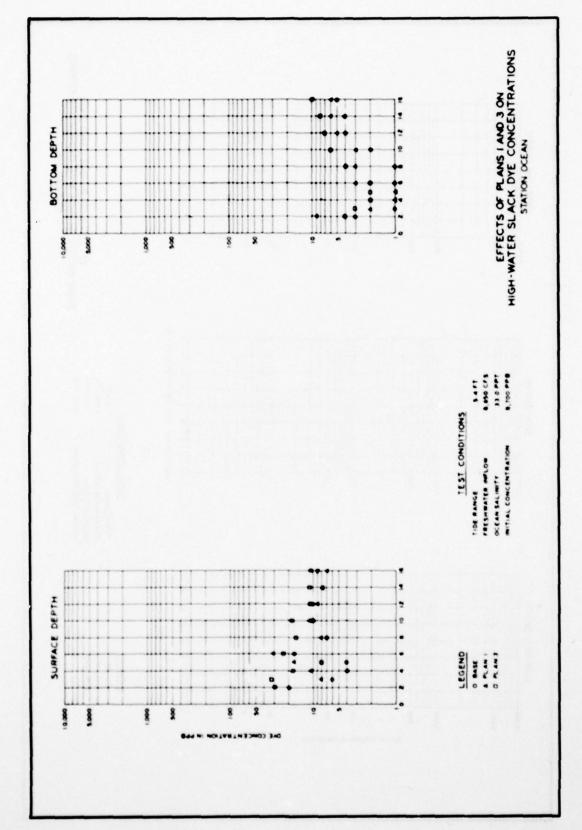
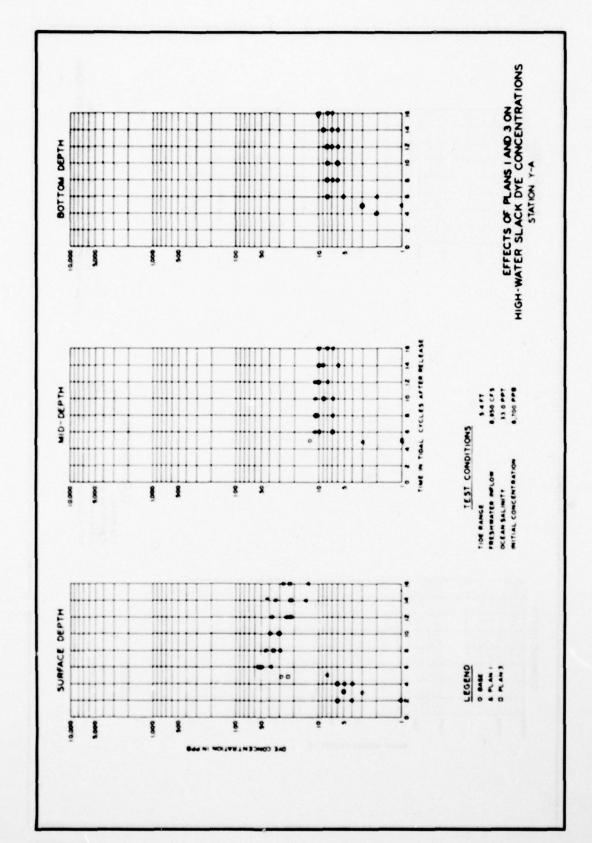
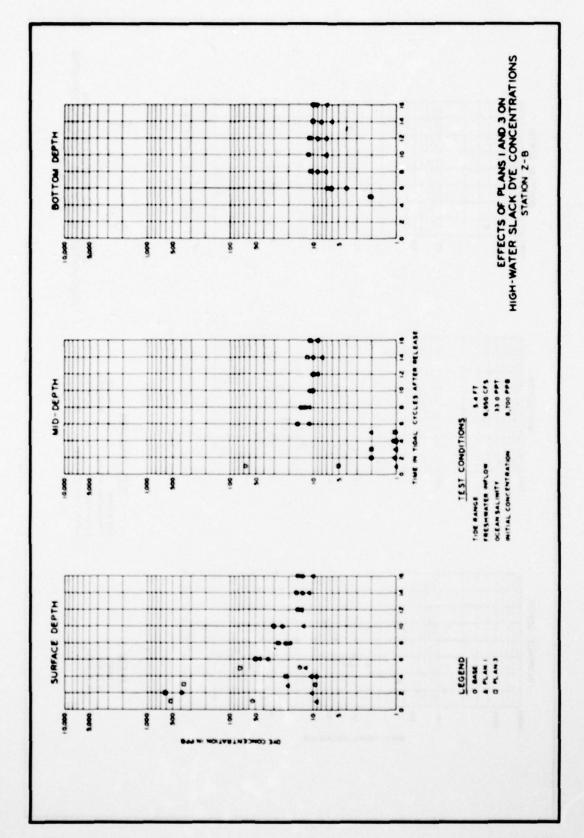


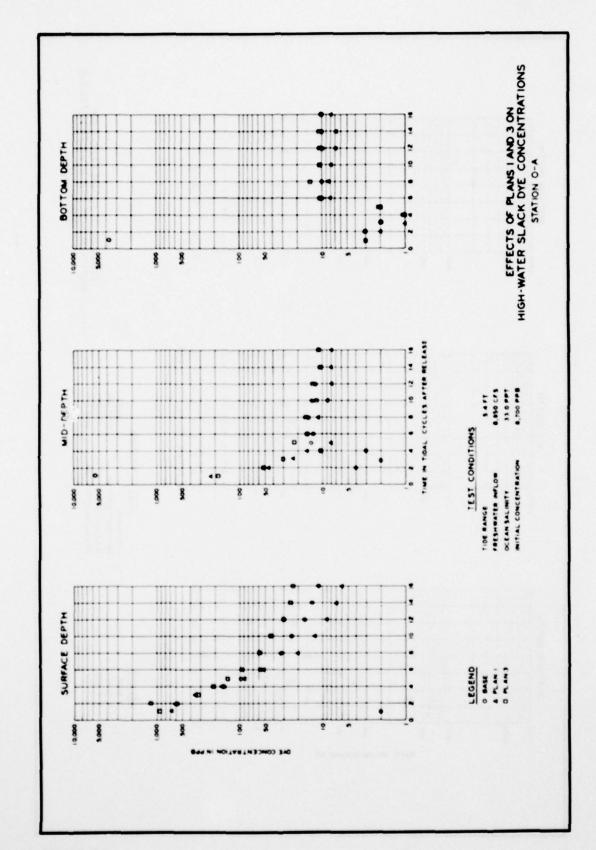
PLATE 169





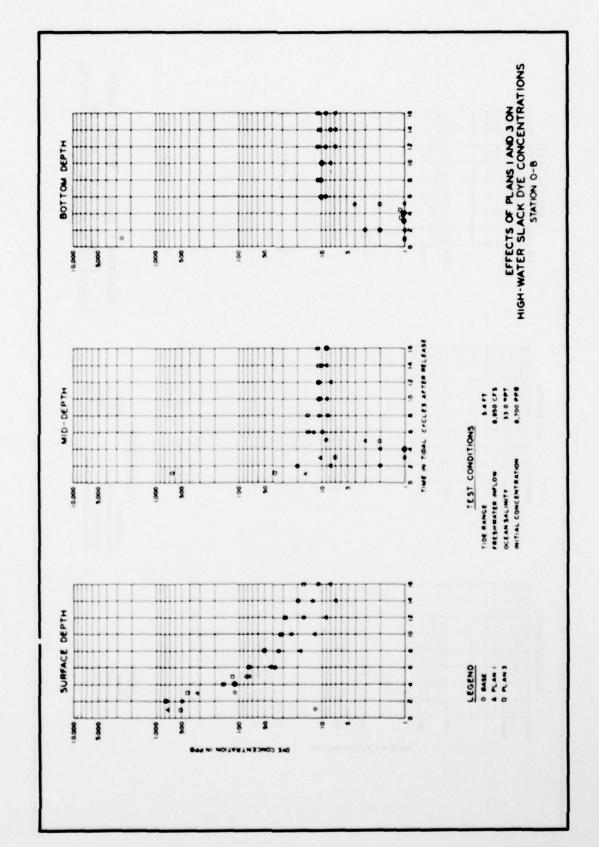


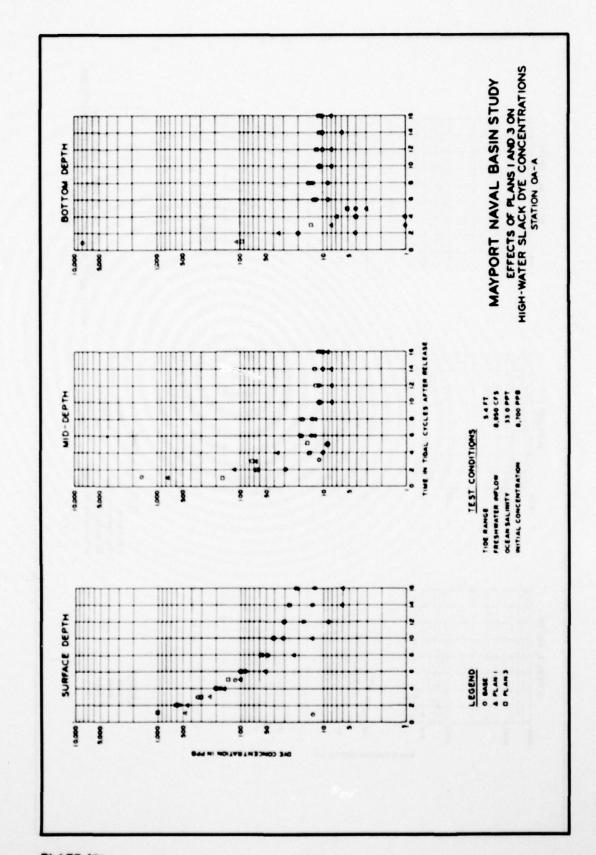


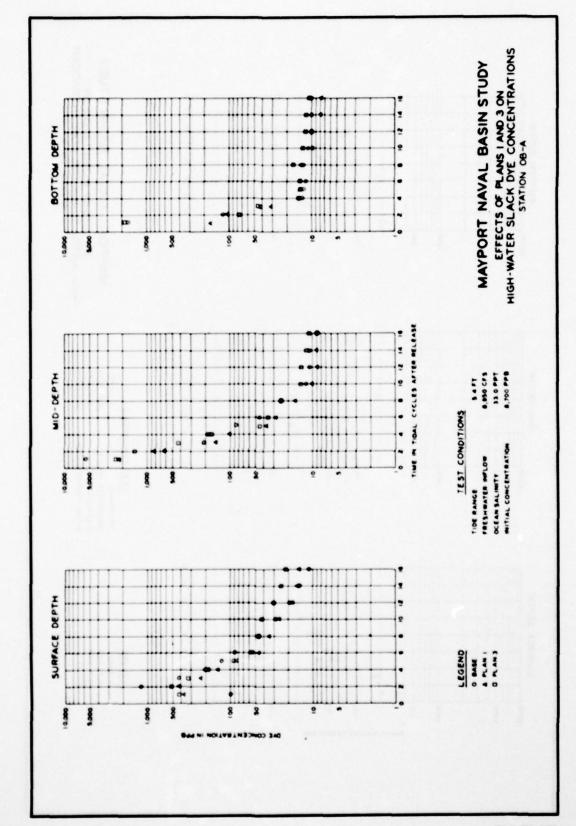


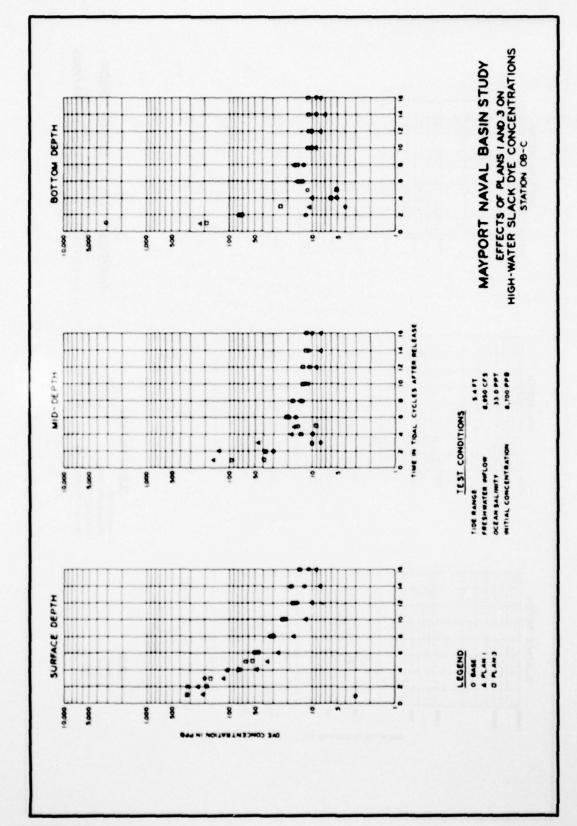
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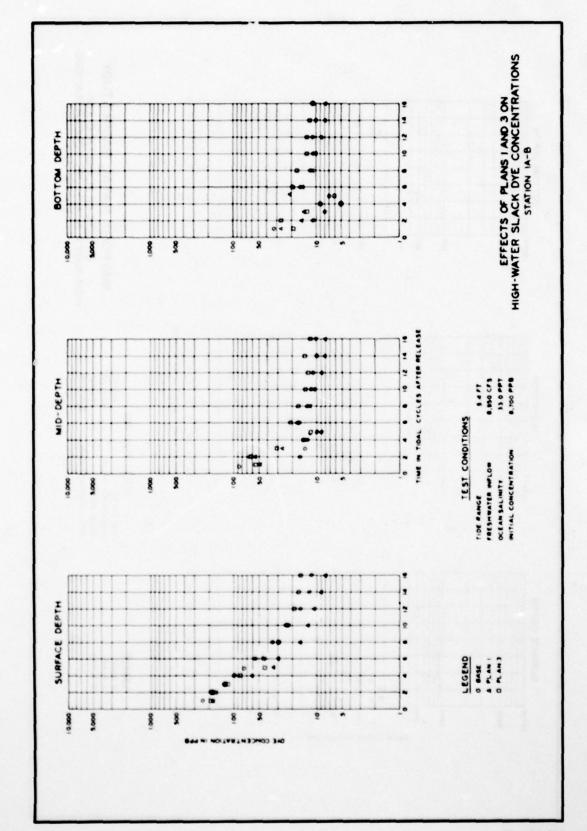
PLATE 174

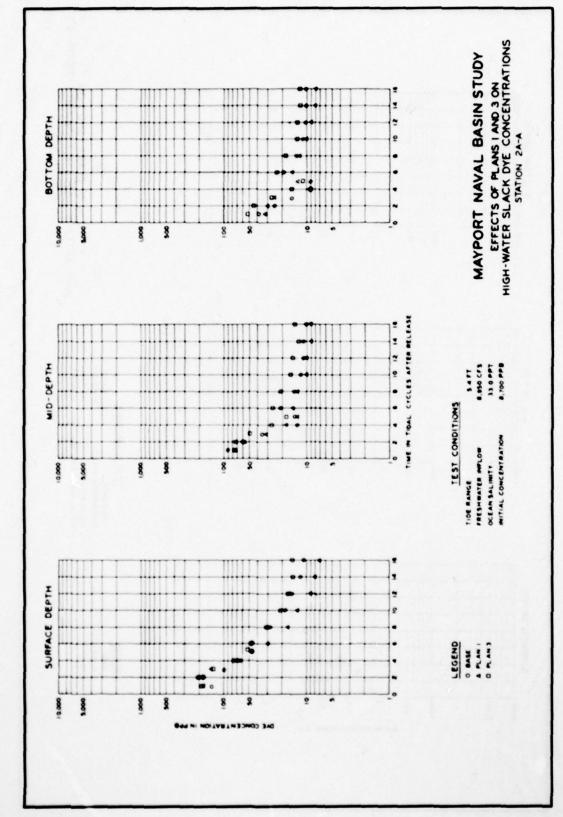


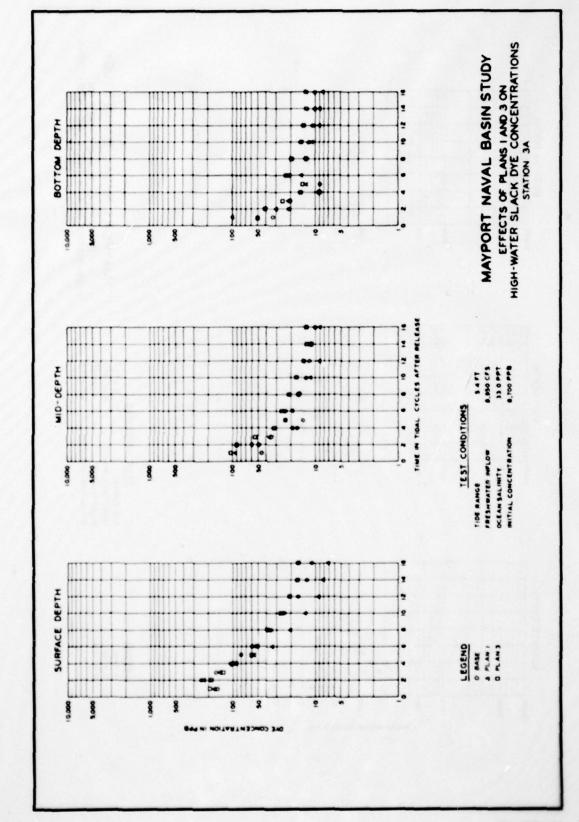


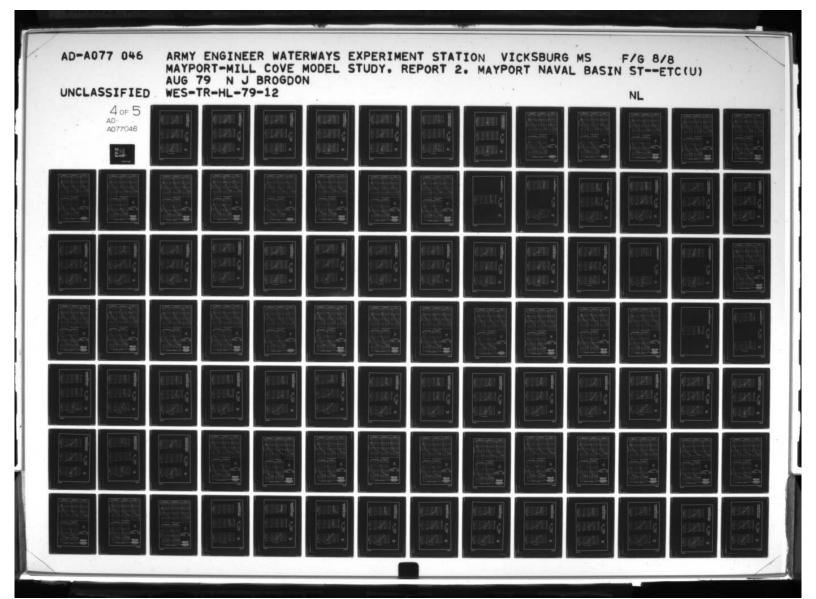


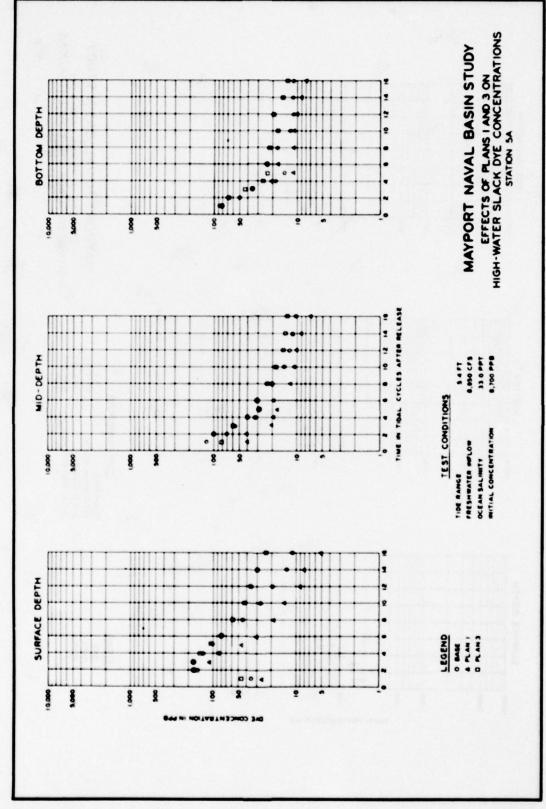


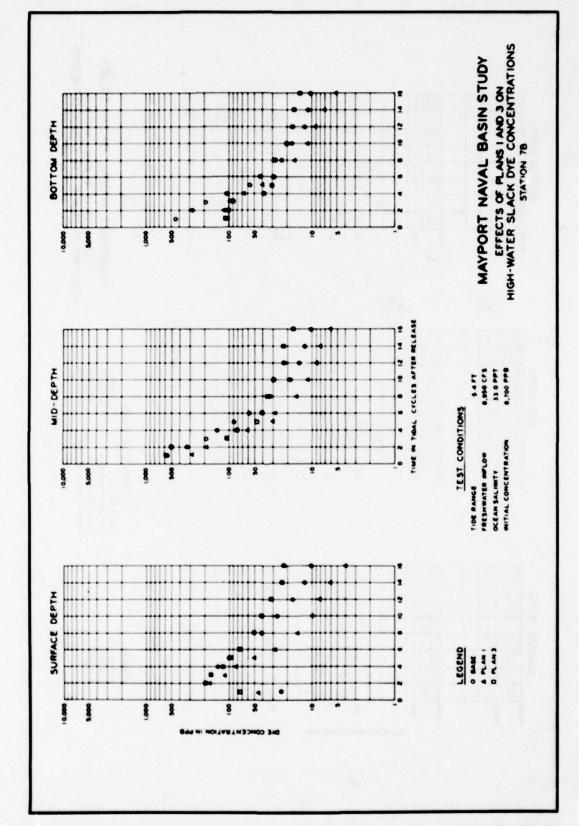












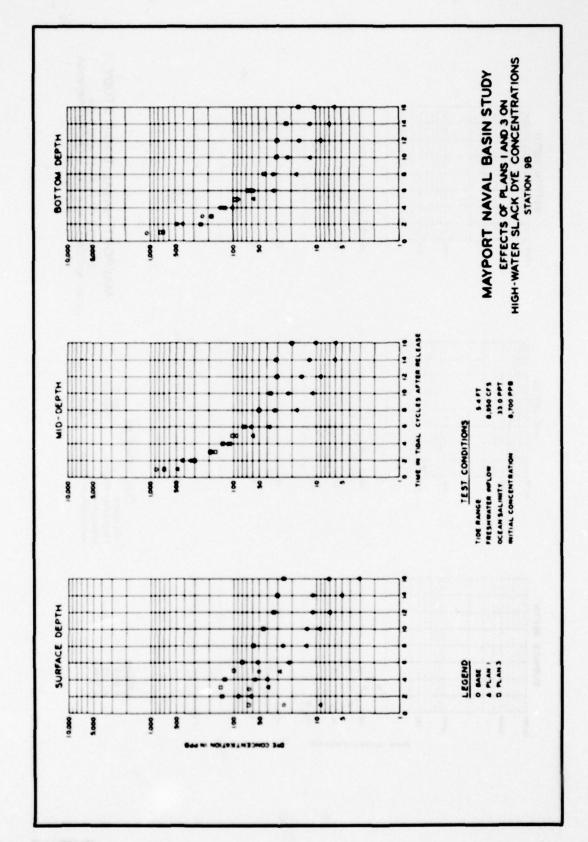
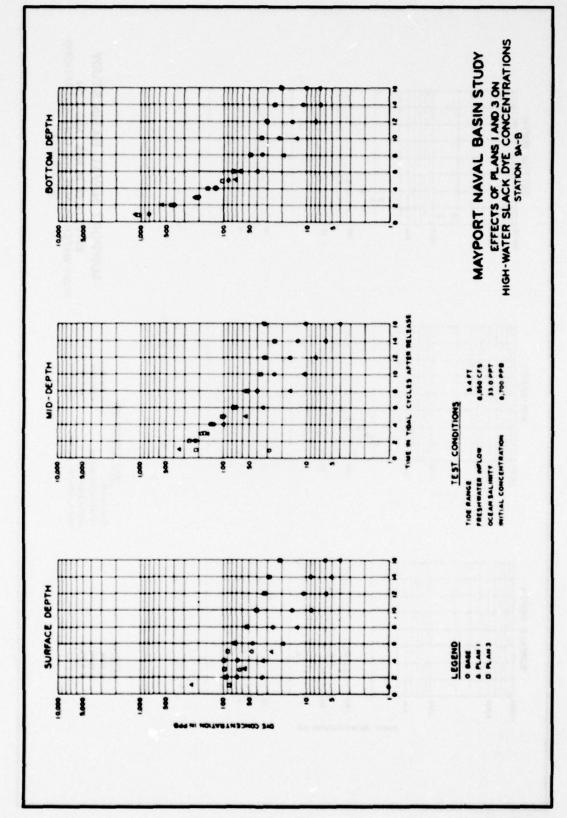
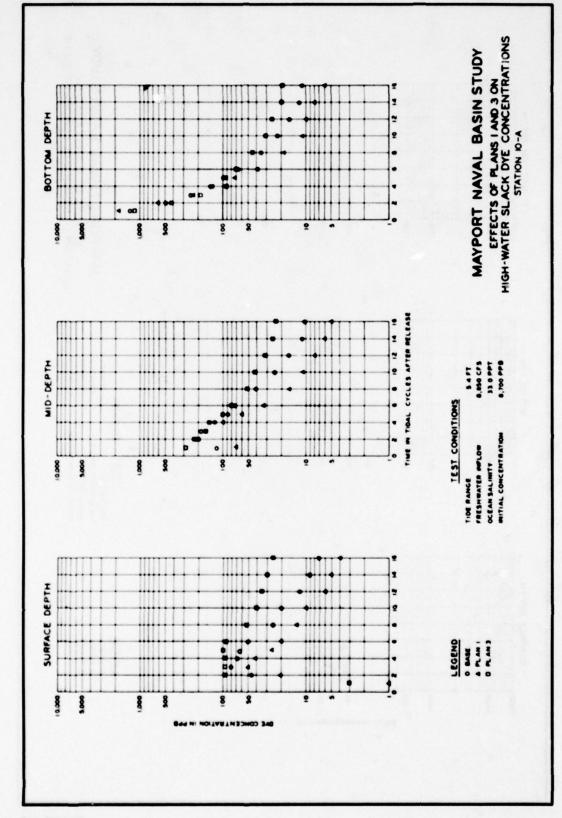
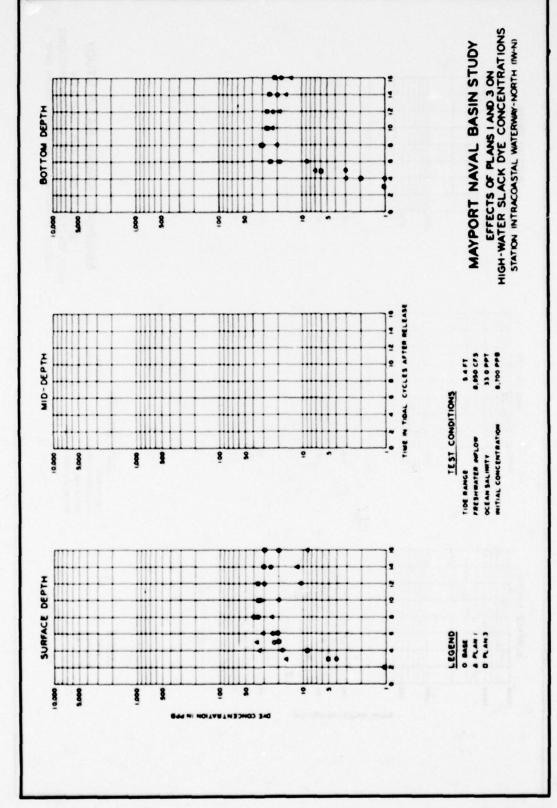
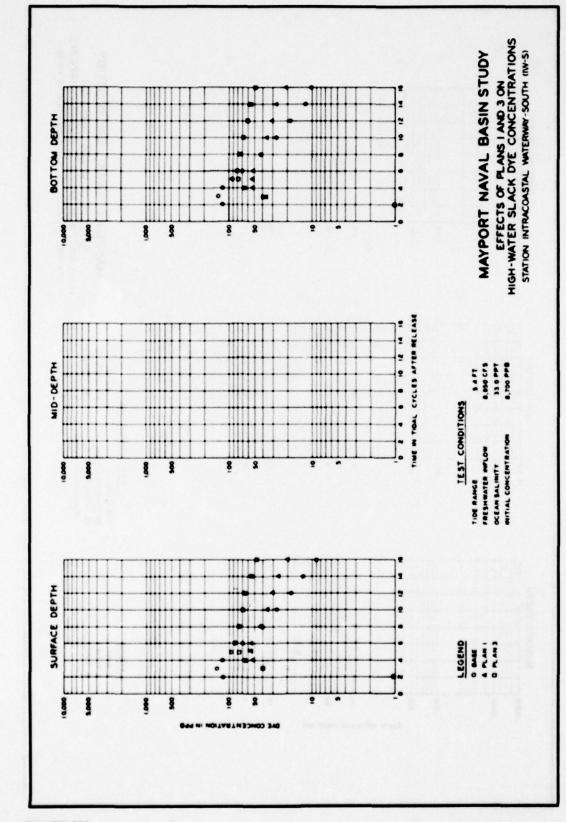


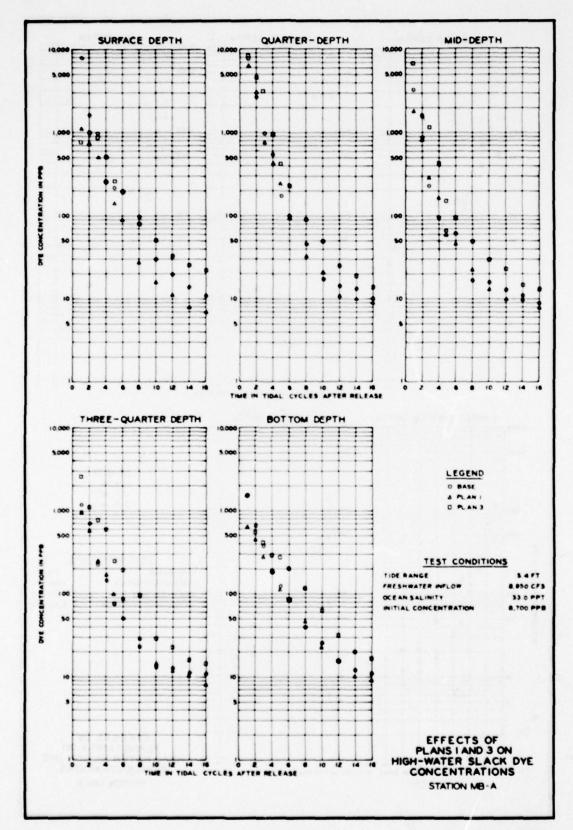
PLATE 184

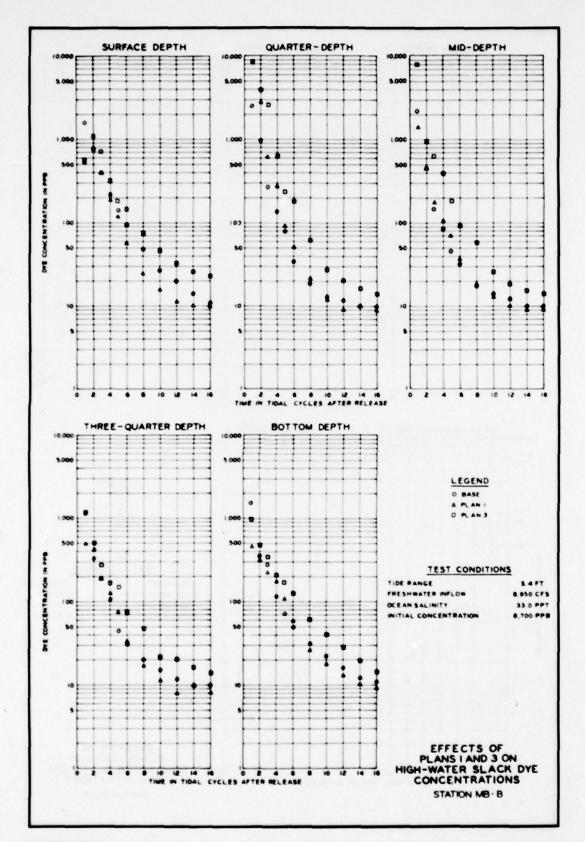


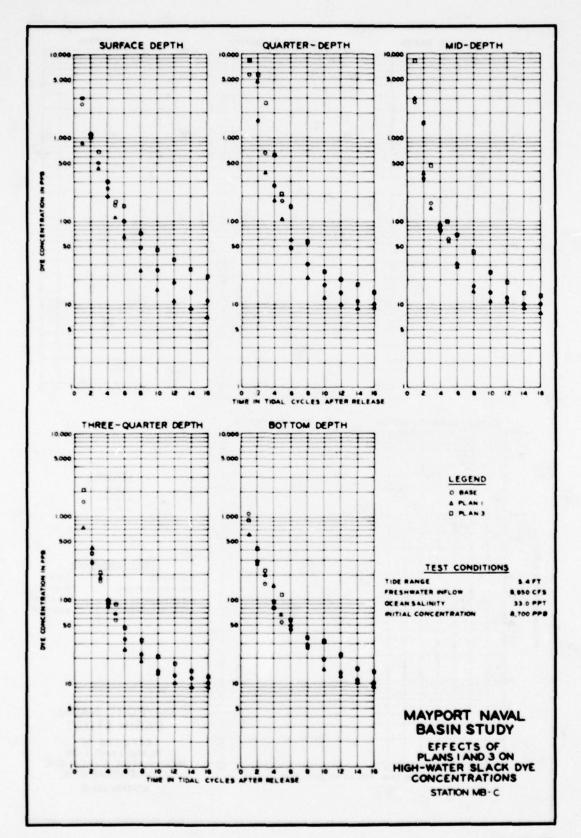


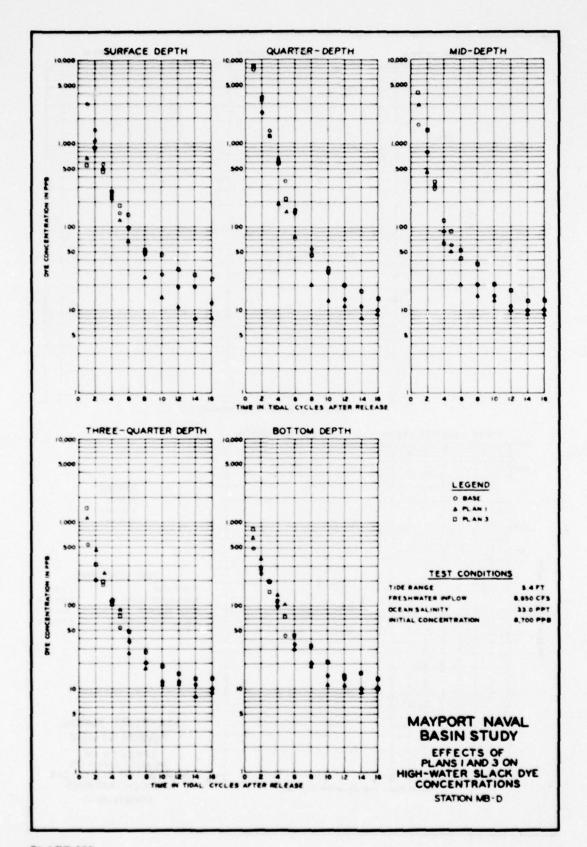


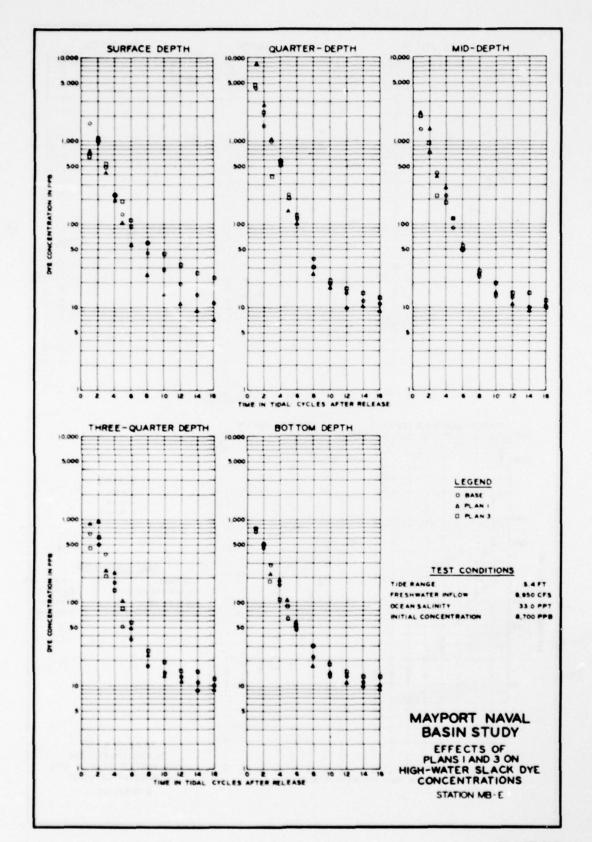












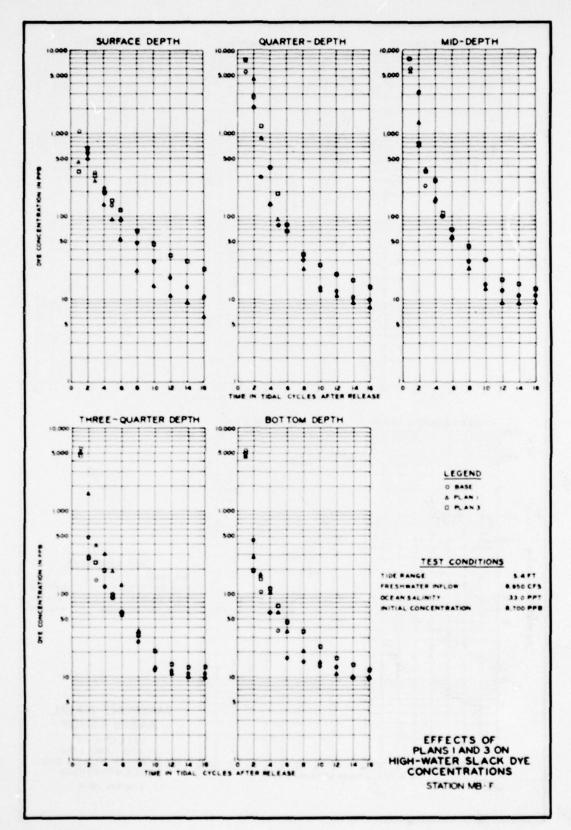
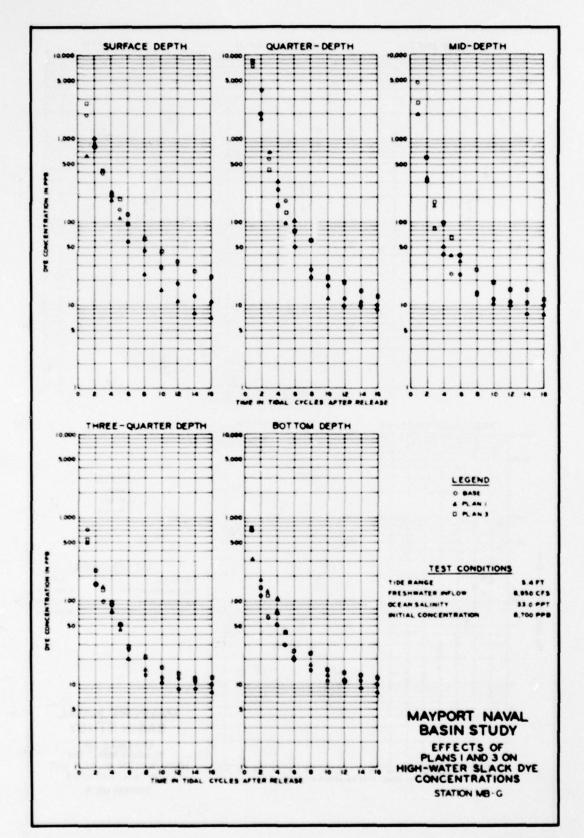
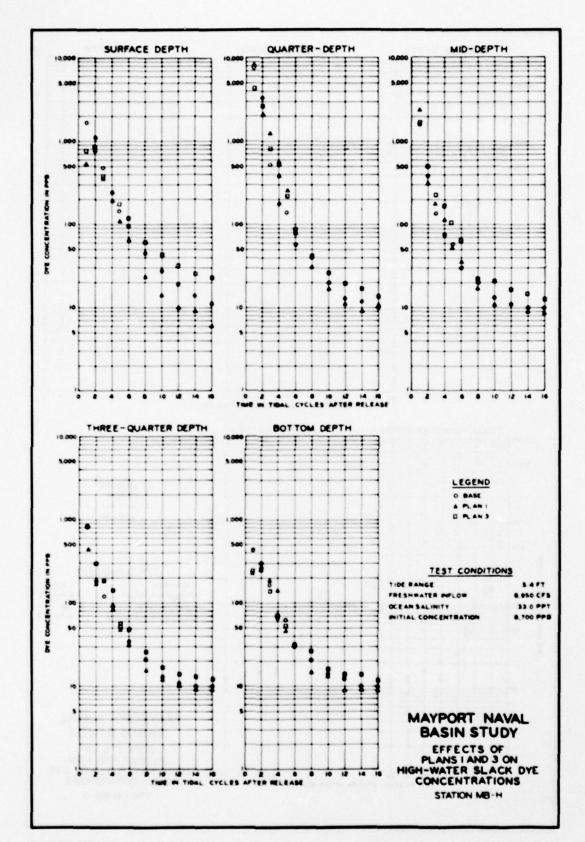
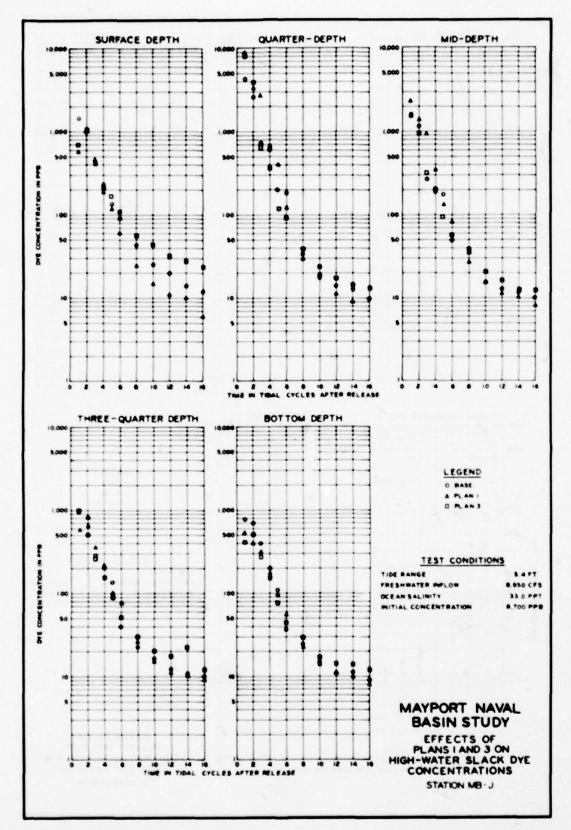
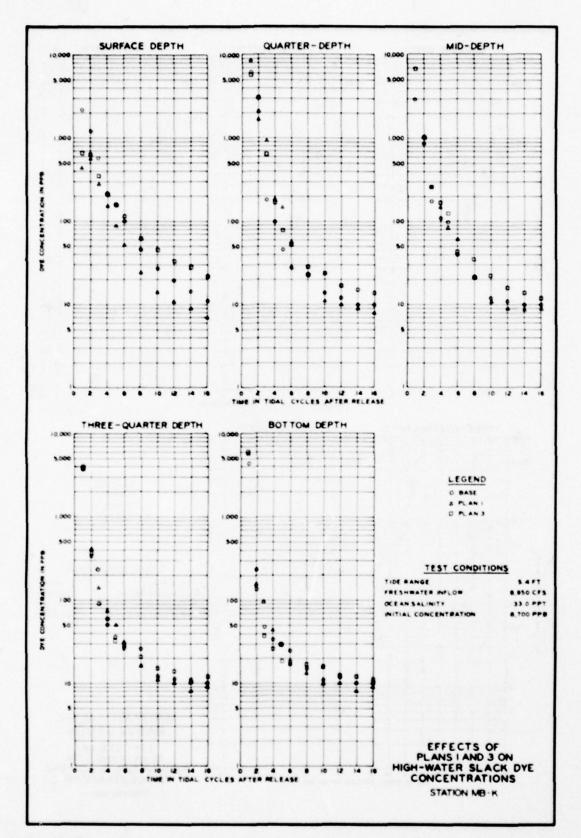


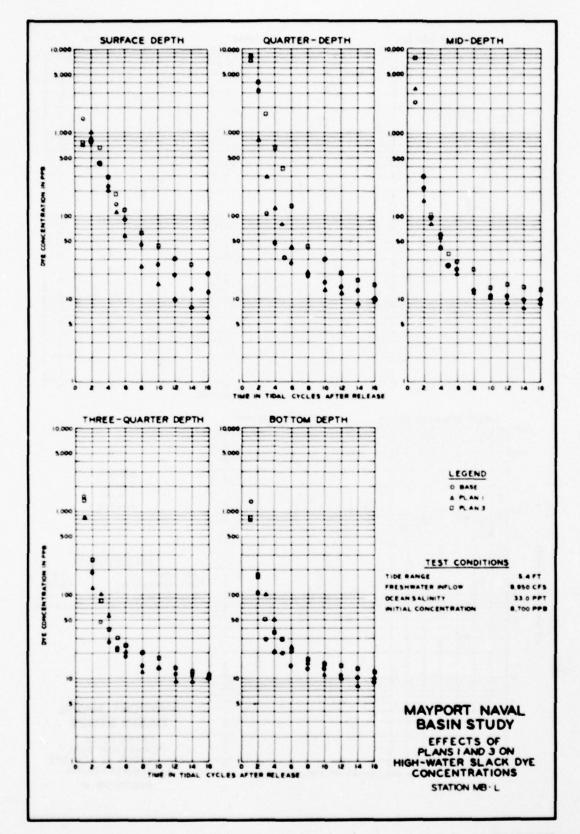
PLATE 194

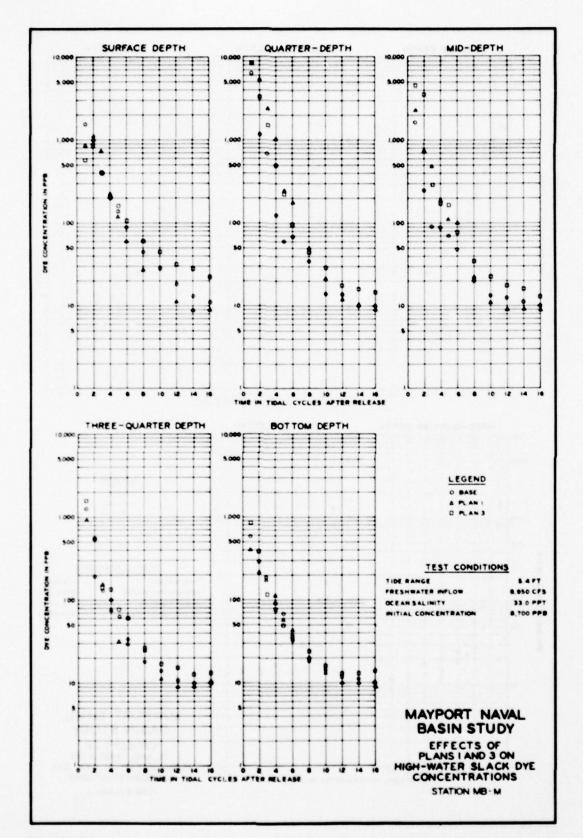


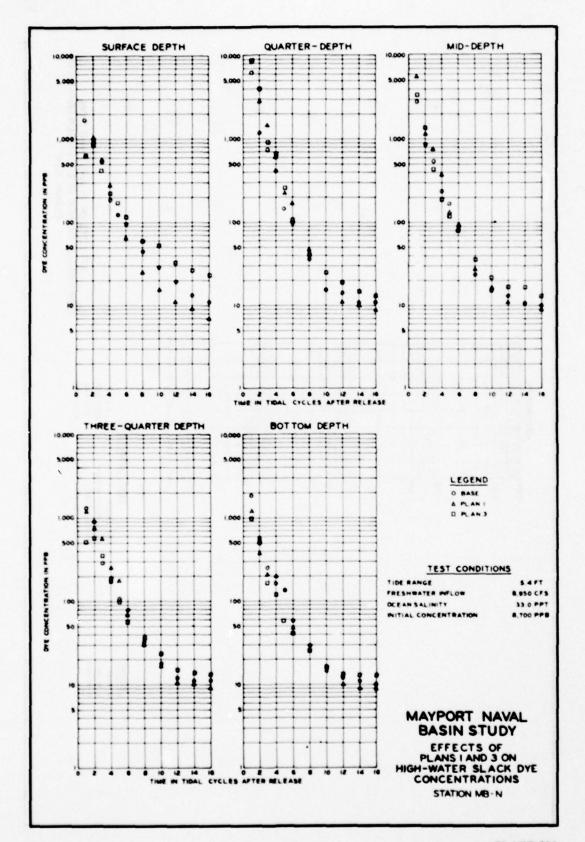


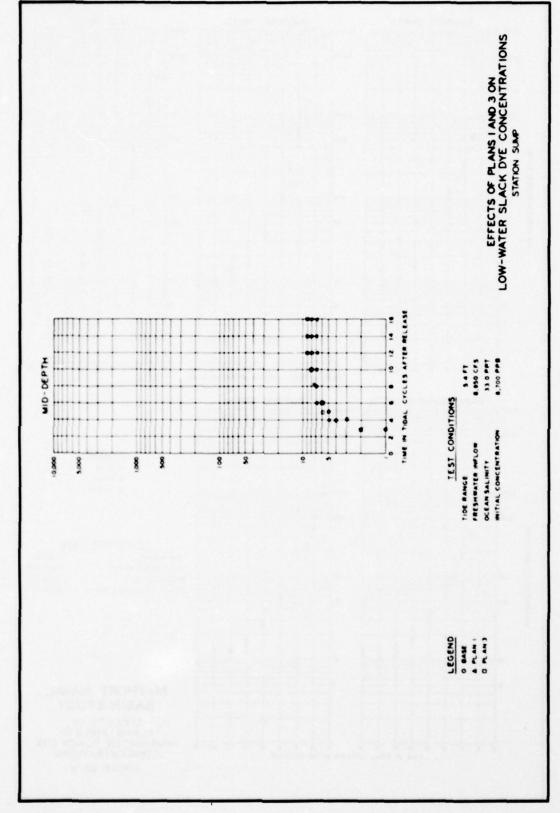


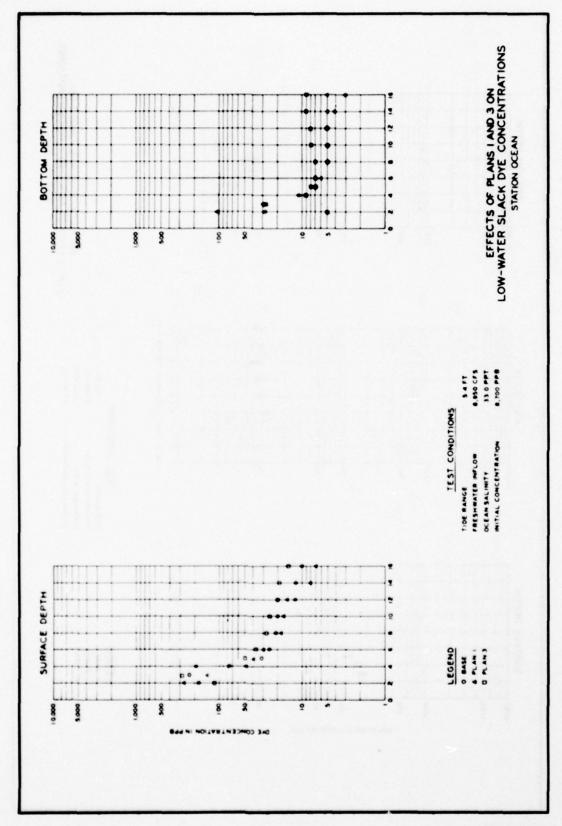


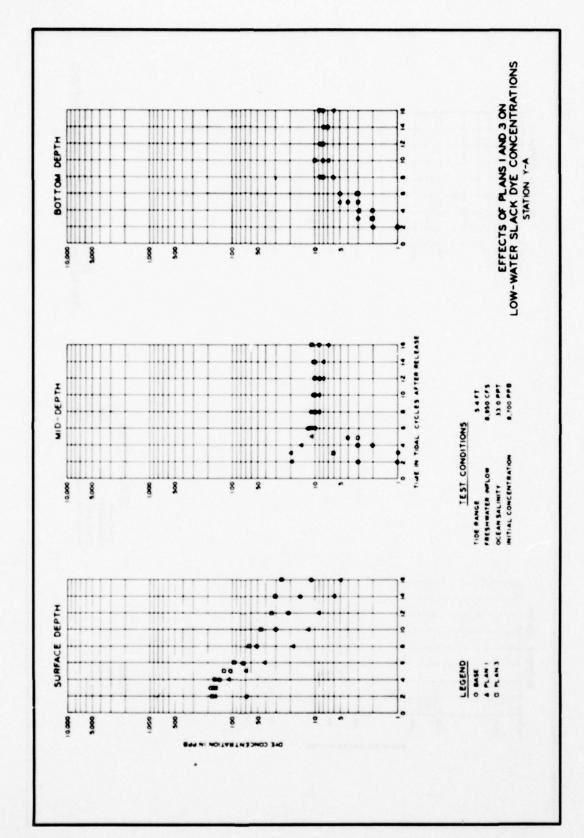


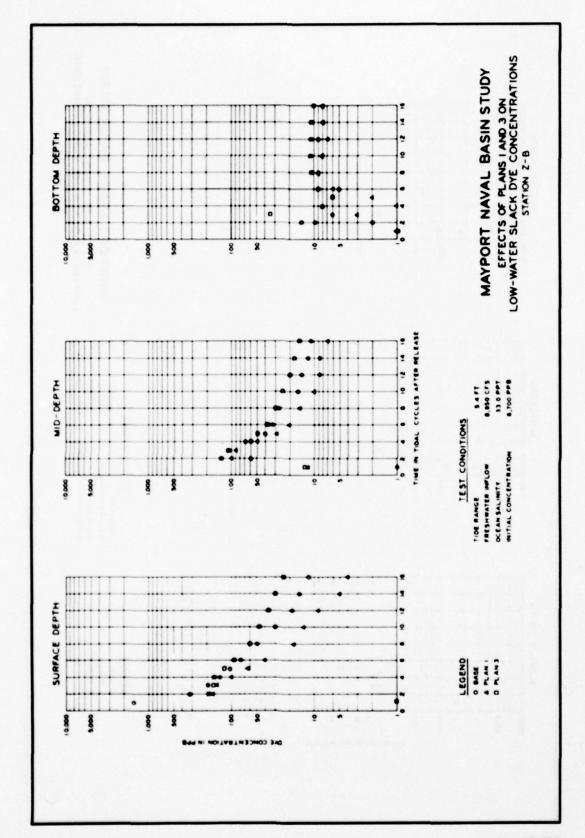


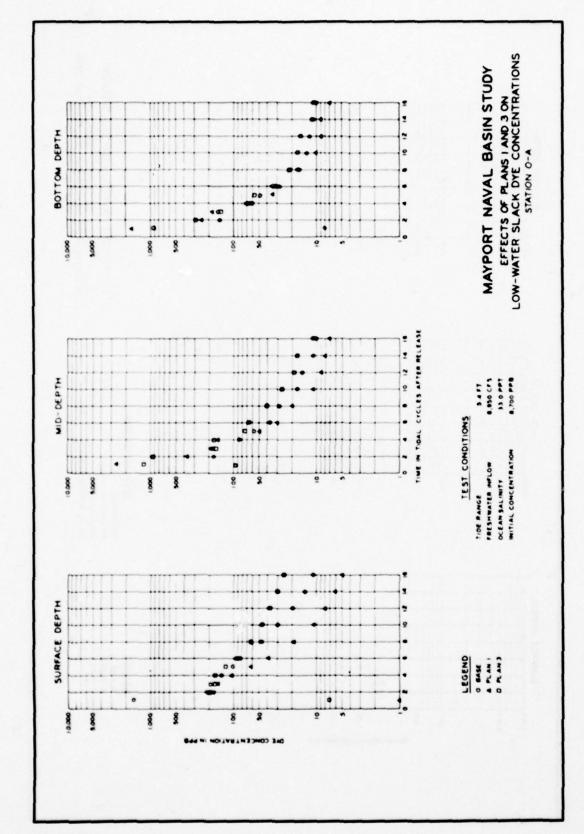


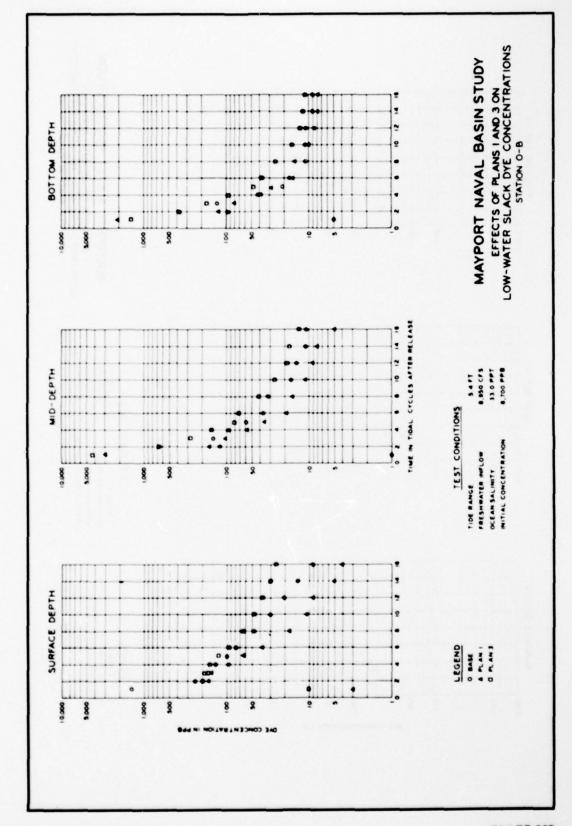


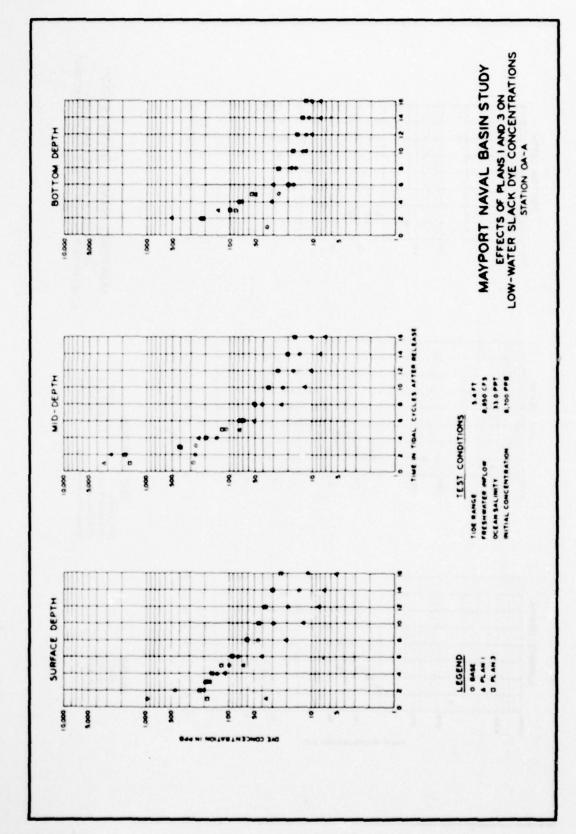


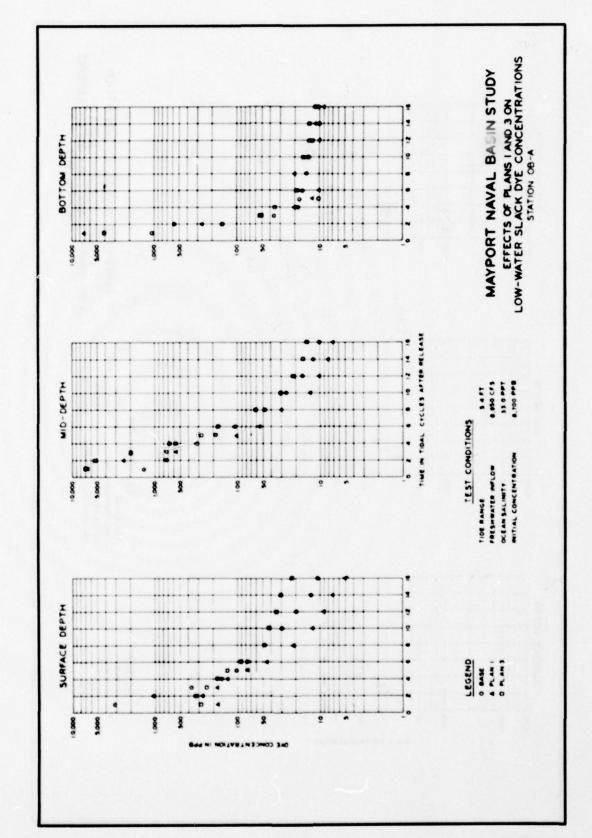












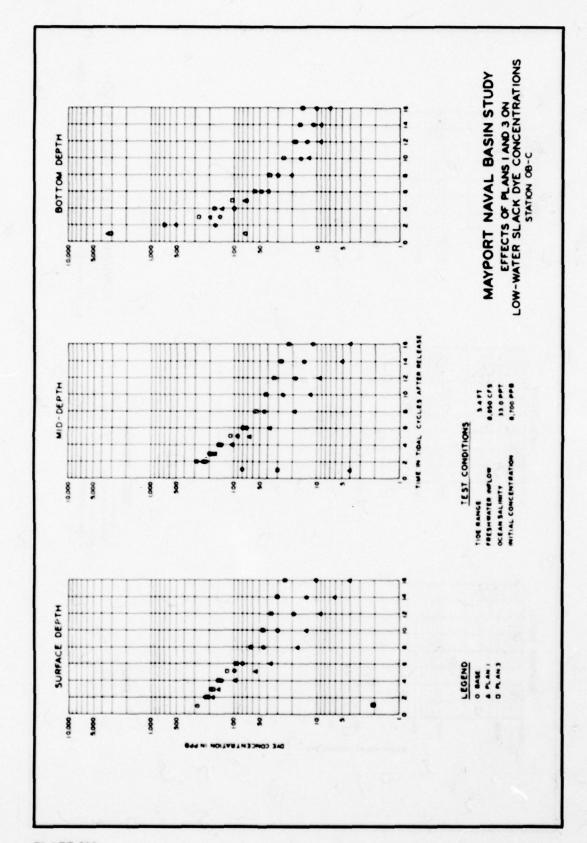
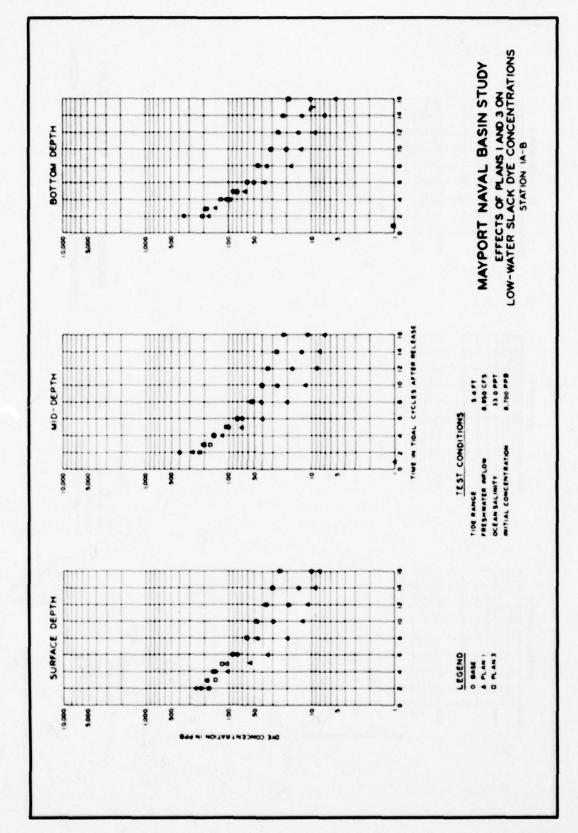
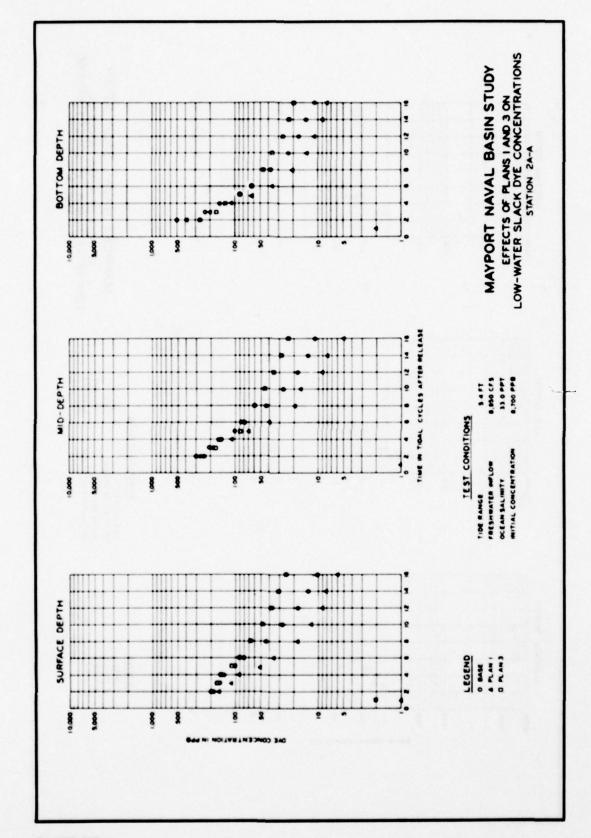
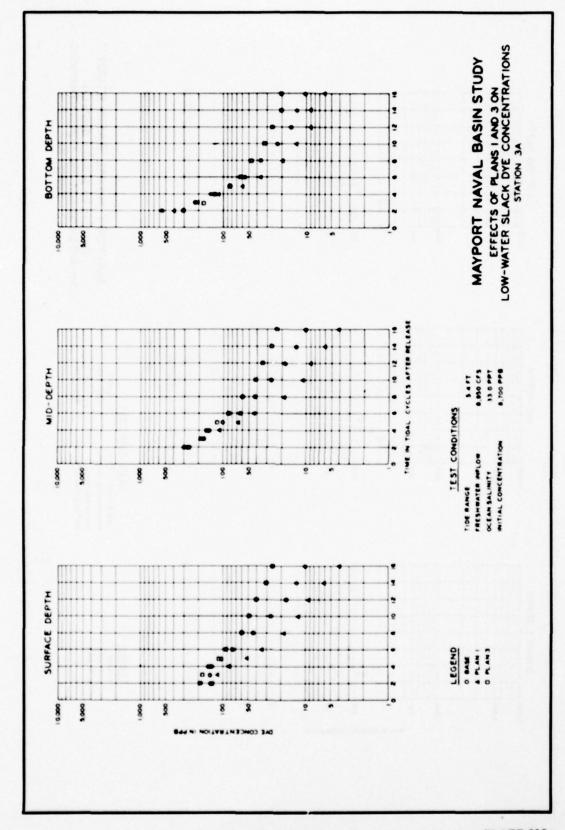
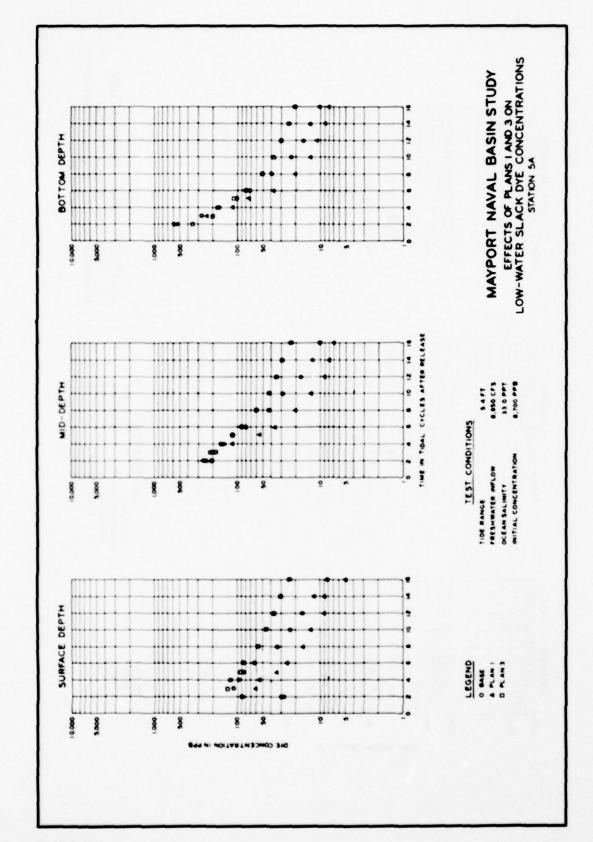


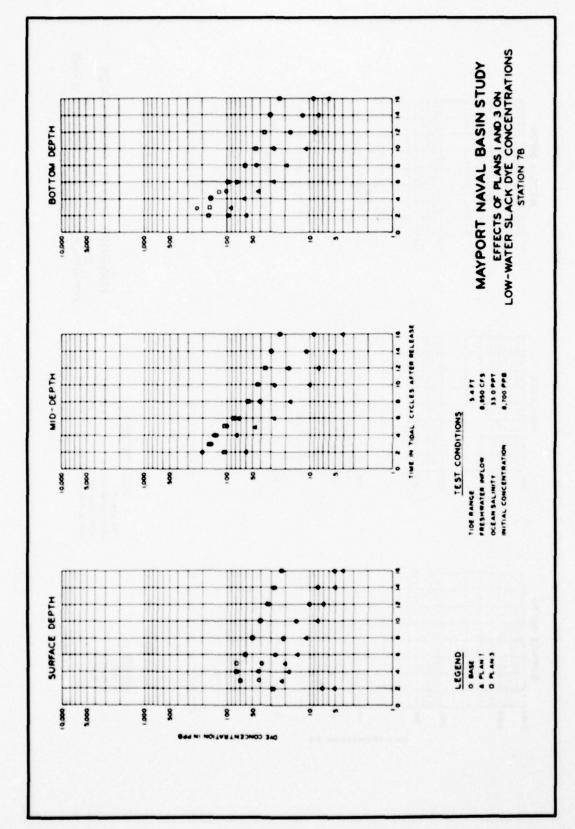
PLATE 210

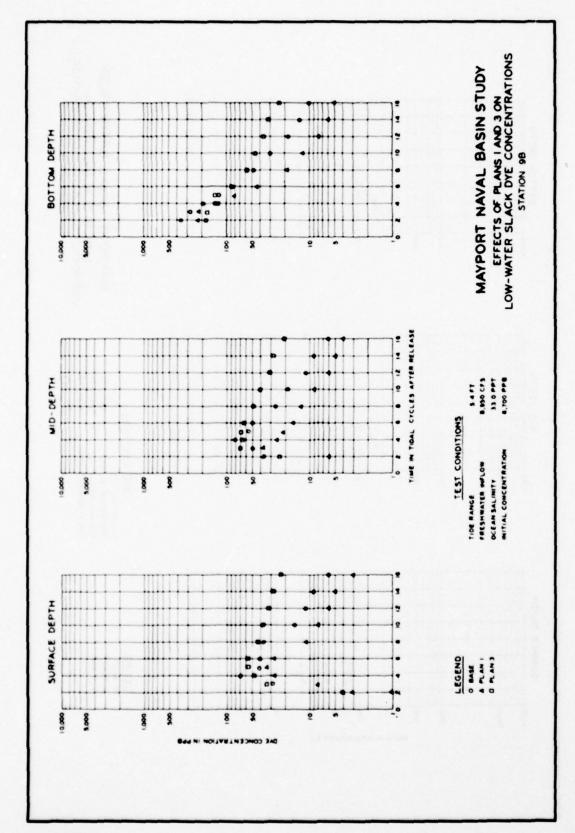


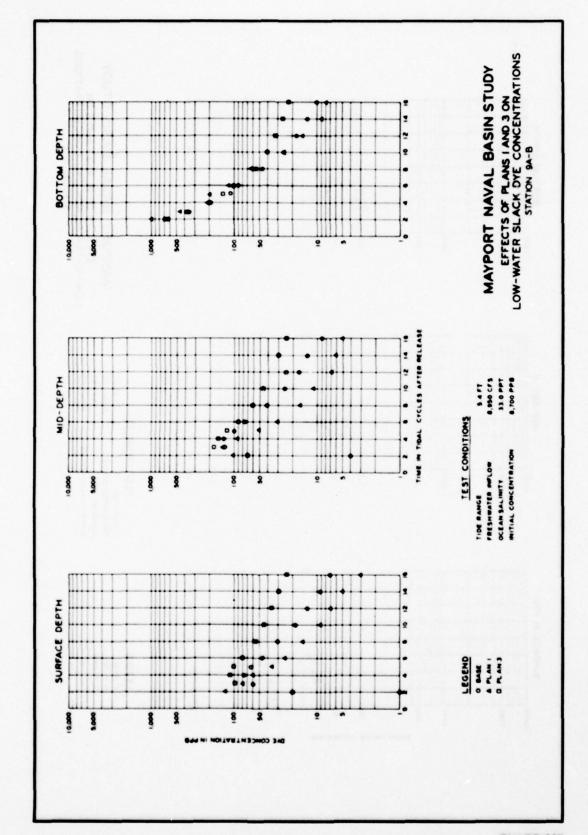


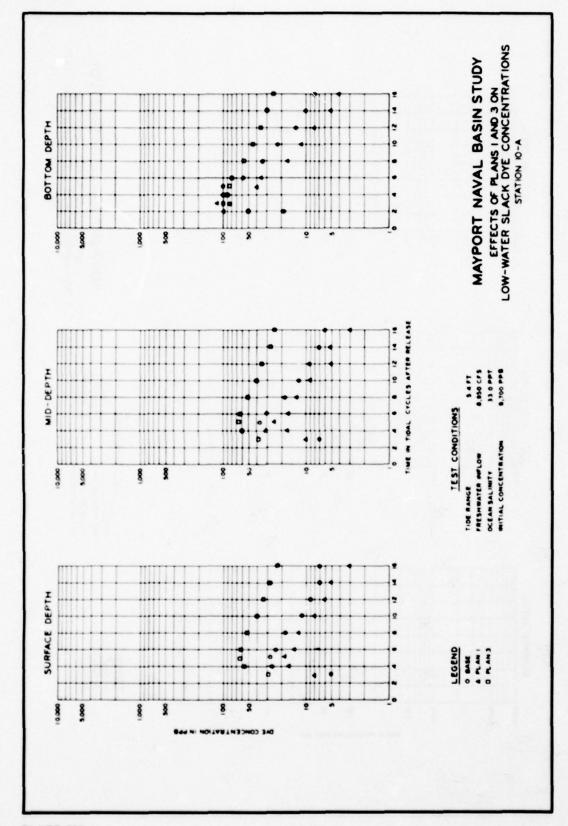


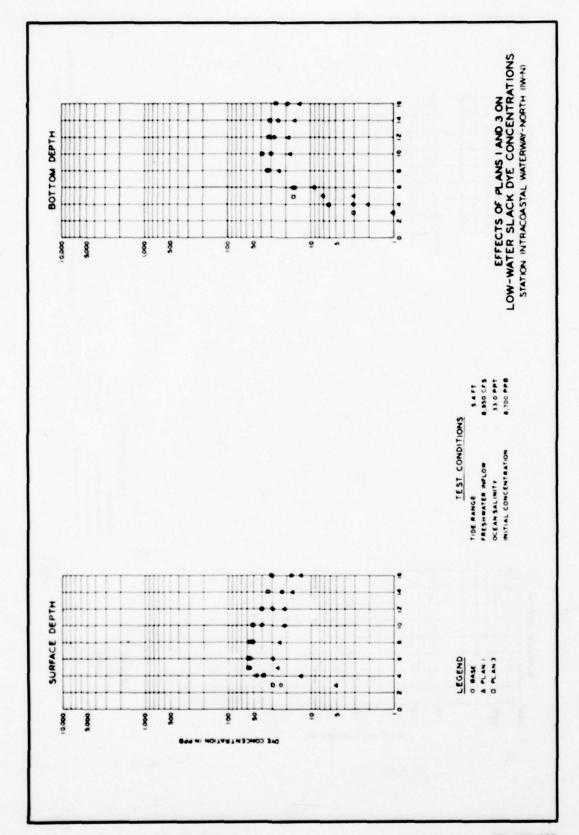


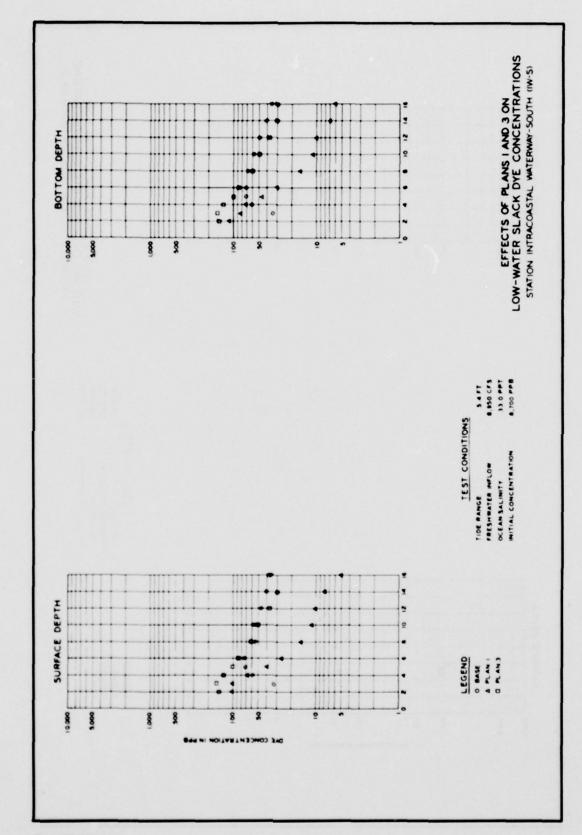


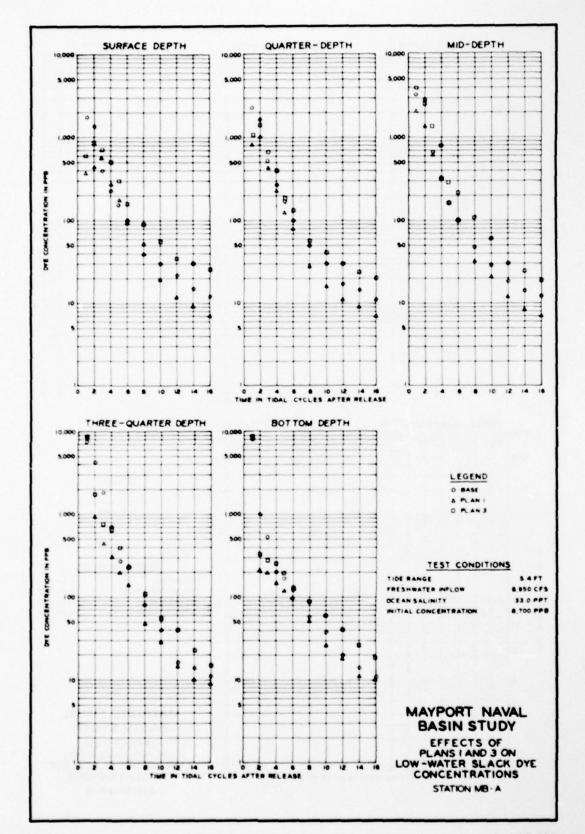


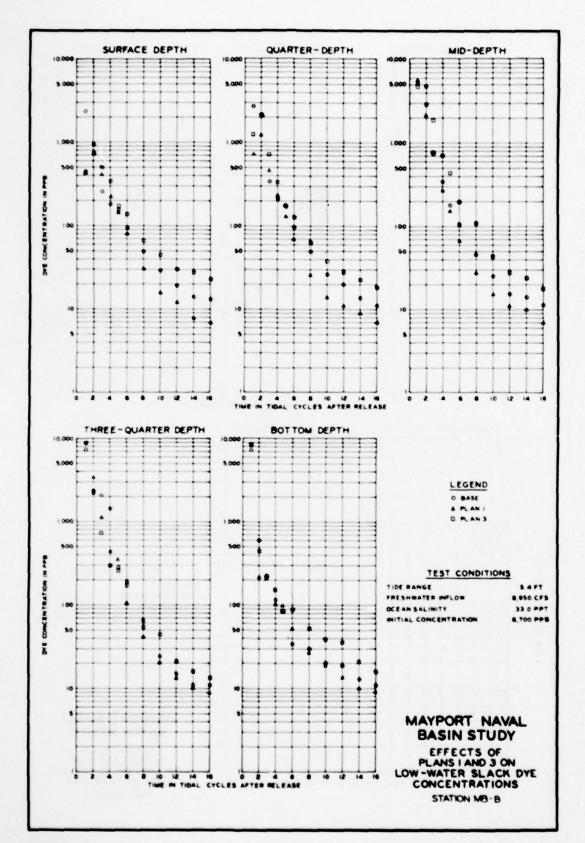


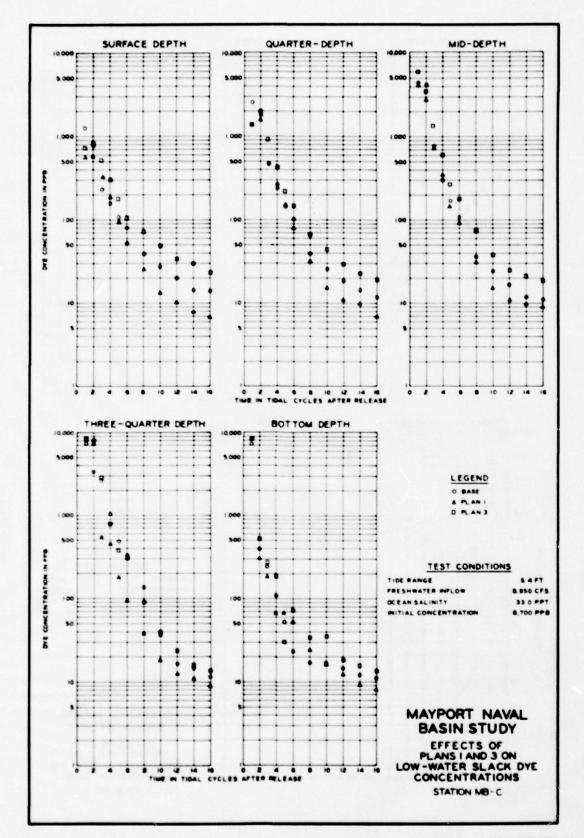












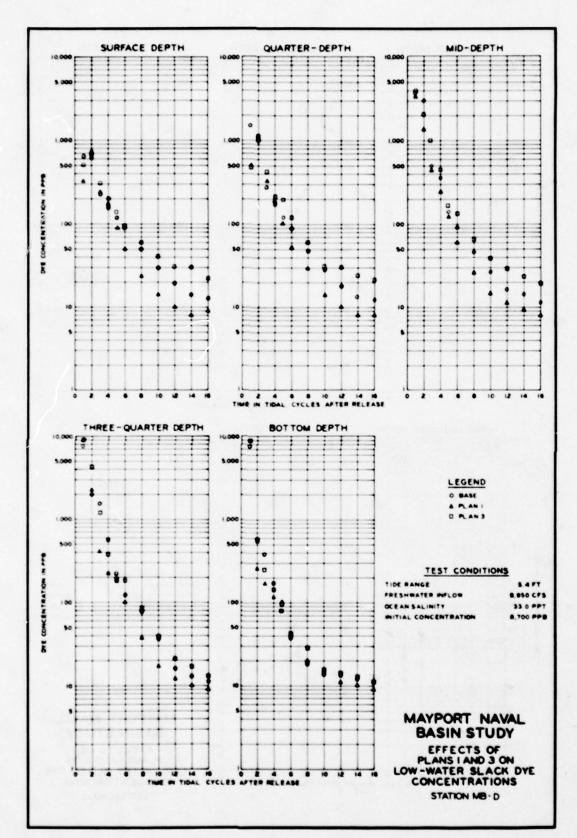
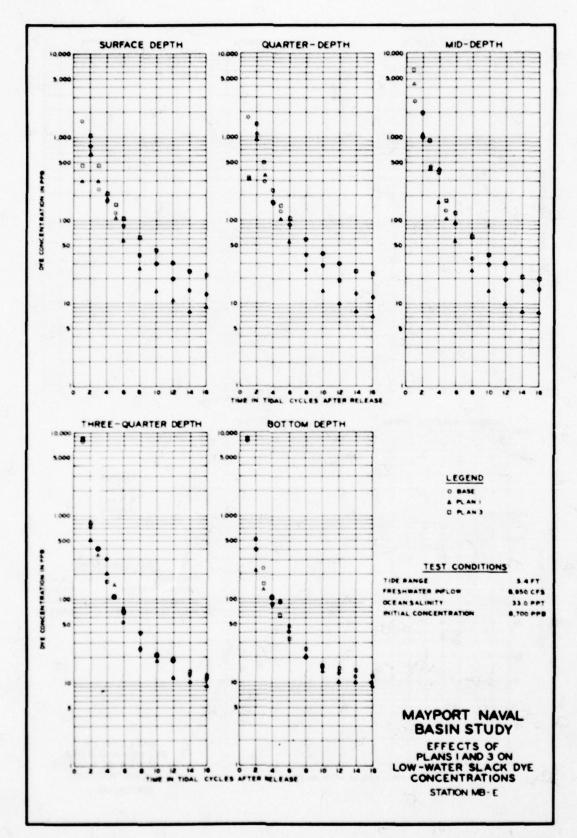
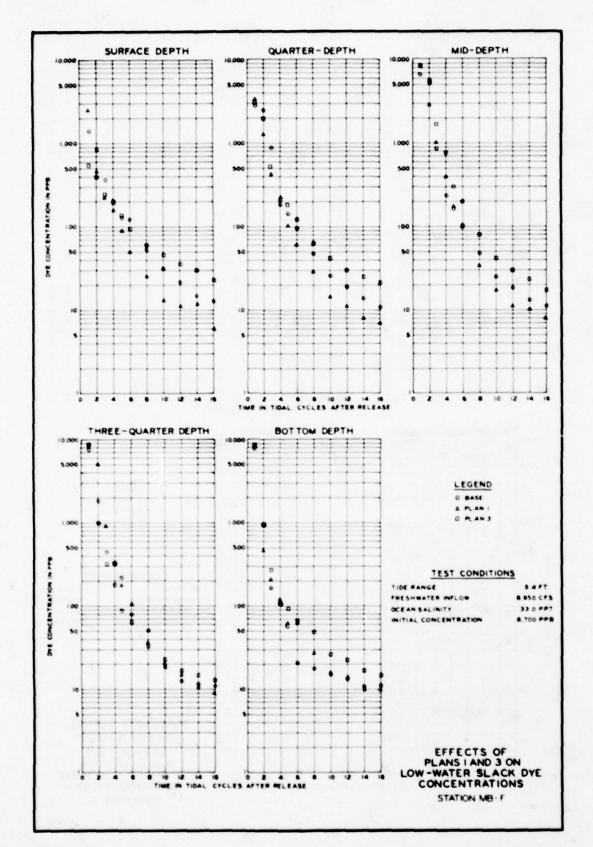
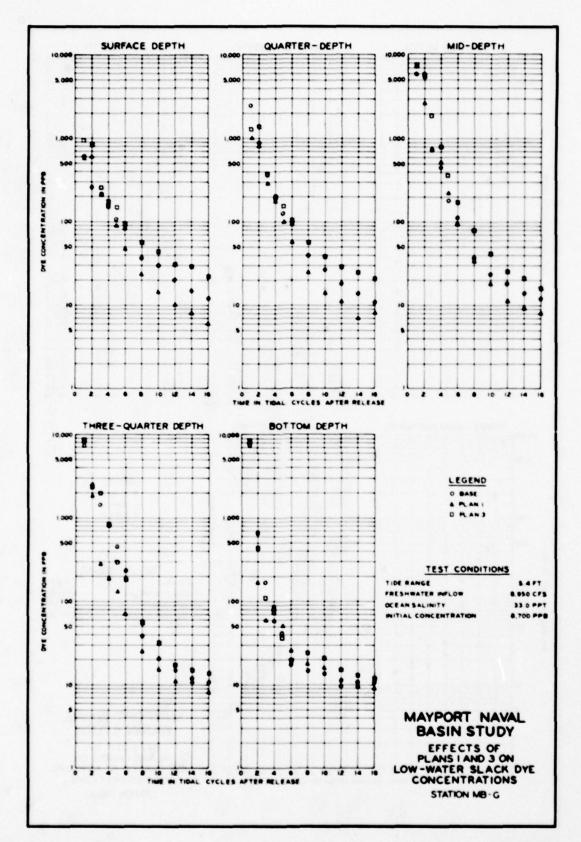
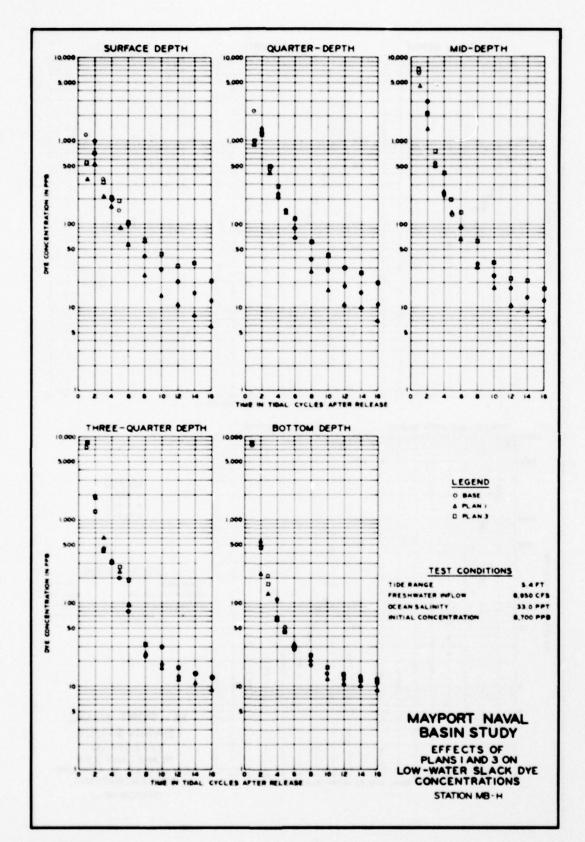


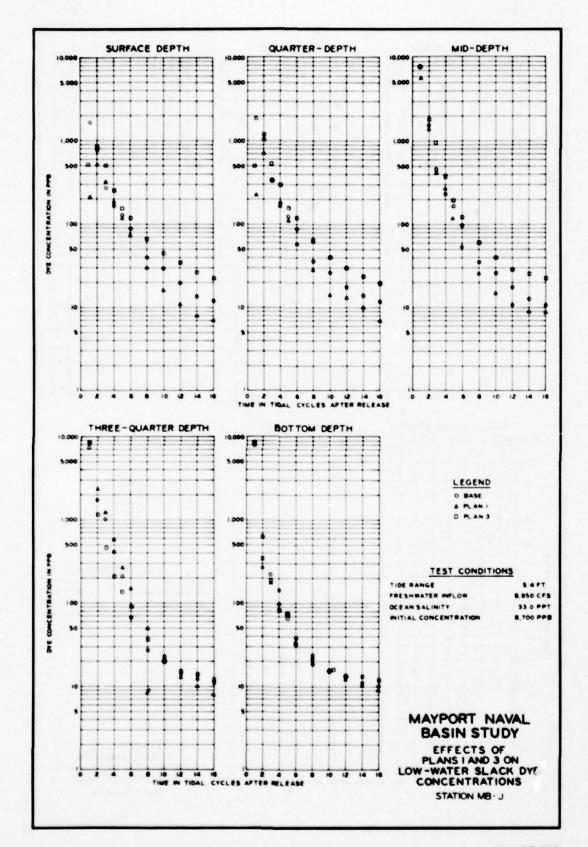
PLATE 224

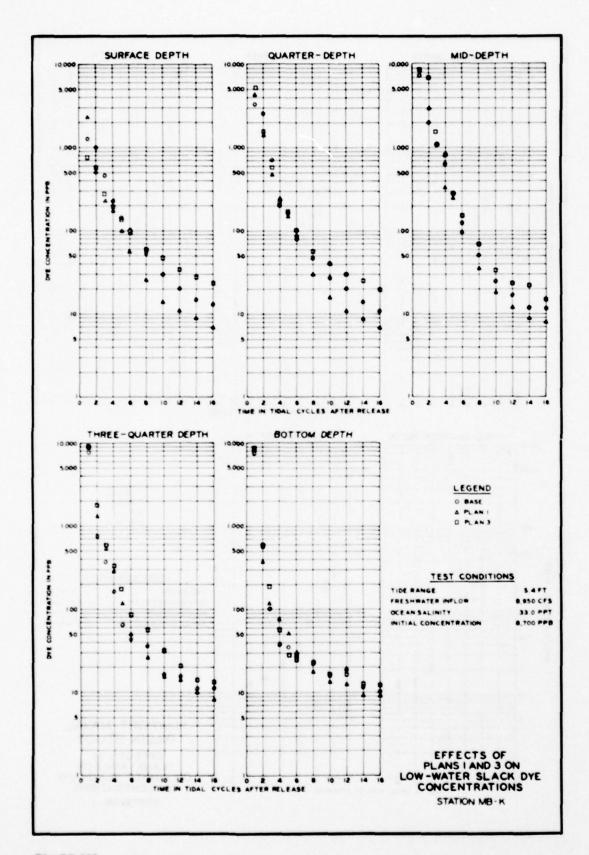


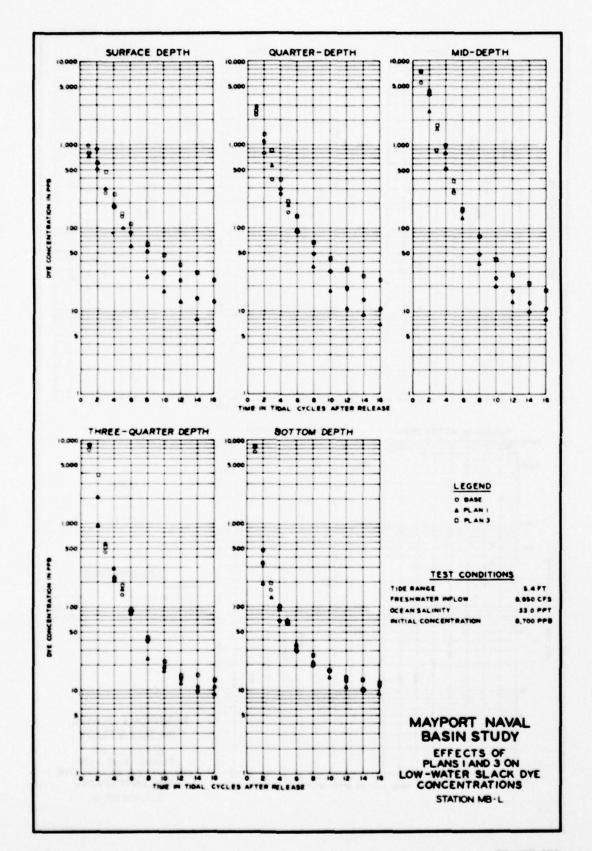


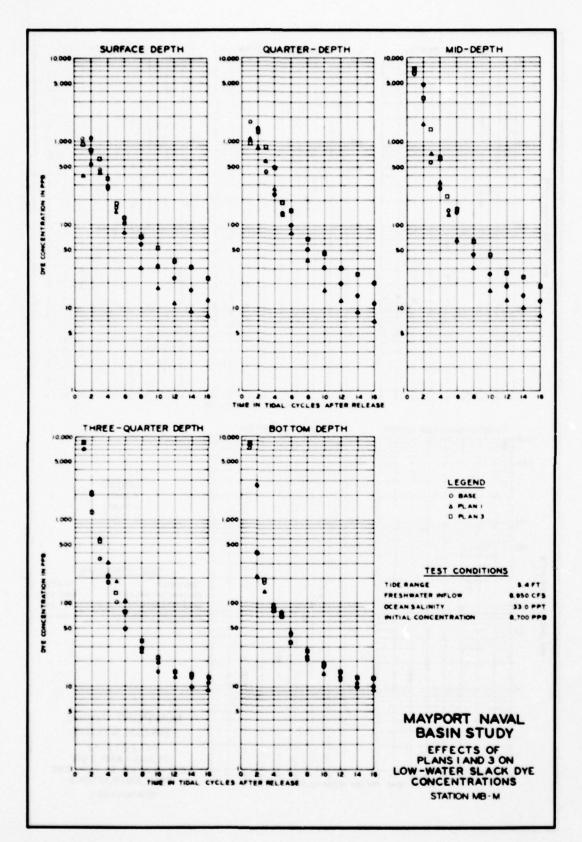


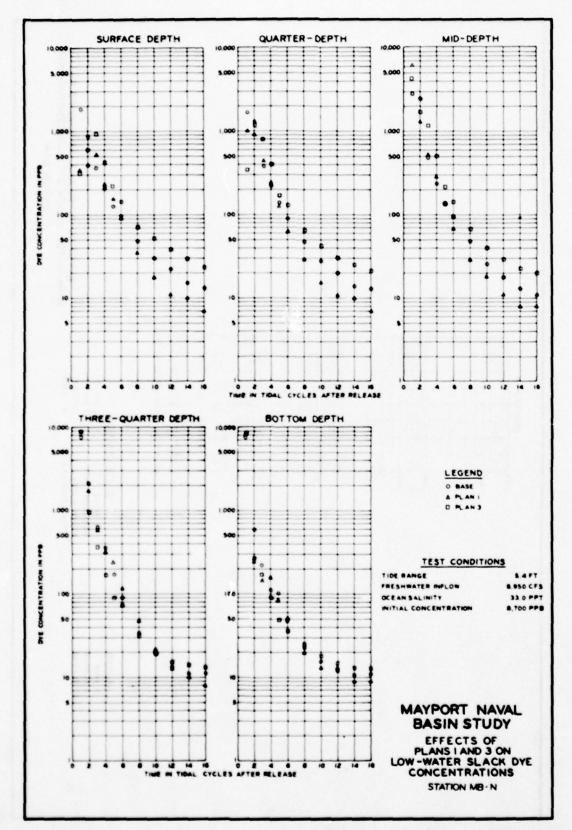




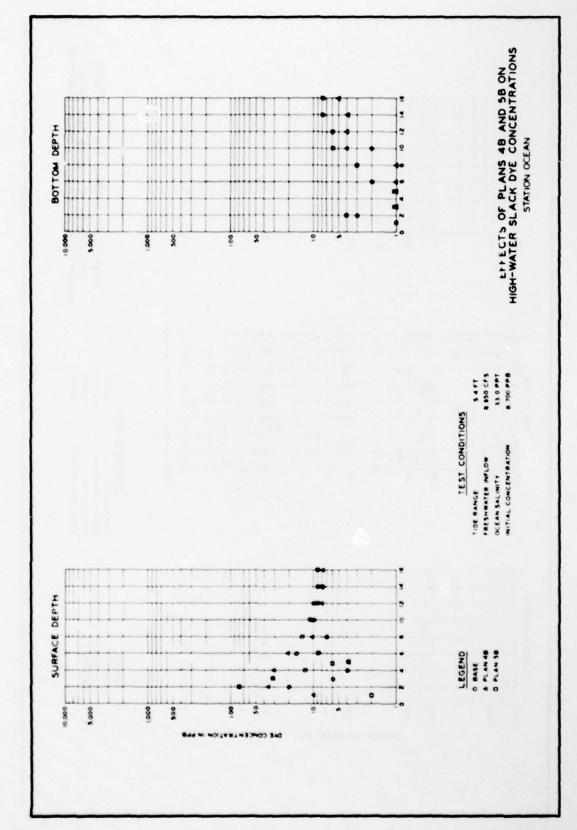








EFFECTS OF PLANS 4B AND 5B ON HIGH-WATER SLACK DYE CONCENTRATIONS STATION SUMP TIME IN TIDAL CYCLES AFTER RELEASE 0 2 1 1 10 33 0 PFT MID-DEPTH TEST CONDITIONS D**O** TIDE RANGE FRESHWATER INFLOW OCEAN SALINITY INITIAL CONCENTRATION 8 8 5 8 886 0000 O BASE A PLANSE O PLANSE



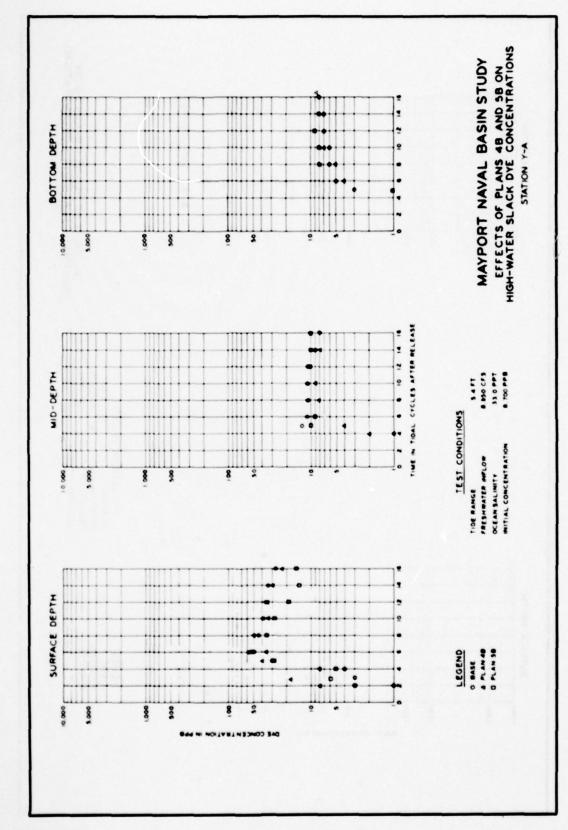
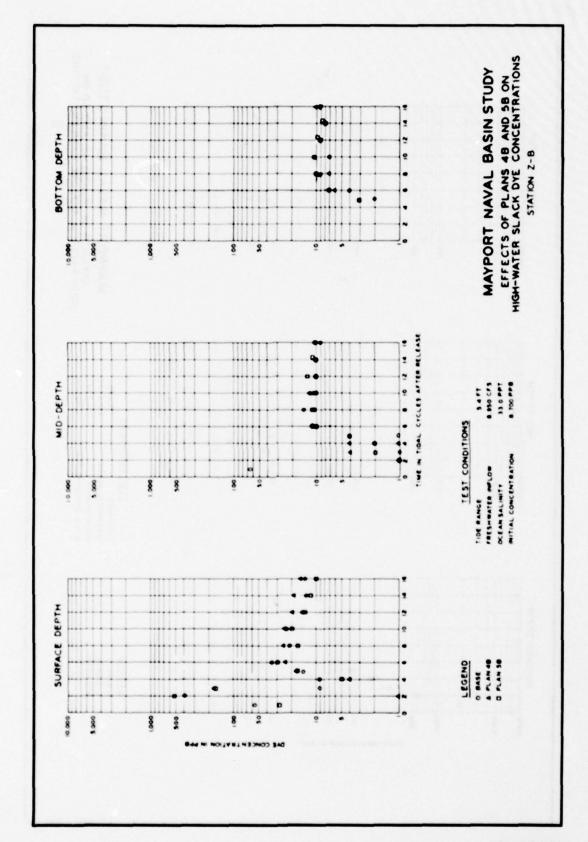
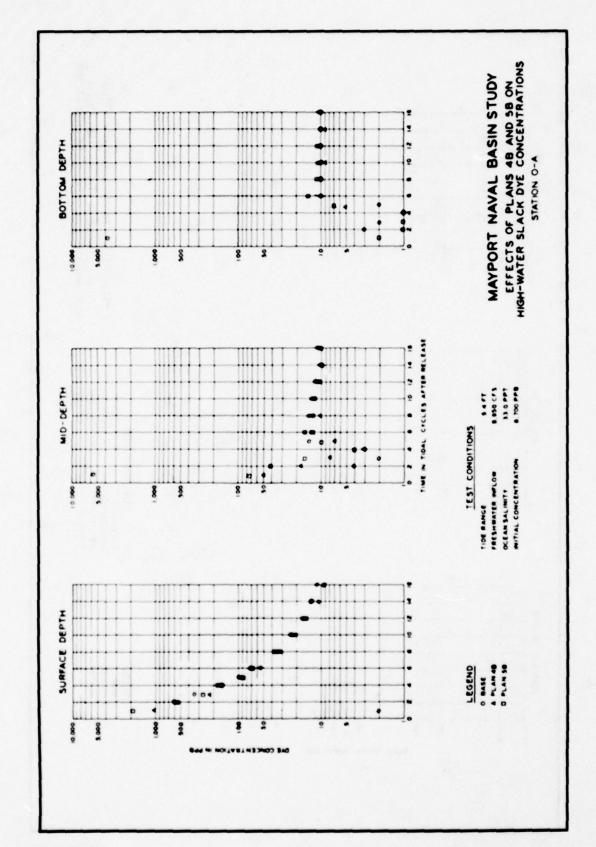
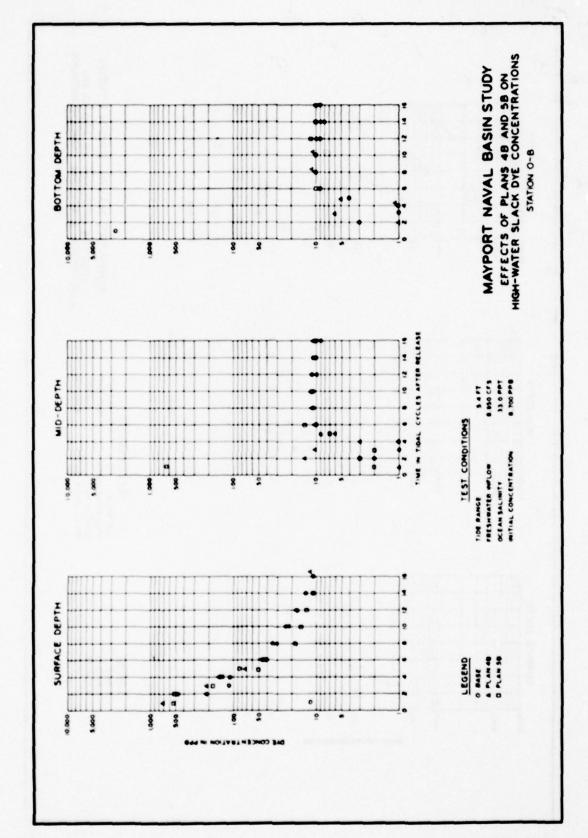
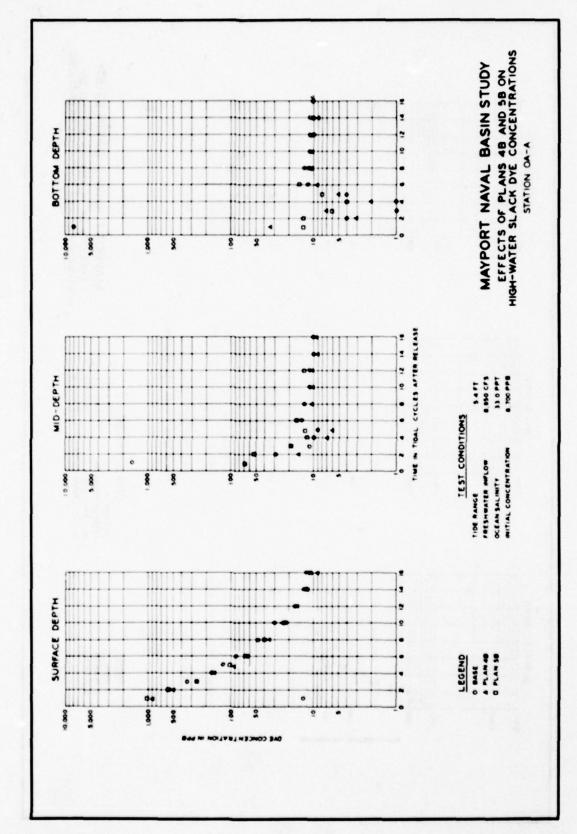


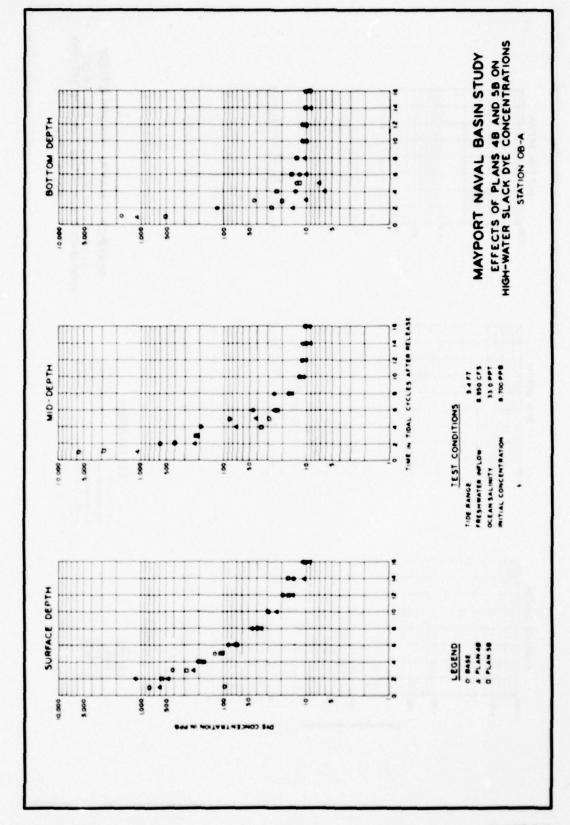
PLATE 236

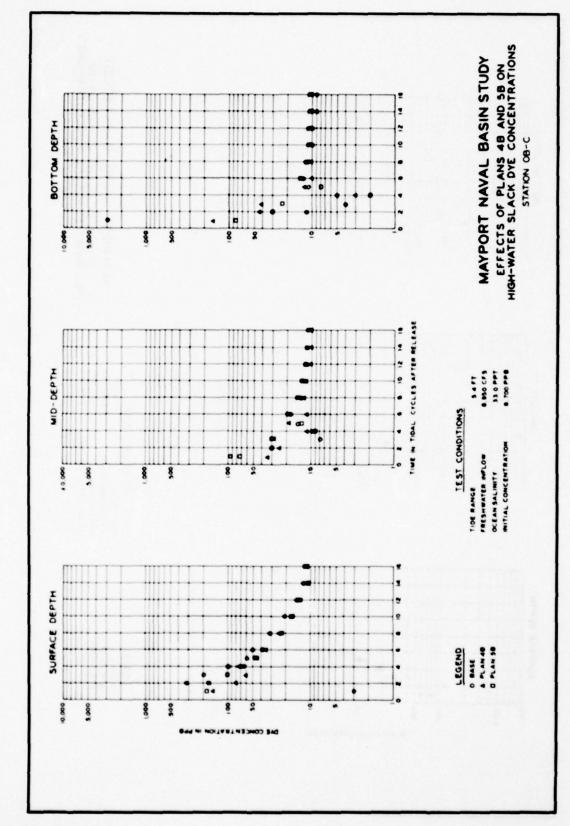


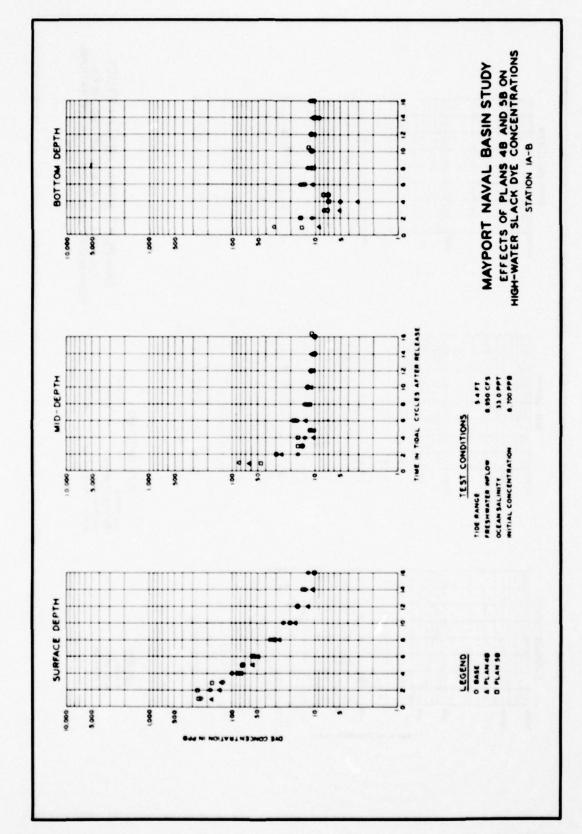


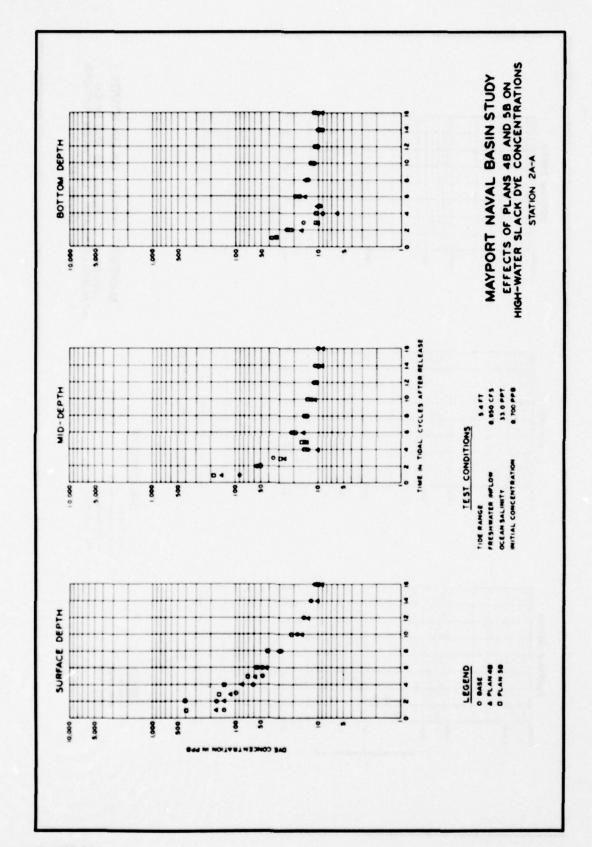


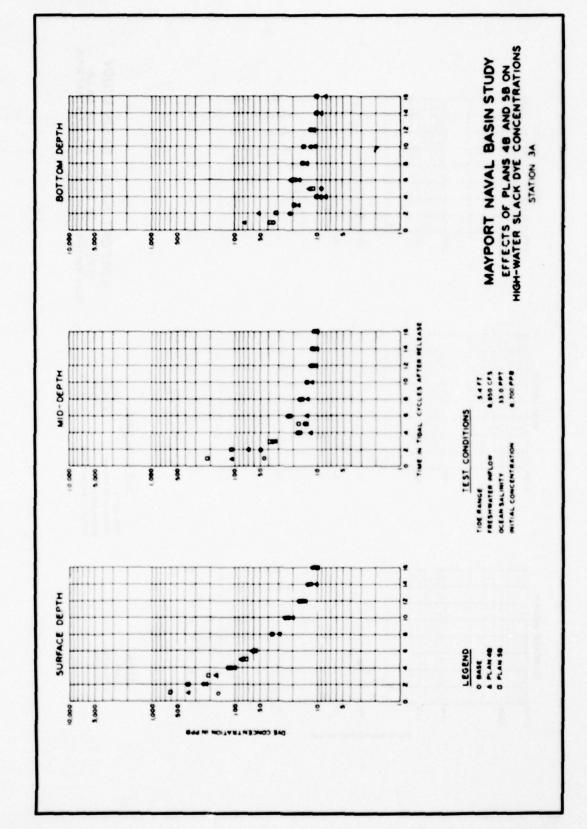


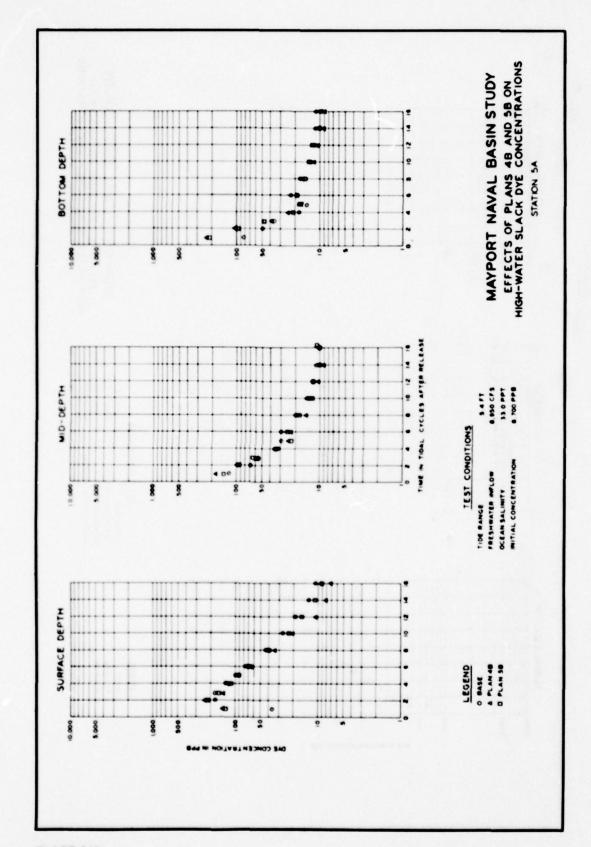


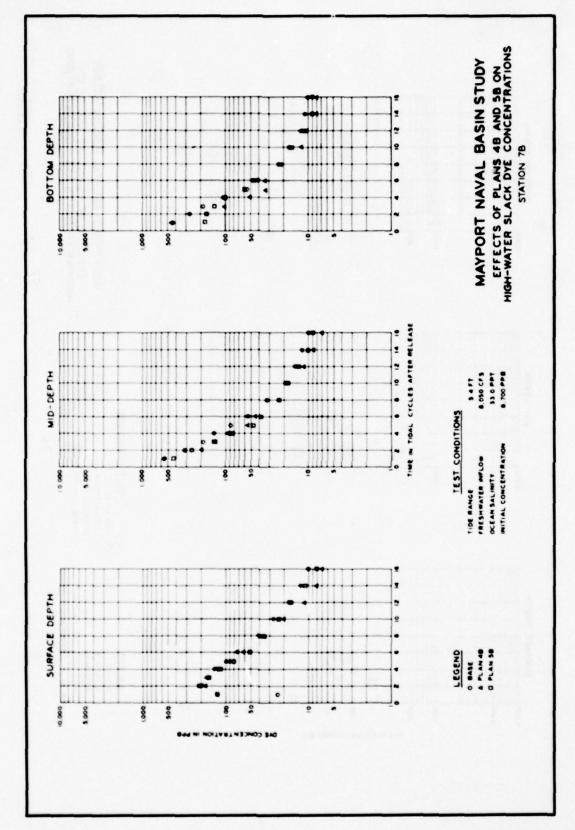


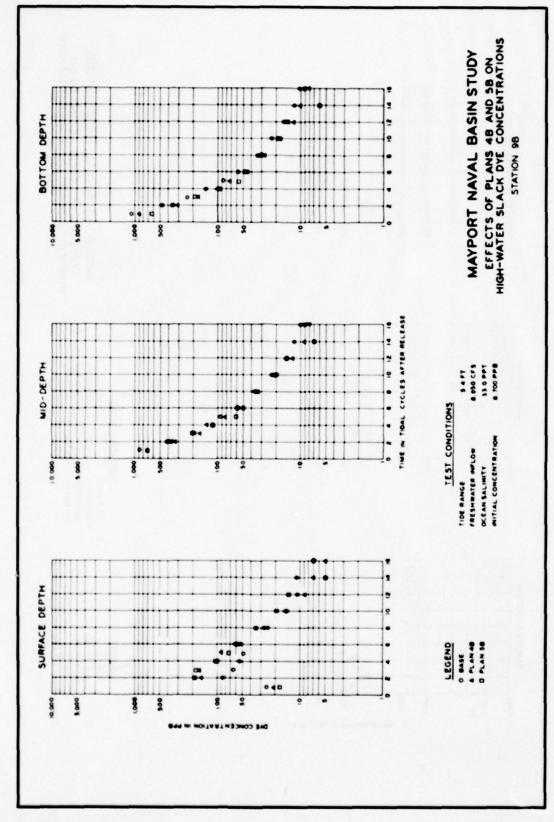


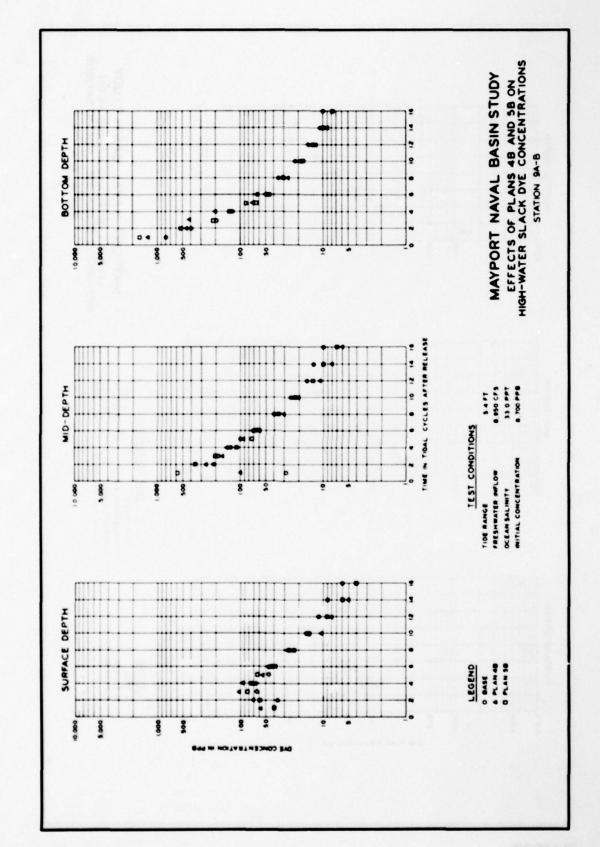


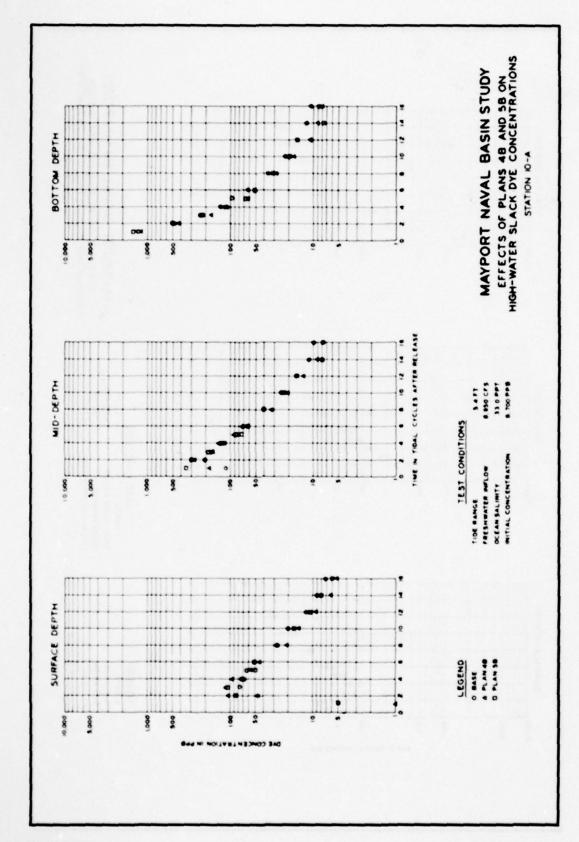


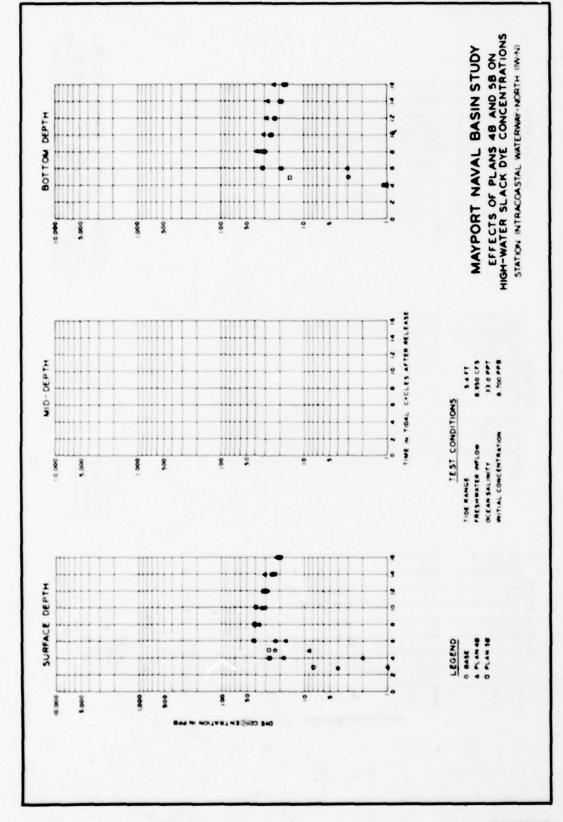


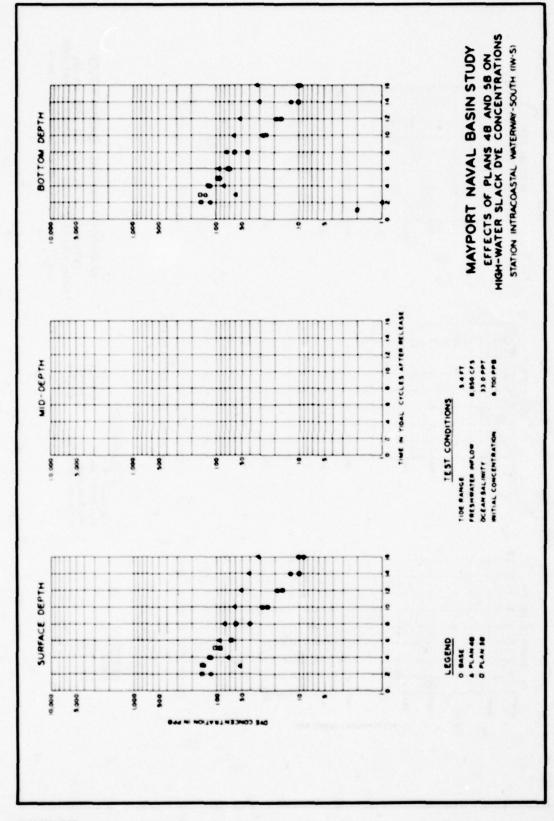


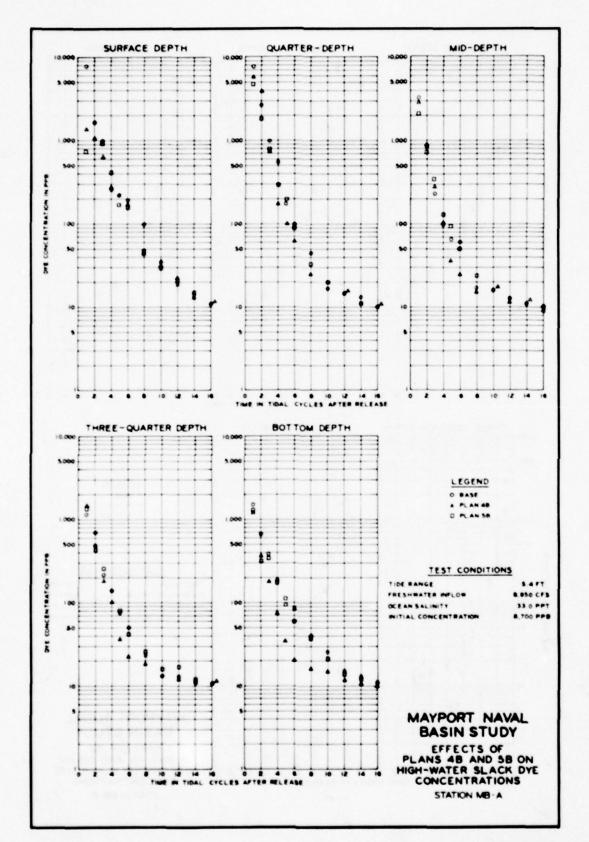


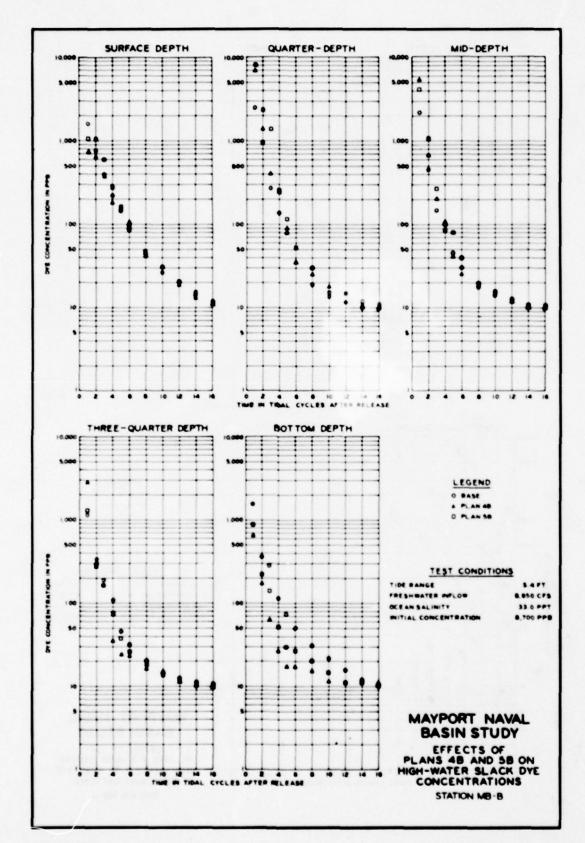


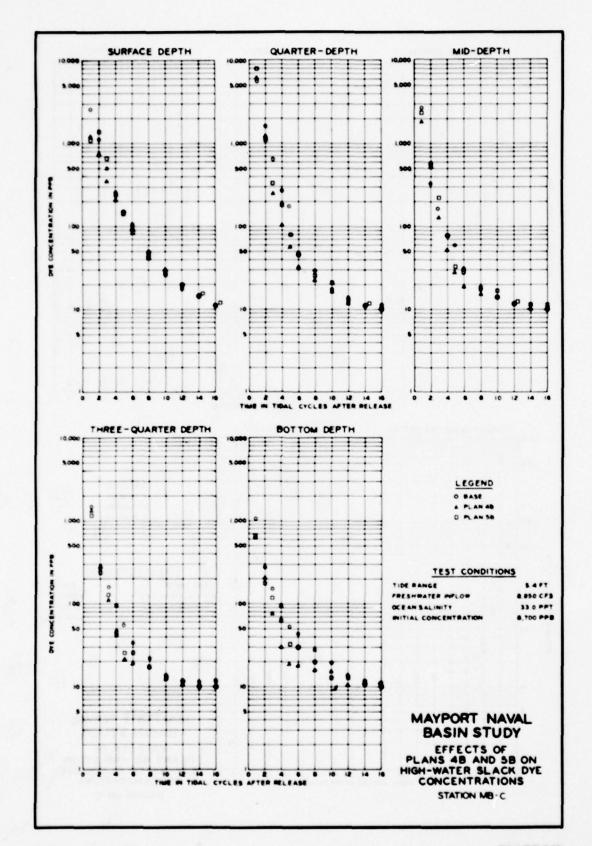


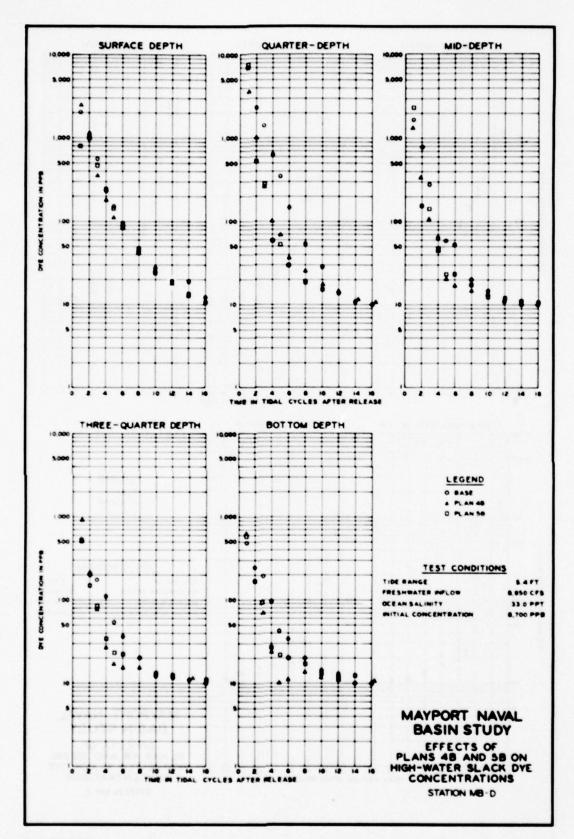


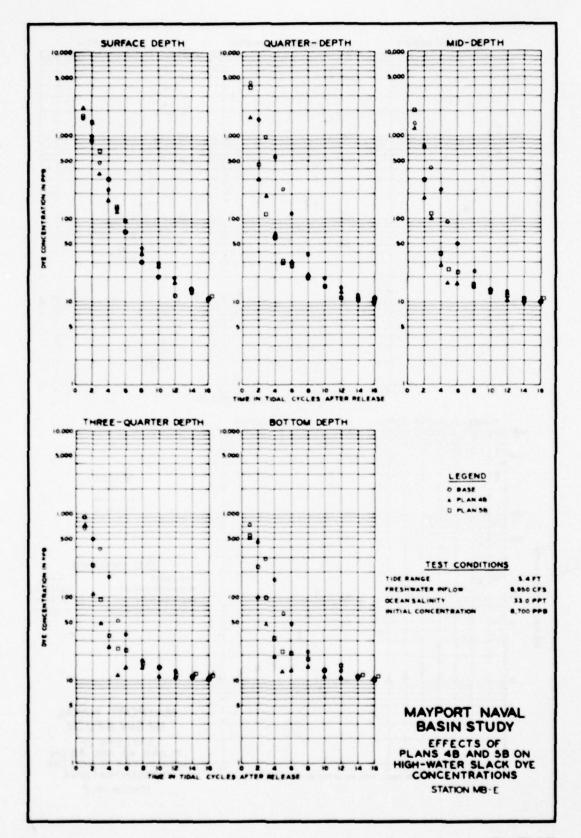


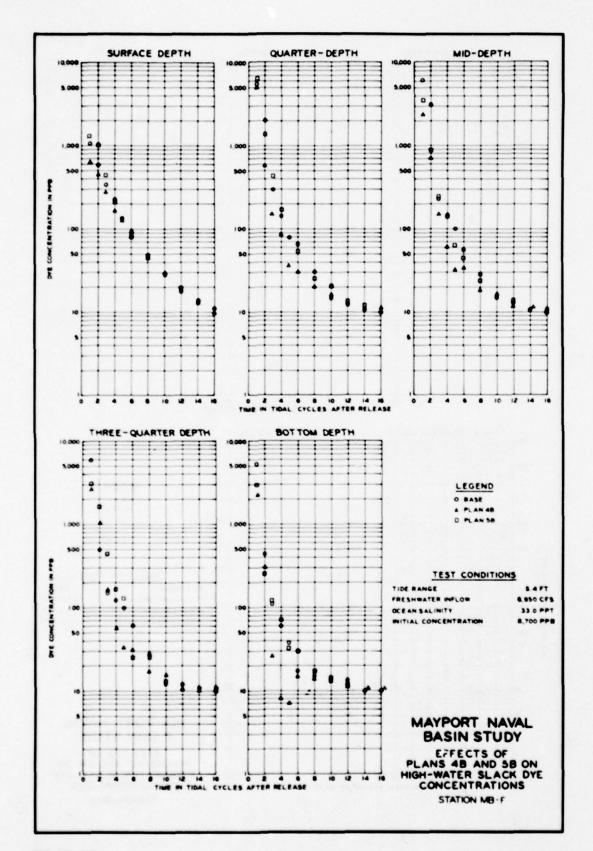


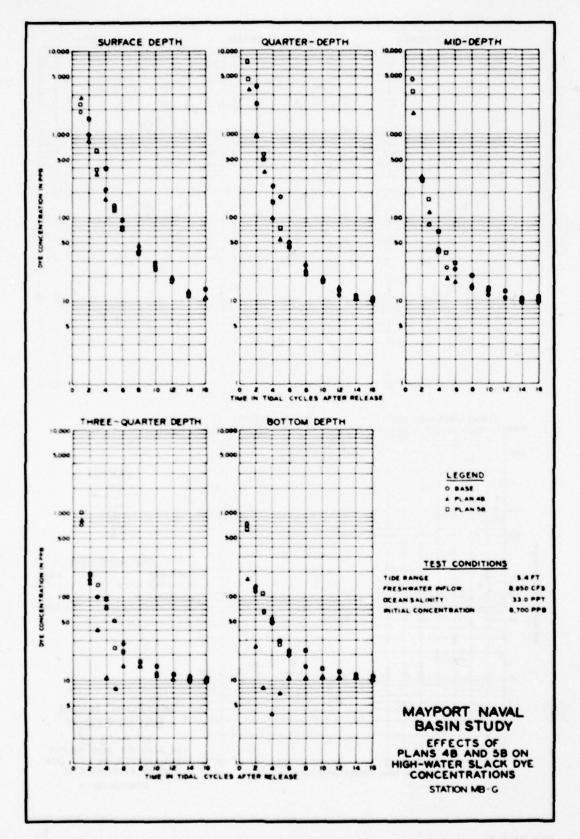


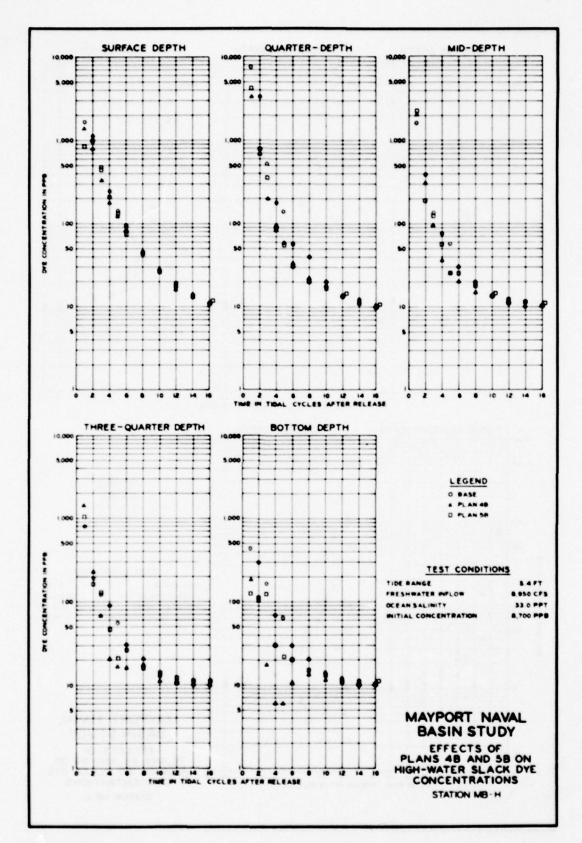


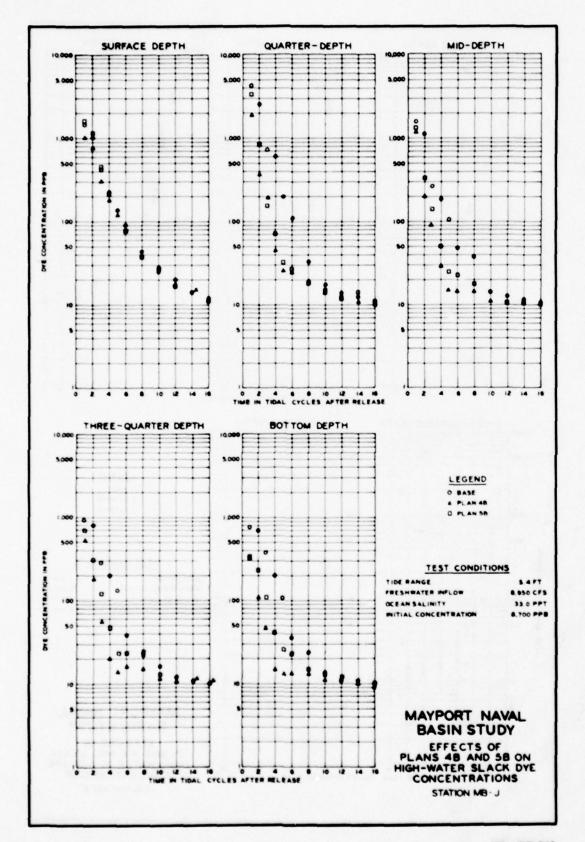


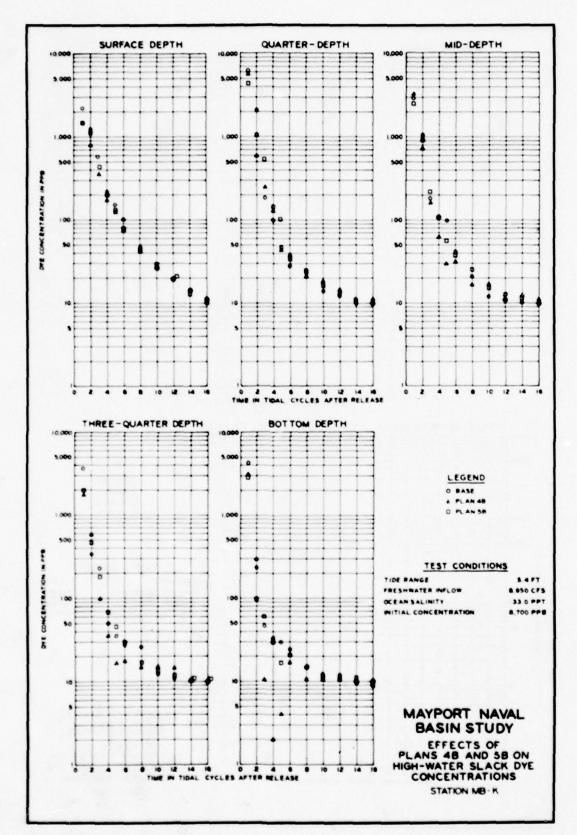


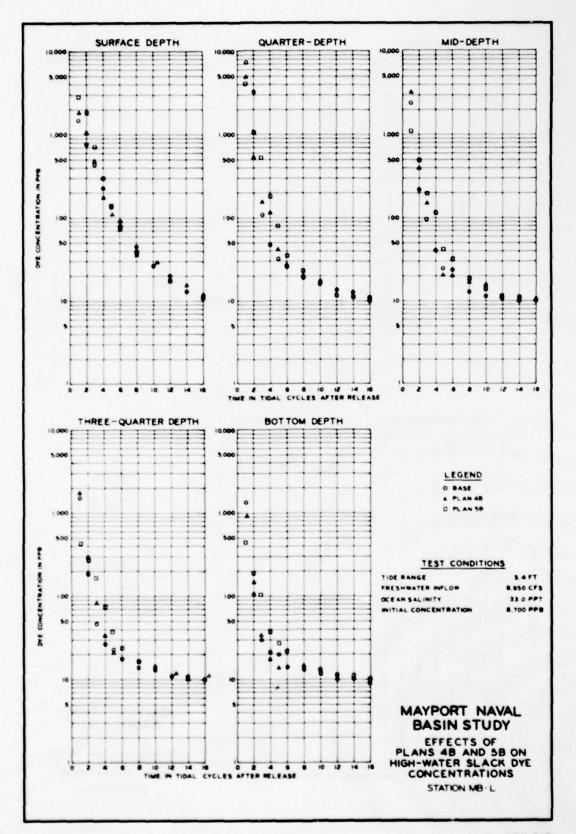


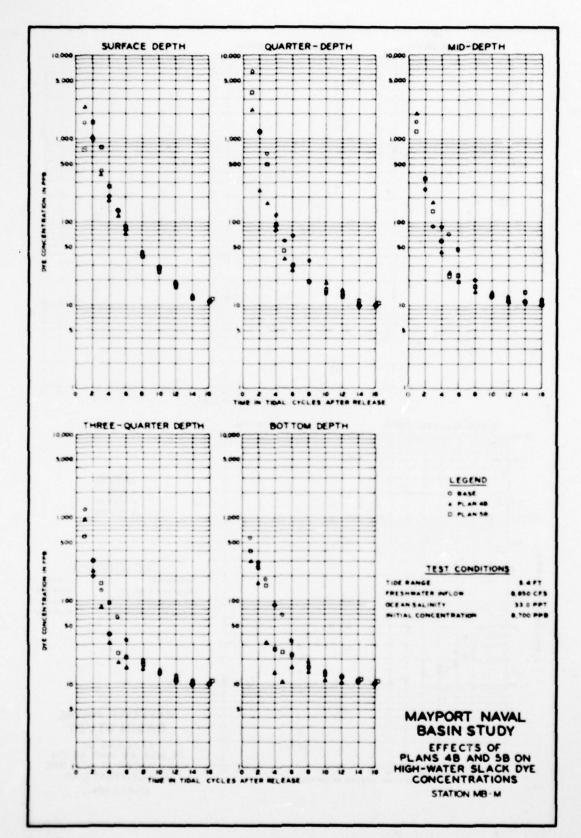


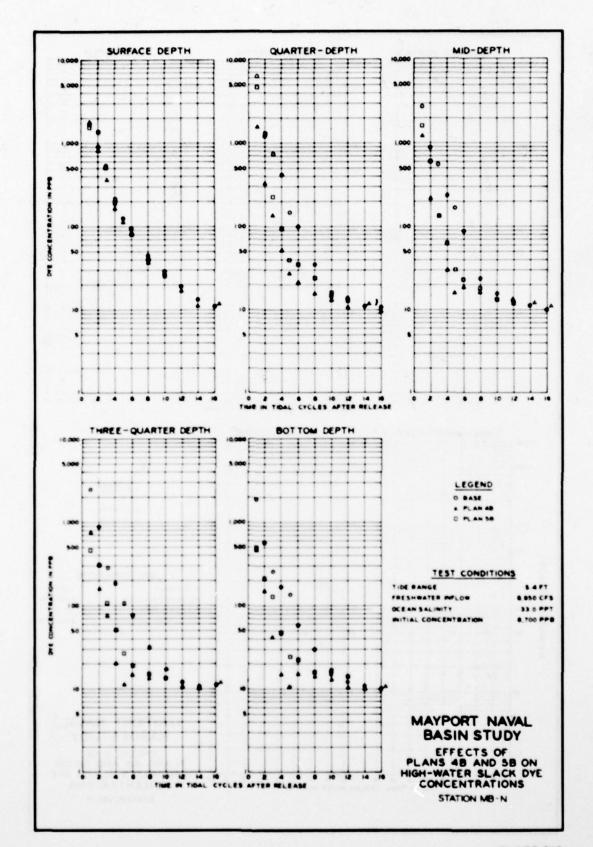


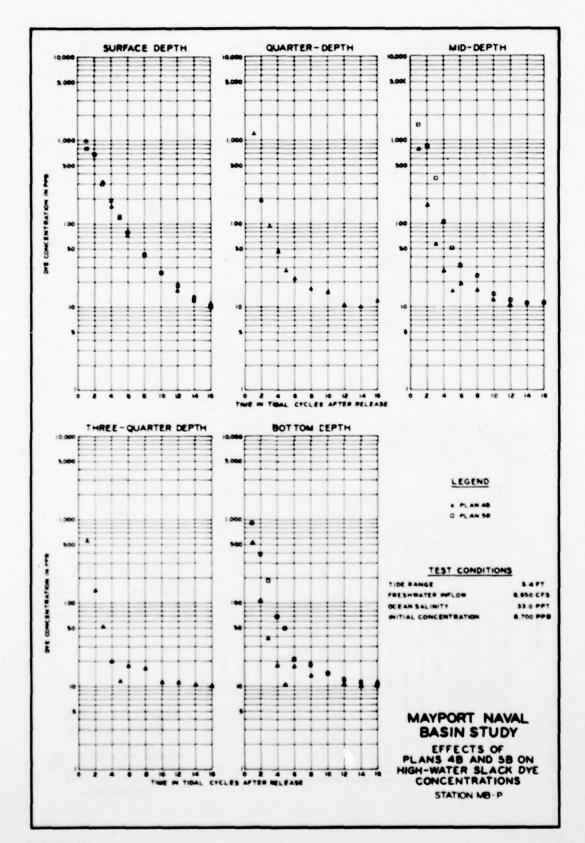


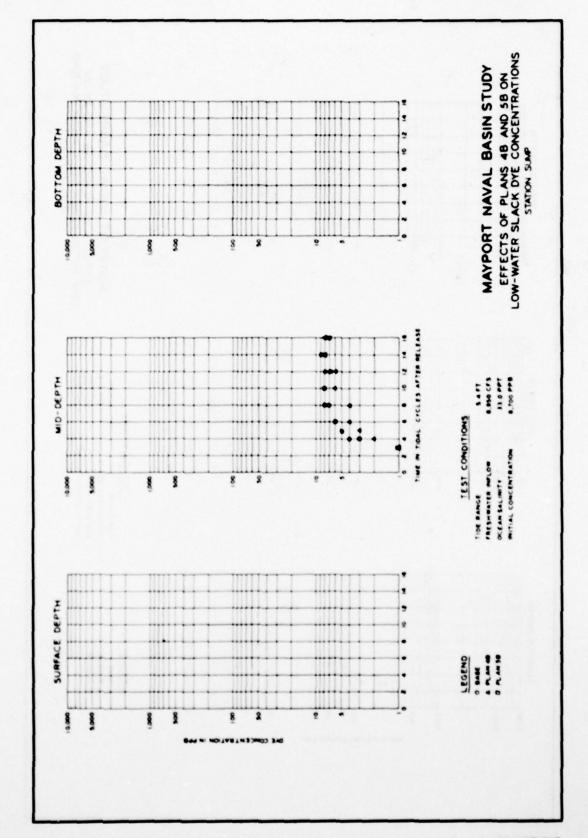


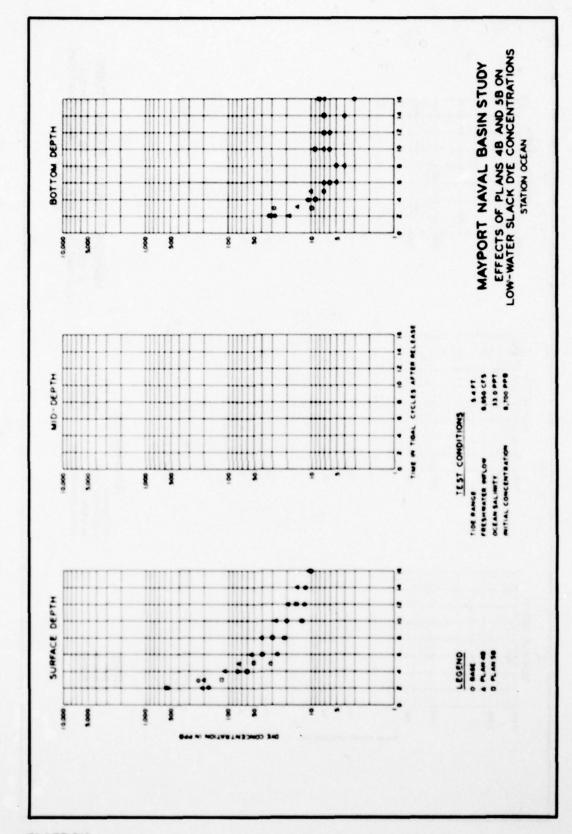


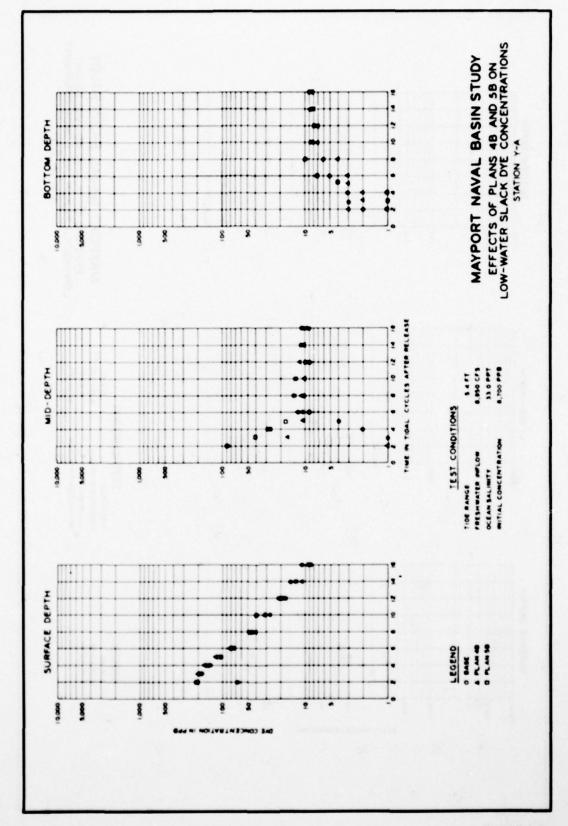


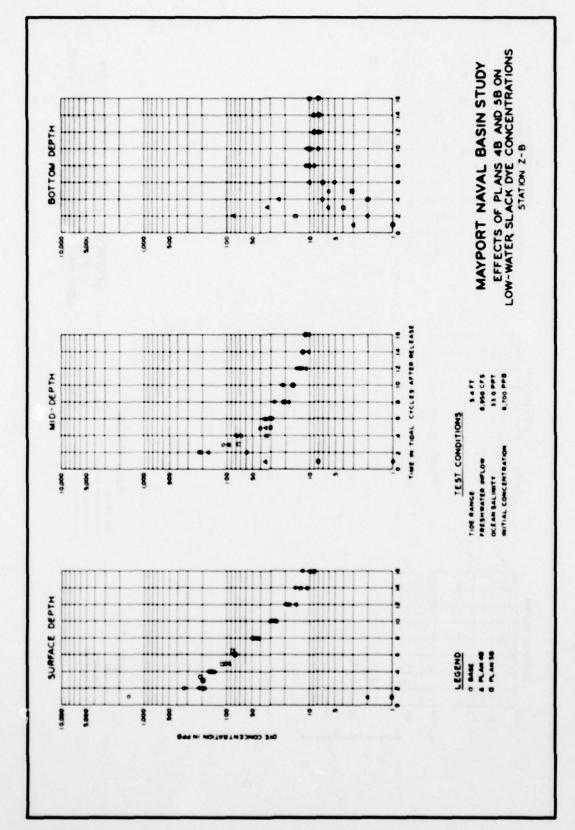


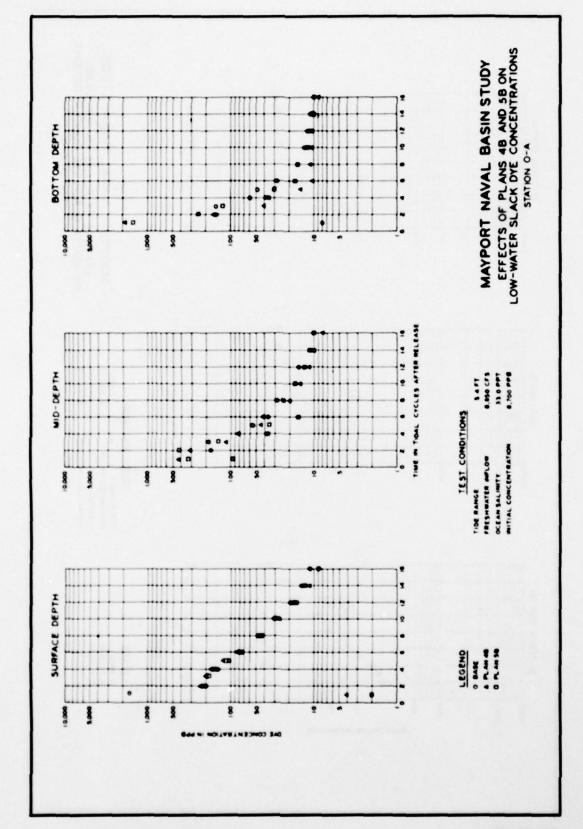












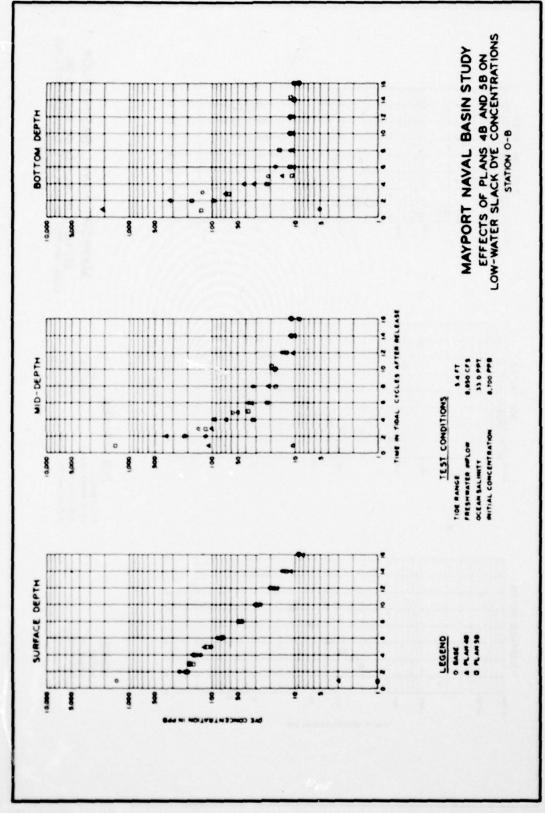
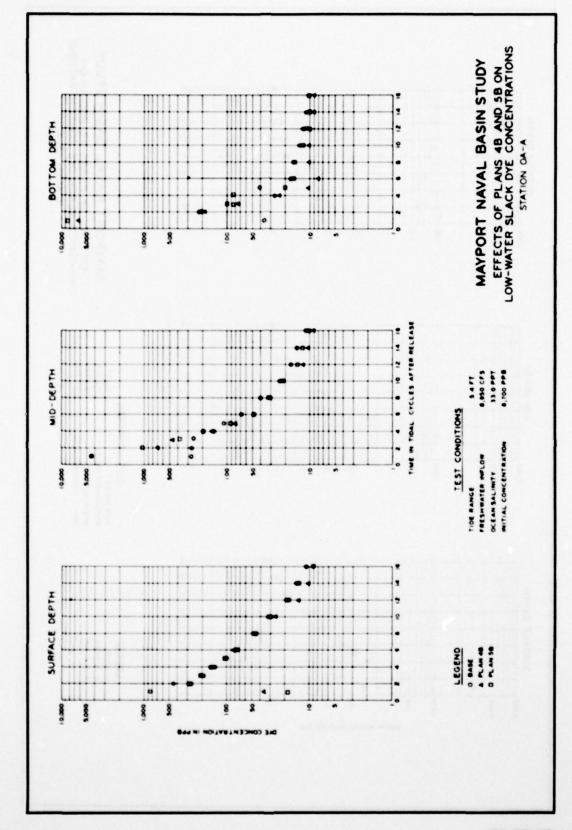
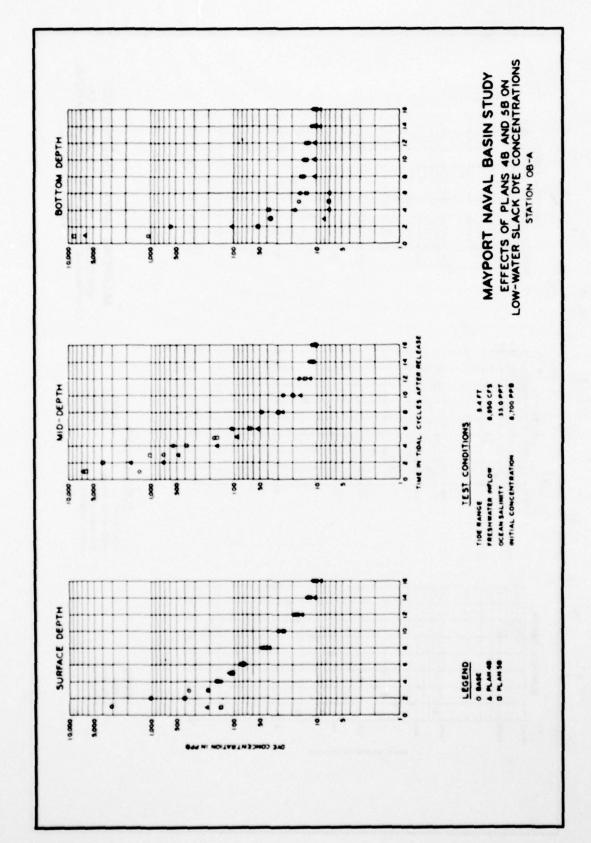
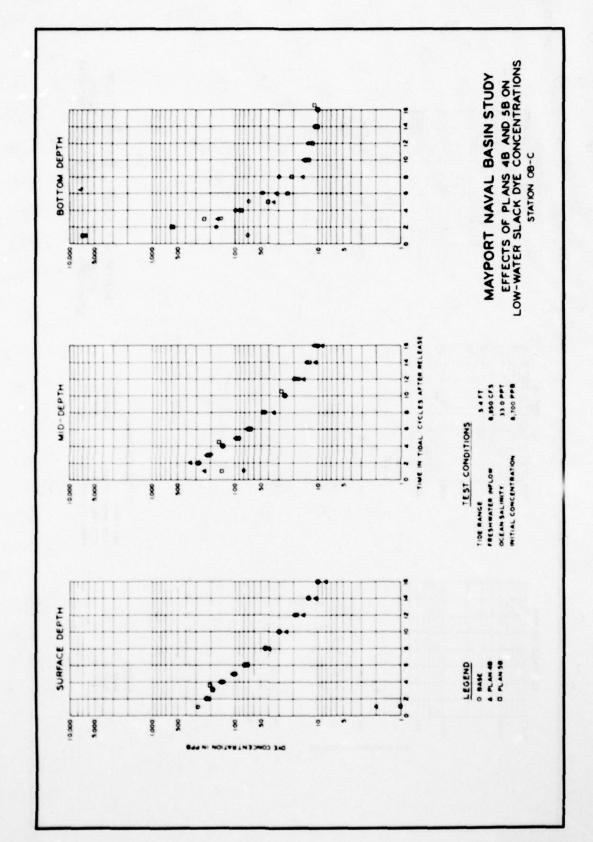
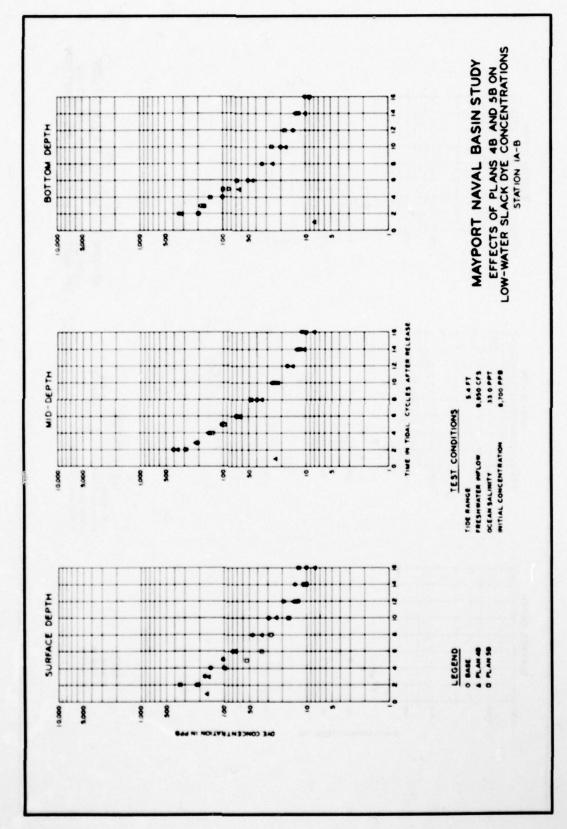


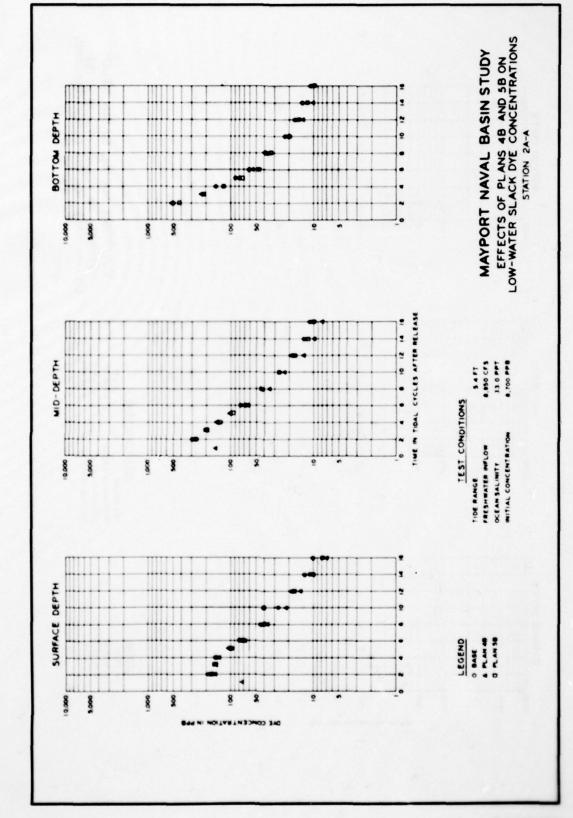
PLATE 272

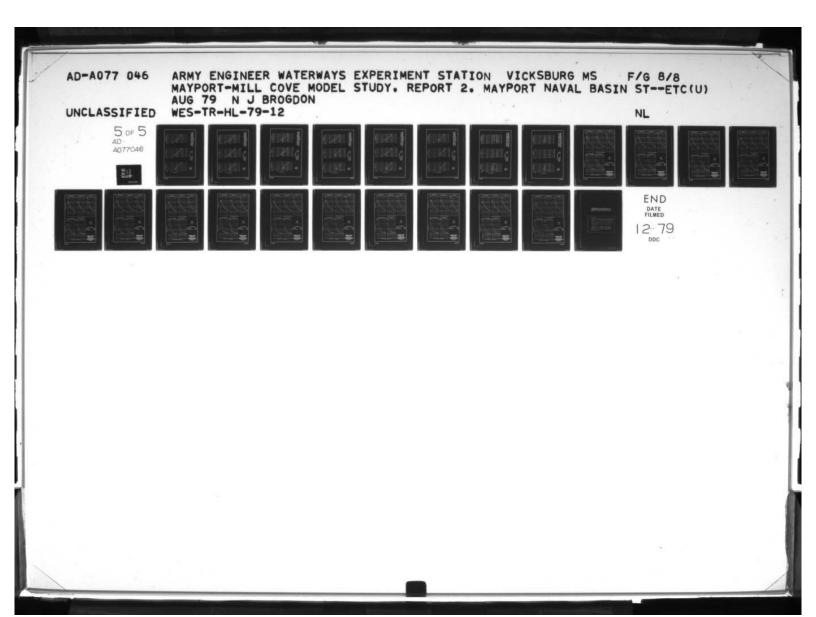


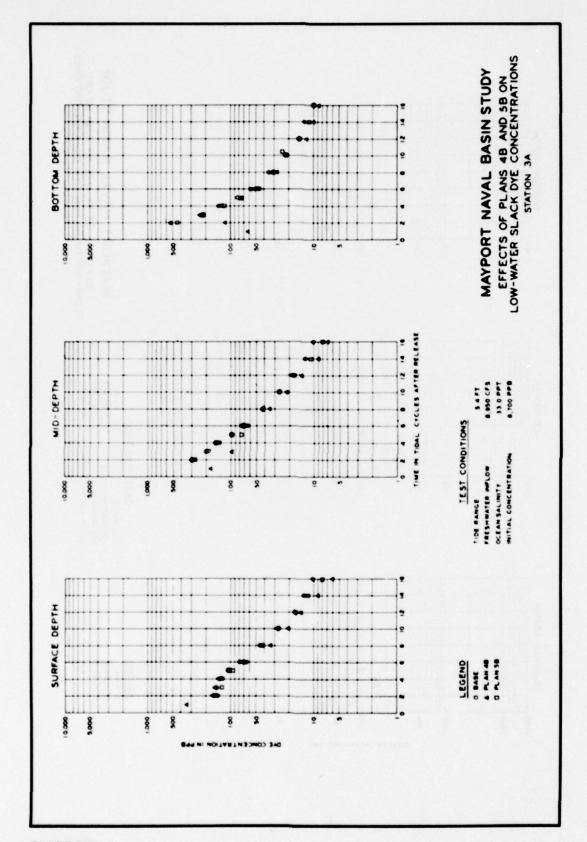


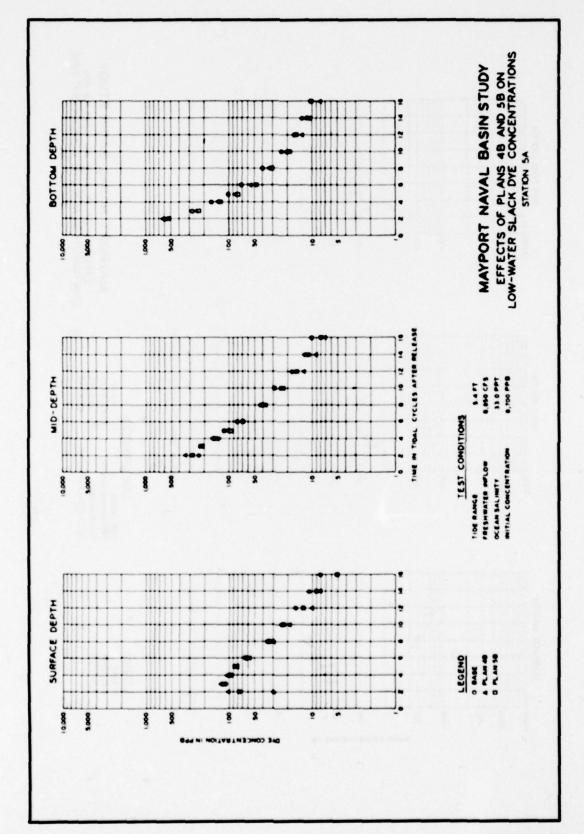


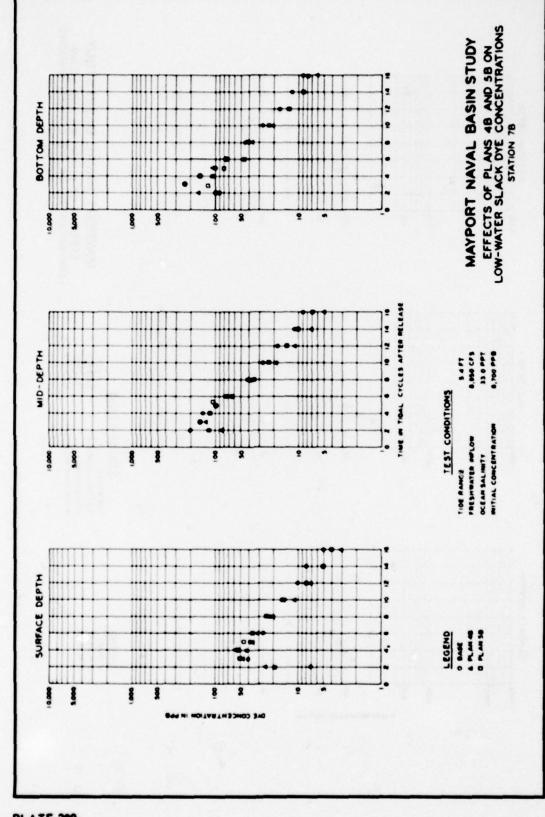


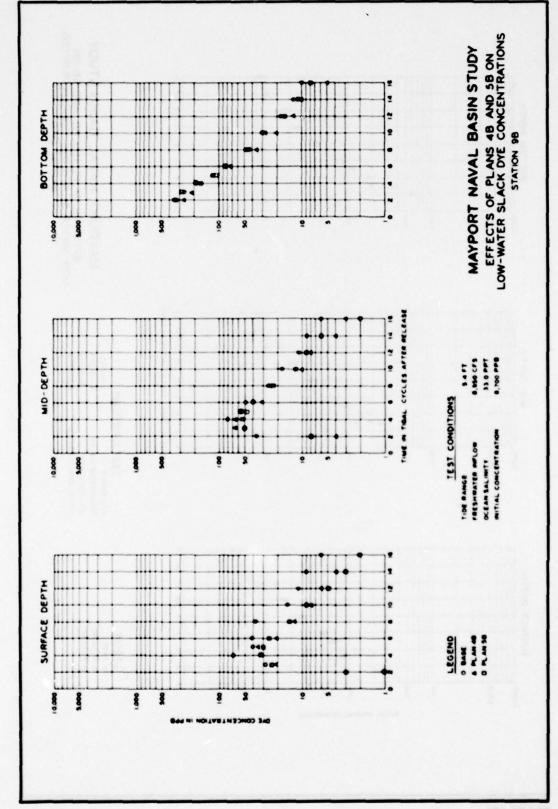


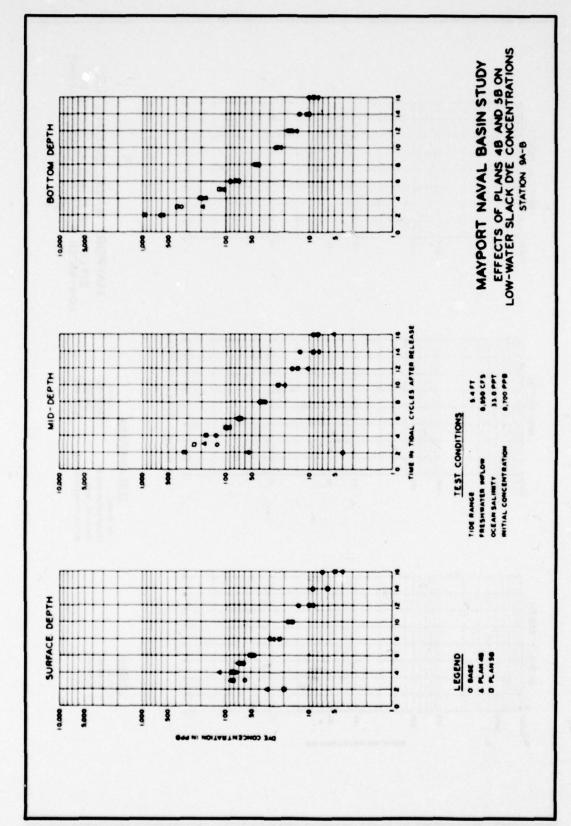


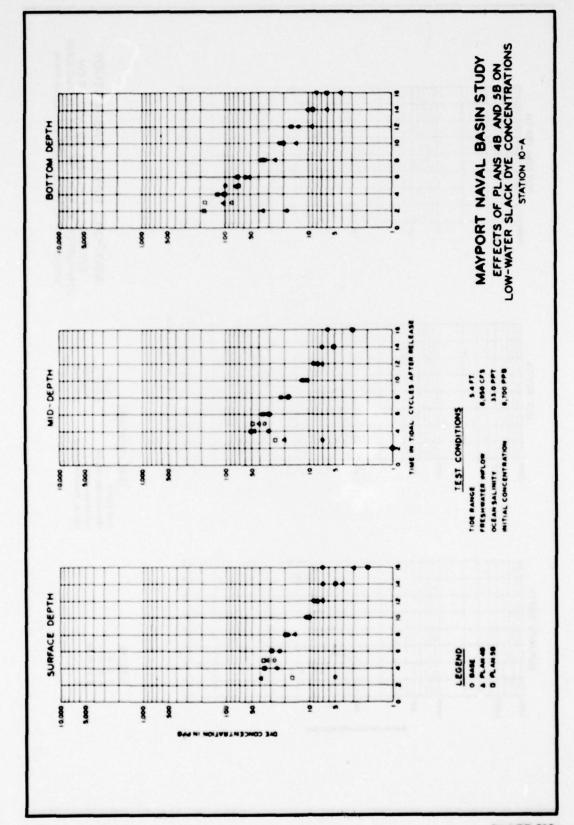












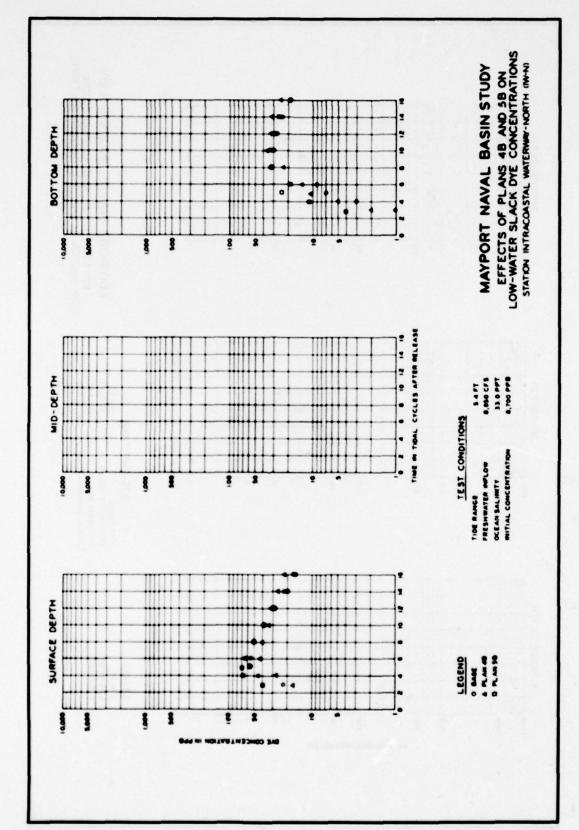
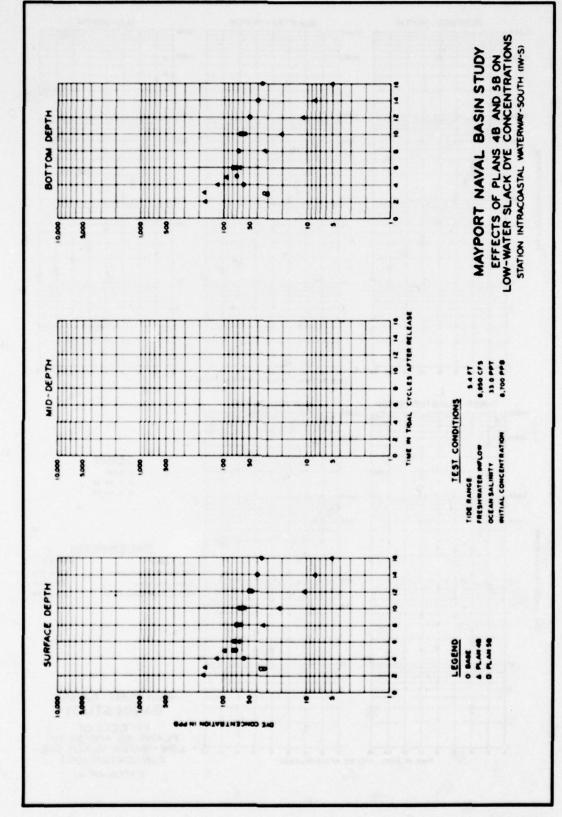
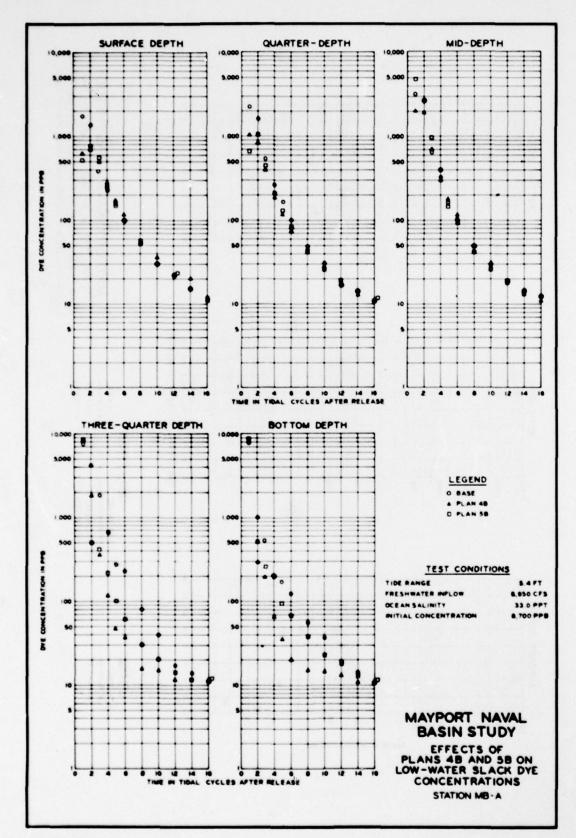
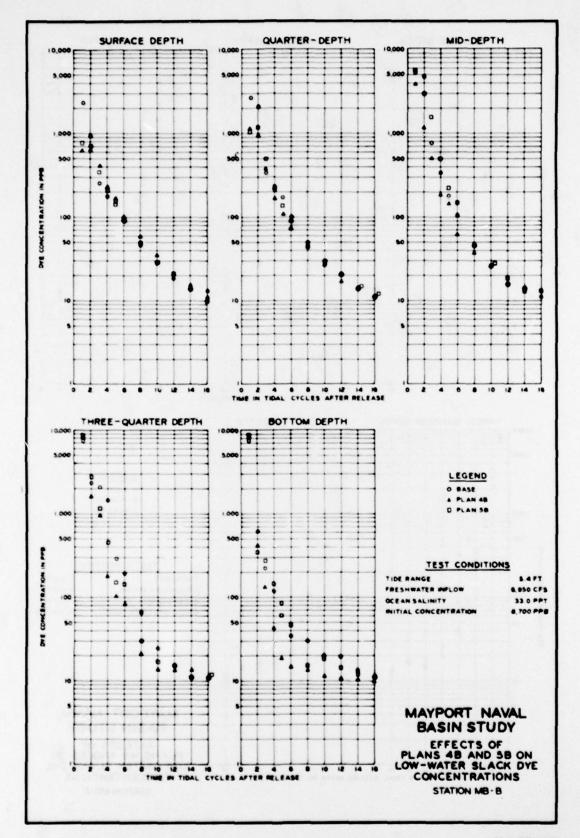
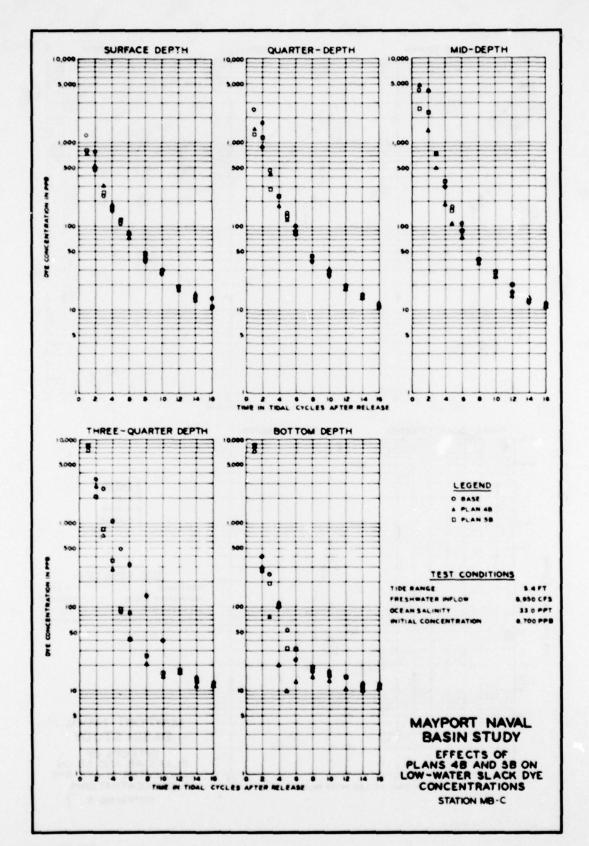


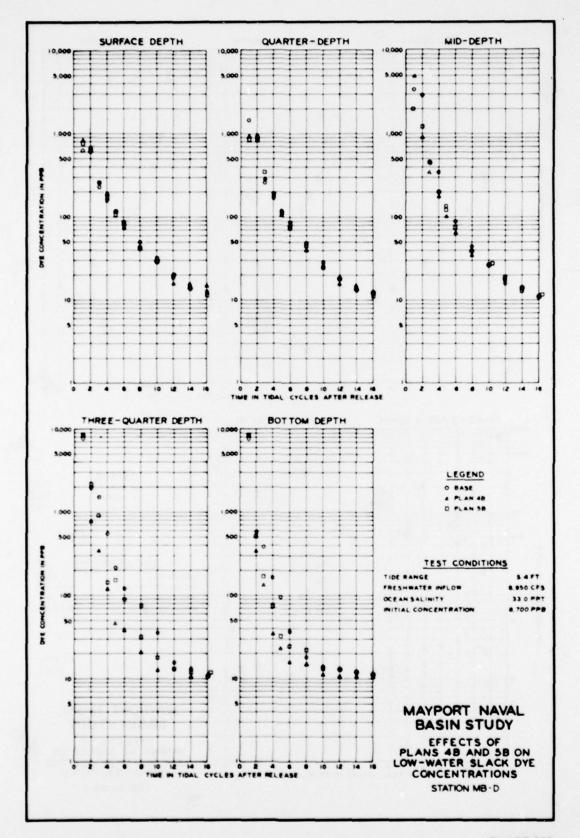
PLATE 284

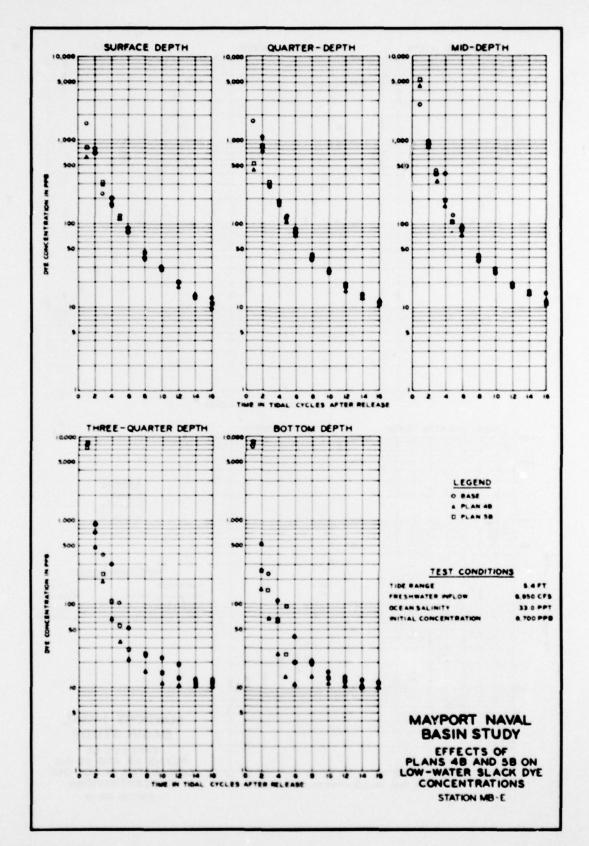


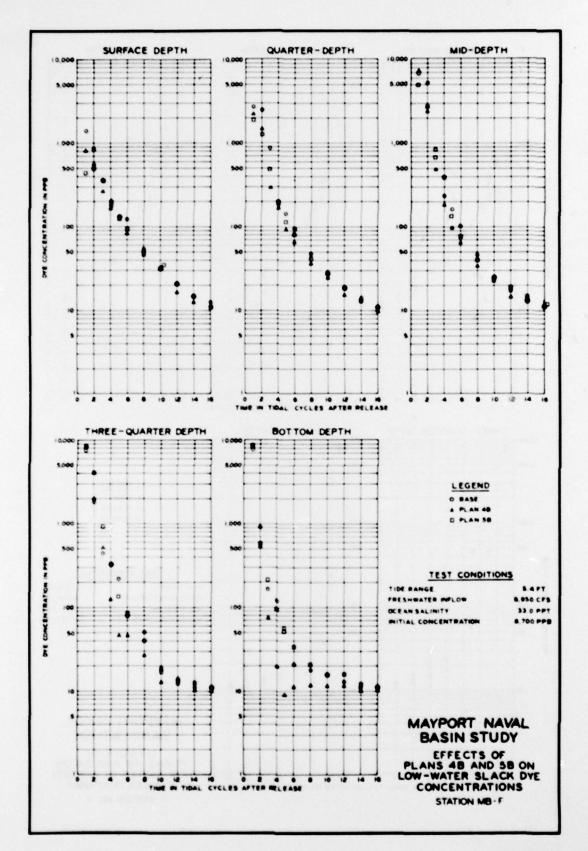


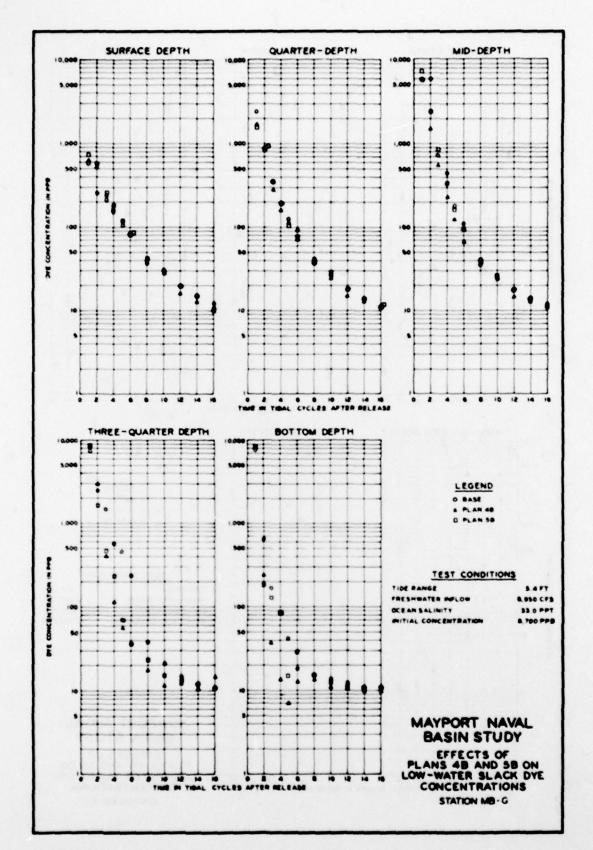


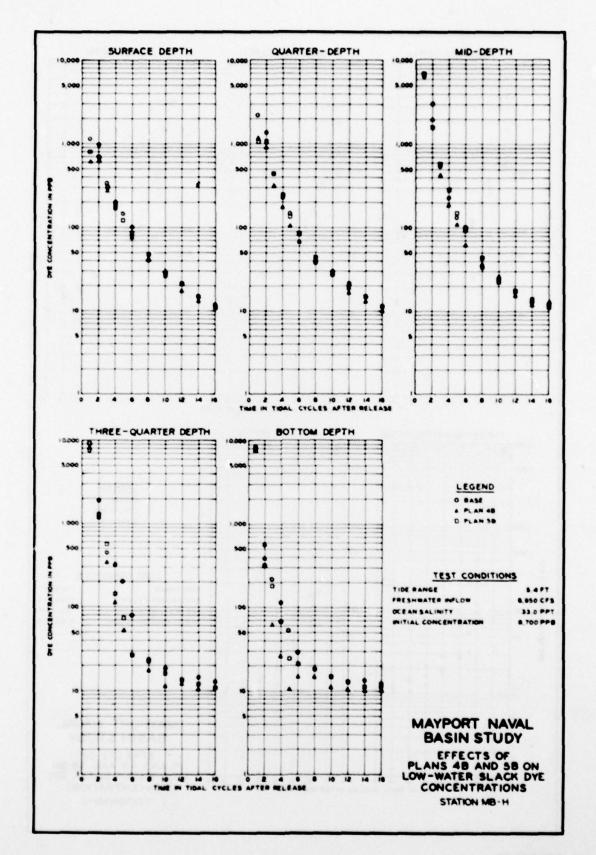


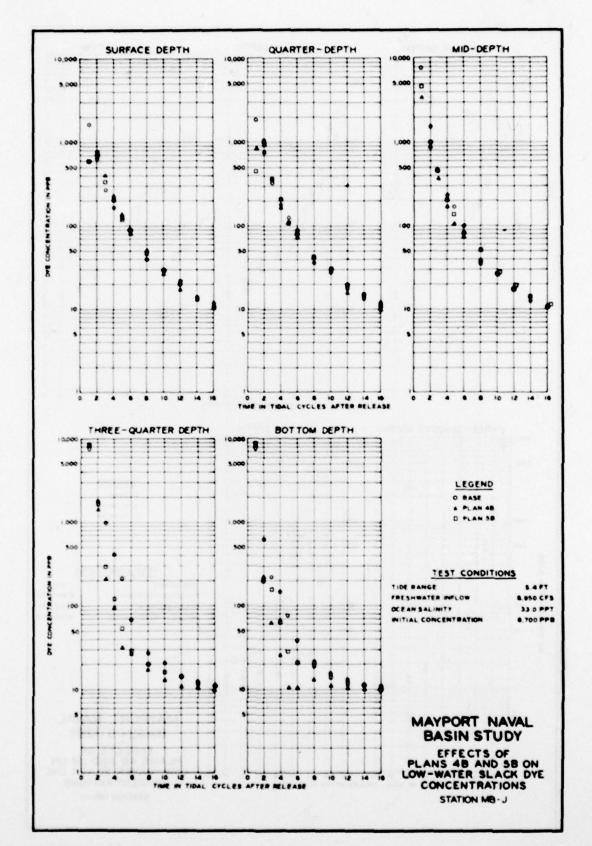


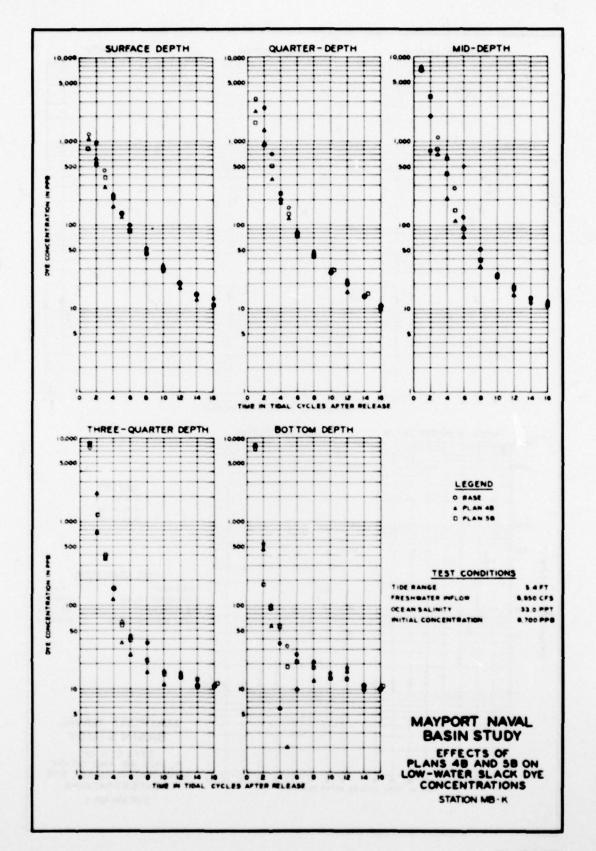


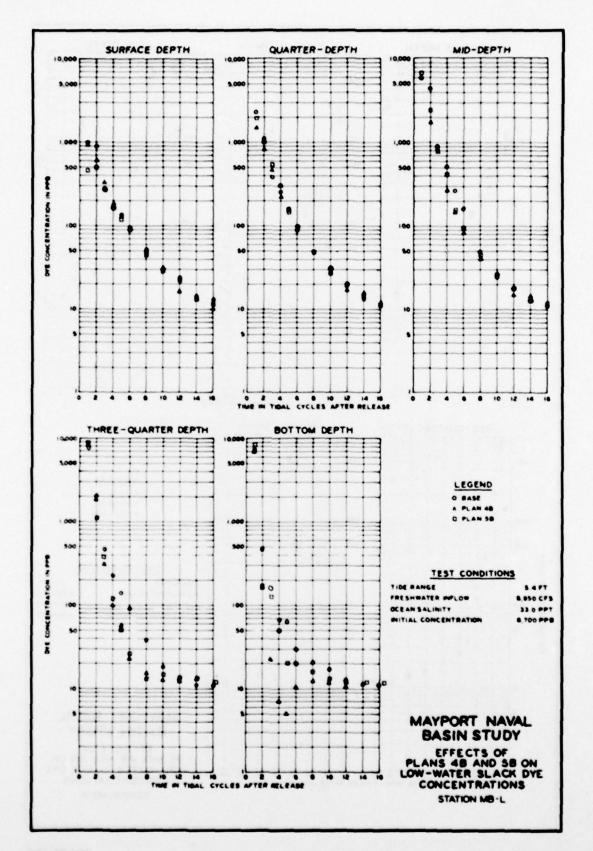


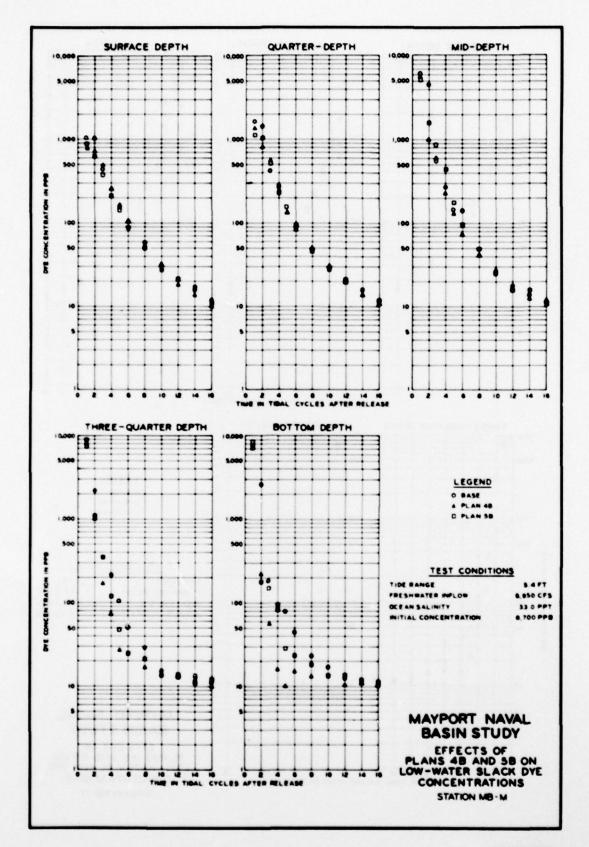


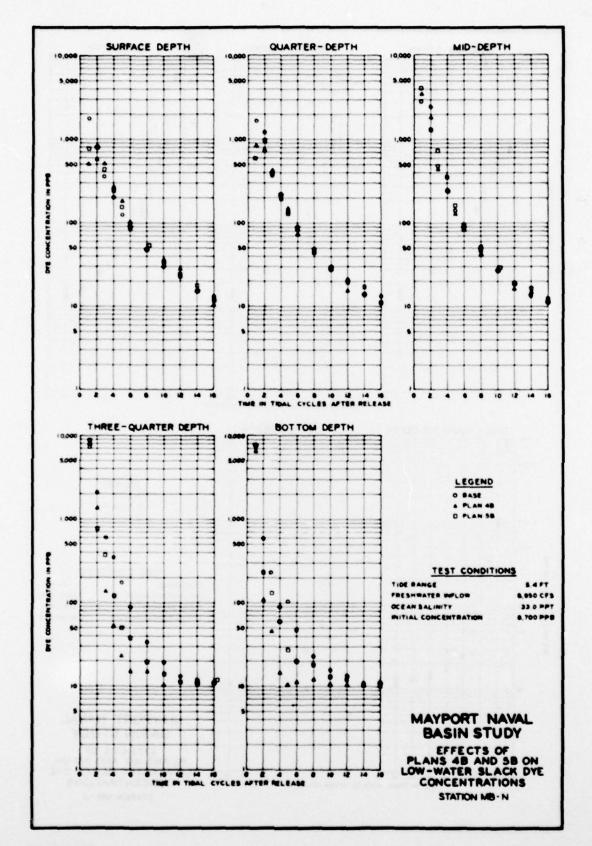


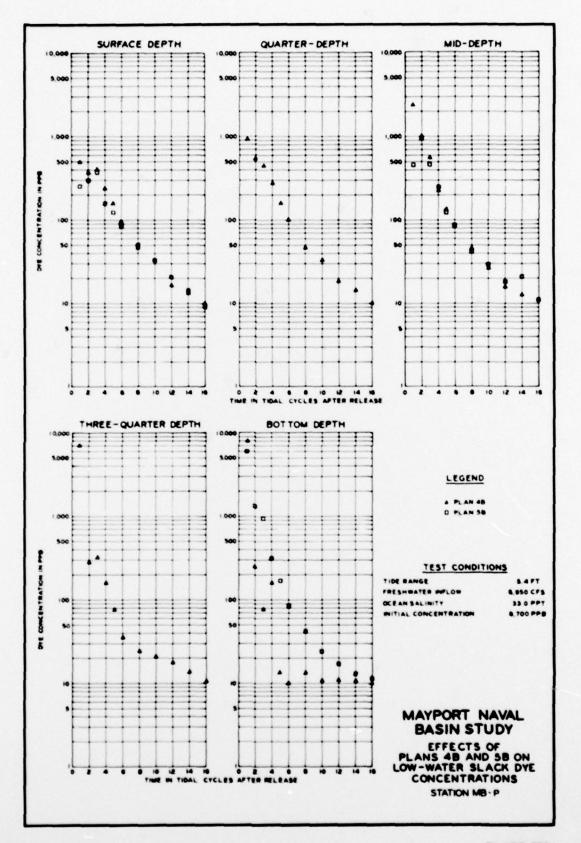












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Brogdon, Noble J

Mayport-Mill Cove model study; Report 2: Mayport Naval Basin study; hydraulic model investigation / by Noble J. Brogdon, Jr. Vicksburg, Miss. : U. S. Waterways Experiment Station ; Springfield, Va. : available from National Technical Information Service, 1979.

47, [56] p., 299 leaves of plates : ill. ; 27 cm. (Technical report - U. S. Army Engineer Waterways Experiment Station ; HL-79-12, Report 2)

Prepared for U. S. Army Engineer District, Jacksonville, Jacksonville, Florida.

Fixed-bed models.
 Flushing.
 Hydraulic models.
 Mayport-Mill Cove.
 Mayport Naval Basin.
 Salinity.

7. Shoaling. I. United States. Army. Corps of Engineers. Jacksonville District. II. Series: United States. Waterways Experiment Station, Vicksburg, Miss. Technical report; HL-79-12, Report 2.

TA7.N34 no.HL-79-12 Report 2